

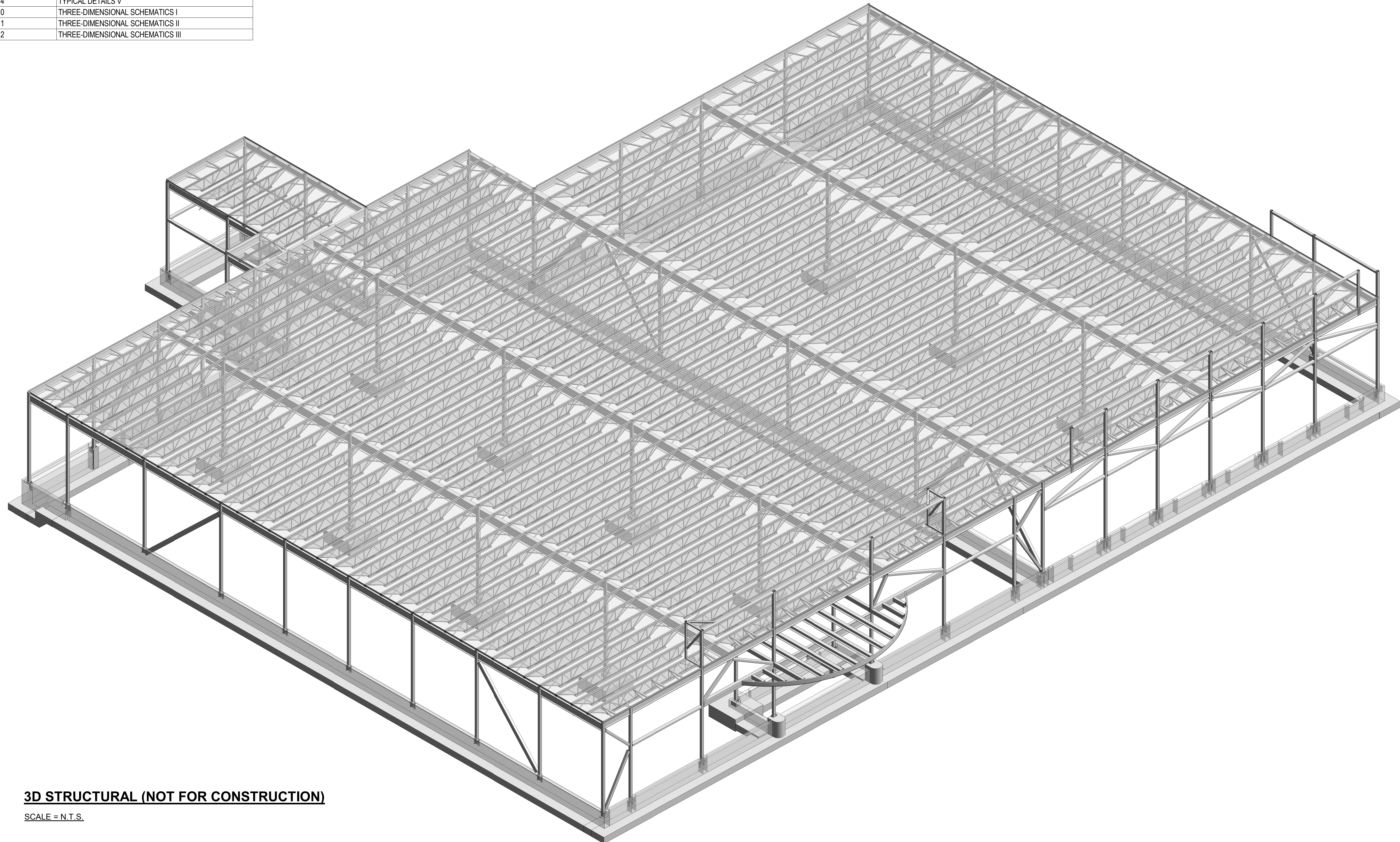
DRAWING LIST	
DRAWING NUMBER	DRAWING TITLE
S-0.0	COVER SHEET
S-0.1	SPECIFICATIONS I
S-0.2	SPECIFICATIONS II
S-1.0	FOUNDATION PLAN - CRU 1B
S-1.1	FOUNDATION PLAN - CRU 1C
S-1.2	ROOF FRAMING PLAN - CRU 1B
S-1.3	ROOF FRAMING PLAN - CRU 1C
S-1.4	SNOW DRIFT AND MECHANICAL PLAN - CRU 1B
S-1.5	SNOWDRIFT AND MECHANICAL PLAN - CRU 1C
S-2.0	ELEVATIONS-CRU 1B
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S-3.0	CANOPY AND HIGH PARAPET FRAMING PLAN
S-5.0	LOADING AND SCHEDULE
S-6.0	TYPICAL DETAILS I
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S-7.0	THREE-DIMENSIONAL SCHEMATICS I
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1300 FANSHAWE PARK RD. EAST. - CRU #1B AND C

STRUCTURAL DRAWINGS

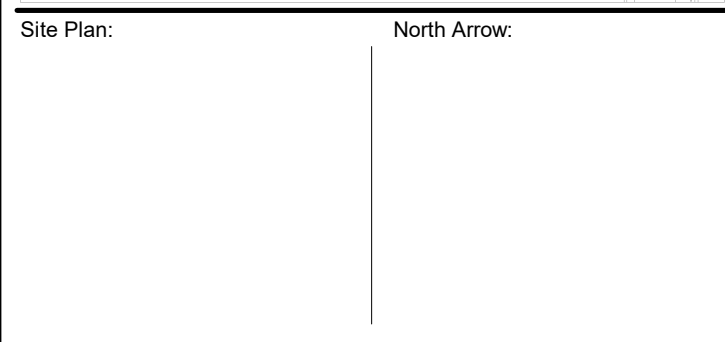
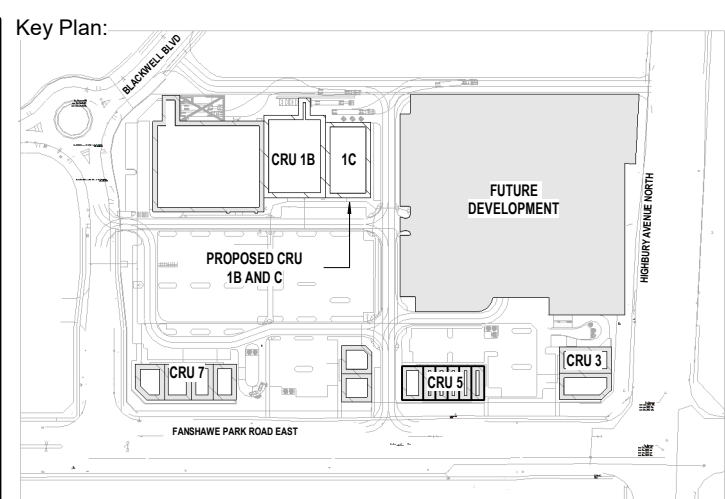
PROJECT #2023-102

1300 FANSHAWE PARK RD. EAST. LONDON, ON.



3D STRUCTURAL (NOT FOR CONSTRUCTION)

SCALE = N.T.S.



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Seal: _____ Seal: _____

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 CONTRACTOR IS FULLY RESPONSIBLE FOR MATTERS AFFECTING CONSTRUCTION.
 ANY MATERIAL ALTERATIONS CARRIED OUT DURING CONSTRUCTION BY THE CONTRACTOR OR ASSOCIATED SUB-CONTRACTOR SHALL BE CONFIRMED WITH THE ENGINEER PRIOR TO INSTALL. FAILURE TO DO SO RESULTS IN FULL CONTRACTOR RESPONSIBILITY FOR SYSTEMS AFFECTED.

No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 23 2024	ISSUED FOR COORDINATION

ISSUES/REVISION TABLE

Project:

WESTDELL

DEVELOPMENT CORP

1300 FANSHAWE PARK RD. EAST. - CRU #1B AND C

1300 FANSHAWE PARK RD. EAST. LONDON, ON.

Drawing Title:

COVER SHEET

Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
 Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
 Project Date: AUG. 2023
 Project No: 2023-102

Drawing No: S-0.0 | Revision: 2

CAST-IN-PLACE CONCRETE AND CONCRETE REINFORCING

23. PARKING STRUCTURE
- 23.1 THE DESIGN COMPLIES WITH THE STRUCTURAL REQUIREMENTS OF CAN/CSA S413-14, PARKING STRUCTURES.
- 23.2 SELECTED PROTECTION SYSTEMS:
- | STRUCTURAL COMPONENT | SYSTEM | CAN/CSA S413-14 REF. |
|-----------------------|--------------|----------------------|
| SUSPENDED SLABS/BEAMS | M (MEMBRANE) | TABLE 1 |
- 23.3 ALL CONCRETE SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.4 MINIMUM CONCRETE COVER SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES, TABLE 1, AND CLAUSE 7.3.8, UNLESS NOTED OTHERWISE.
- 23.5 ALL GUARDS, VEHICLE GUARDRAILS, EXPOSED HARDWARE AND EMBEDDED MATERIALS SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.6 ALL WELDED REINFORCEMENT SHALL MEET THE REQUIREMENTS OF MOST UP TO DATE CAN/CSA S413, PARKING STRUCTURES AND ASTM A1064/A1064M.
- 23.7 ALL MEMBRANES SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.8 ALL SERVICES SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.9 ALL HEATING CABLES AND HEATING PIPES FOR SNOW MELTING SYSTEMS SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.10 CURBS SHALL BE CAST AROUND STEEL COLUMNS AT SUSPENDED FLOORS AND AT GRADE LEVEL AND SHALL MEET THE ADDITIONAL REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.11 STEEL COLUMN BASE PLATES, ANCHOR BOLTS, AND THE PORTION OF STEEL COLUMNS LOCATED BELOW SLABS ON GROUND SHALL BE PROTECTED AGAINST CORROSION.
- 23.12 ALL EXPOSED STRUCTURAL STEEL SHALL BE PAINTED AND SHALL MEET THE REQUIREMENTS OF CAN/CSA S413, PARKING STRUCTURES.
- 23.13 THE CONSTRUCTION OF THE PARKING STRUCTURE SHALL BE IN ACCORDANCE WITH CAN/CSA A23.1 AND CAN/CSA S16, EXCEPT AS ADDITIONALLY SPECIFIED IN CAN/CSA S413, PARKING STRUCTURES.
- 23.14 THE SLAB SURFACE SHALL NOT BE OVERWORKED DURING FINISHING. MULTIPLE PASSES OF POWER FLOATS OR TROWELS CAN CREATE A FINE PASTE THAT IS SUBJECT TO SCALING. A SINGLE PASS IS NOT DETRIMENTAL.
- 23.15 VEHICLES THAT CAN TRACK ROAD SALTS INTO THE STRUCTURE SHALL NOT BE ALLOWED IN THE STRUCTURE UNTIL THE MEMBRANE SYSTEM IS INSTALLED.

MASONRY

1. DESIGN
- 1.1 MASONRY DESIGN TO CAN/CSA S304-14 "DESIGN OF MASONRY STRUCTURES" (LIMIT STATES DESIGN)
- 1.2 TOLERANCES TO LATEST CSA A371 "MASONRY CONSTRUCTION FOR BUILDINGS"
- 1.3 CONSULTANT AND G. C. WILL INSTALL MASONRY AND REJECT MASONRY THAT IS CHIPPED, CRACKED, OR BLEMISHED (STREAKED, STAINED OR OTHERWISE DAMAGED).
- 1.4 MAKE GOOD REJECTED MASONRY AS DIRECTED BY CONSULTANT
2. MATERIALS
- 2.1 ALL MATERIALS USED IN MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF LATEST CAN/CSA A371 "MASONRY CONSTRUCTION FOR BUILDINGS"
- 2.2 HOLLOW CONCRETE MASONRY UNITS TO LATEST CAN/CSA A165 MIN. COMPRESSIVE STRENGTH = 25MPa U.N.O.
- 2.3 MASONRY MORTAR/GROUT FILL TO CAN/CSA A179 "FINE GROUT" MIN. 20MPa STRENGTH AT 28 DAYS AND NOT LESS THAN MASONRY COMPRESSIVE STRENGTH, 175-200MM SLUMP TYPE U.N.O.
- 2.4 MASONRY CONNECTORS AND REINFORCEMENT TO LATEST CSA A370
- 2.5 HOT DIP GALVANIZING: TO ASTM A123/A123M AND ASTM A153/A153M, CLASS B2, MINIMUM 458 G/M² ZINC COATING ON ALL SURFACES.
- 2.6 MANUFACTURES HAVING PRODUCTS CONSIDERED ACCEPTABLE FOR USE:
- 2.6.1 BLOK-LOK
- 2.6.2 FERRO
- 2.7 ALL LADDER STEEL TO BE HEAVY DUTY 4.76 mm (3/16") GAUGE SIDE WIRE
3. ERECTION
- 3.1 CONSTRUCT MASONRY PLUMB, LEVEL AND TRUE TO LINE, WITH VERTICAL JOINTS IN ALIGNMENT.
- 3.2 LAY OUT COURSES AND BOND TO ACHIEVE CORRECT COURSES, HEIGHTS, AND CONTINUITY OF BOND ABOVE AND BELOW OPENINGS, WITH MINIMUM OF CUTTING.
- 3.3 LAY MASONRY IN FULL BED OF MOTOR, PROPERLY JOINED WITH OTHER WORK.
- 3.4 BUTTERING CORNERS OF JOINTS, AND DEEP OR EXCESSIVE FURROWING OF MORTAR JOINTS ARE NOT PERMITTED.
- 3.5 DO NOT USE CHIPPED, CRACKED OR OTHERWISE DAMAGED UNITS.
- 3.6 BUILD IN ITEMS REQUIRED TO BE BUILT INTO MASONRY. PREVENT DISPLACEMENT OF BUILT-IN ITEMS DURING CONSTRUCTION.
- 3.7 CHECK PLUMB, LOCATION AND ALIGNMENT FREQUENTLY, AS WORK PROGRESSES.
- 3.8 BRACE DOOR FRAMES TO MAINTAIN PLUMB, FILL SPACES BETWEEN FRAME JAMBS AND MASONRY WITH GROUT.
- 3.9 MAINTAIN MATERIALS AND SURROUNDING AIR TEMPERATURE TO MINIMUM 5 DEGREES CELSIUS AND MAXIMUM 50 DEGREES CELSIUS PRIOR TO, DURING, AND 48 HOURS AFTER COMPLETION OF MASONRY WORK.
- 3.10 DO NOT USE ANTI-FREEZE, LIQUID SALTS, OR OTHER SUBSTANCES TO LOWER THE FREEZING POINT OF MORTAR OR GROUT. CONFORM TO LATEST CSA A179
- 3.11 PROVIDE HEATED ENCLOSURES AND HEAT AS NECESSARY DURING COLD WEATHER CONSTRUCTION.
- 3.12 PREVENT FRESHLY LAID MASONRY FROM DRYING TOO RAPIDLY DURING HOT WEATHER BY MEANS OF WATERPROOF, NON-STAINING COVERINGS.
- 3.13 INSTALL ALL LOOSE STEEL LINTELS, CENTRE LINTEL OVER OPENING WIDTH
- 3.14 PROVIDE TEMPORARY BRACINGS FOR MASONRY WALLS TO RESIST WIND PRESSURE AND OTHER LATER LOADS DURING AND AFTER ERECTION UNTIL PERMANENT LATERAL SUPPORT IS IN PLACE
- 3.15 CONTACT CONSULTANT/ARCHITECT ABOUT SIZE AND LOCATION OF MASONRY MOVEMENT JOINTS PRIOR TO SITE FABRICATION.
- 3.16 SECURE WALL TIES TO STRUCTURAL BACK-UP AT MAXIMUM SPACING OF 400 mm X 600 mm (16" X 24") O.C. OR SMALLER IF SPECIFIED BY SUPPLIER.
- 3.17 SECURE WALL TIES TO STUDS USING A MINIMUM OF TWO FASTENERS OR MORE IF SPECIFIED BY SUPPLIER.
- 3.18 DOUBLE QUANTITY OF WALL TIES WITHIN 200 mm (8") OF WALL CORNERS, WALL OPENINGS AND ALONG PARAPET WALLS.
4. JOINTING
- 4.1 MAKE VERTICAL AND HORIZONTAL JOINTS EQUAL AND UNIFORM THICKNESS
- 4.2 ALLOW JOINTS TO SET JUST ENOUGH TO REMOVE EXCESS WATER, THEN TOOL WITH ROUND JOINTER TO RESULT IN SMOOTH, COMPRESSED, UNIFORMLY CONCAVE JOINTS.
- 4.3 STRIKE FLUSH JOINTS THAT WILL BE CONCEALED WITHIN THE WALL WHICH WILL RECEIVE A COATING OF PLASTER, TILE, INSULATION, RESILIENT BASE, BITUMINOUS FOUNDATION PROTECTION, OR OTHER JOINT-CONCEALING FINISH. DO NOT STRIKE FLUSH MORTAR JOINTS DESIGNATED TO RECEIVE PAINTED OR OTHER THIN FINISHES.
5. CUTTING
- 5.1 CUT OUT MASONRY NEATLY FOR RECESSED OR BUILT-IN OBJECTS. MAKE CUTS STRAIGHT, CLEAN AND FREE FROM UNEVEN EDGES. MAKE GOOD MASONRY WHICH HAS CRACKED OR BROKEN AS A RESULT OF CUTTING IN BUILT-IN OBJECTS.
6. PROVISIONS FOR MOVEMENT
- 6.1 LEAVE A 9.5 mm (3/8") SPACE BETWEEN MASONRY AND VERTICAL STRUCTURAL ELEMENTS FOR NON-LOADBEARING.
- 6.2 LEAVE A 11 mm (7/16") SPACE BETWEEN TOP OF NON-LOADBEARING WALLS AND PARTITIONS AND STRUCTURAL ELEMENTS.

STRUCTURAL STEEL

1. DESIGN
- 1.1 DESIGN DETAILS AND CONNECTIONS IN ACCORDANCE WITH REQUIREMENTS OF CAN/CSA-S16 AND CAN/CSA-S136 TO RESIST FORCES, MOMENTS, SHEARS, AND TO ALLOW FOR MOVEMENTS INDICATED.
- 1.2 WHEN SHEARS ARE NOT INDICATED ON DRAWINGS, SELECT OR DESIGN CONNECTIONS TO SUPPORT THE MAXIMUM OF A) REACTION FROM MASONRY UNIFORMLY DISTRIBUTED LOAD THAT CAN BE SAFELY SUPPORTED BY BEAM IN BENDING, PROVIDED NO POINT LOADS ACT ON BEAM OR B) MAXIMUM SHEAR CAPACITY OF THE BEAM IF THE BEAM SUPPORTS ANY POINT LOADS OR CANTILEVER OVER COLUMNS.
- 1.3 FOR COMPOSITE CONSTRUCTION, SELECT OR DESIGN MINIMUM END CONNECTION TO RESIST REACTION RESULTING FROM FACTORED MOMENT RESISTANCE AS TABULATED IN THE "HANDBOOK OF THE CANADIAN INSTITUTE OF STEEL CONSTRUCTION" ASSUMING 100% SHEAR CONNECTION WITH DEPTH OF STEEL DECK AND/OR SLAB SHOWN ON DRAWINGS.
2. SHOP DRAWINGS
- 2.1 SUBMIT DRAWINGS STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO OR PROJECT APPLICABLE JURISDICTIONS.
- 2.2 INDICATE PROFILES, SIZES, SPACING, LOCATIONS OF STRUCTURAL MEMBERS, OPENINGS, ATTACHMENTS, FASTENERS, FIELD CONNECTIONS, AND CAMBERS.
- 2.3 INDICATE ALL DETAILS AND INFORMATION NECESSARY FOR ASSEMBLY AND ERECTION PURPOSES, INCLUDING ANCHOR BOLT SETTING DIAGRAM FOR PROPER INSTALLATION.
3. QUALIFICATIONS
- 3.1 FABRICATE STRUCTURAL STEEL MEMBERS TO CISC CODE OF STANDARD PRACTICE AND CSA-W59.
- 3.2 MANUFACTURER QUALIFICATIONS: COMPANY SPECIALIZING IN MANUFACTURING THE PRODUCTS SPECIFIED IN THIS SECTION WITH MINIMUM THREE (3) YEARS EXPERIENCE.
- 3.3 INSTALLER QUALIFICATIONS: COMPANY SPECIALIZING IN PERFORMING THE WORK OF THIS SECTION WITH MINIMUM THREE (3) YEARS EXPERIENCE.
- 3.4 WELDERS CERTIFICATES: EMPLOY ONLY CERTIFIED WELDERS ON THE WORK, WITH VERIFIABLE QUALIFICATION TO CSA-W59 WITHIN THE PREVIOUS TWELVE (12) MONTHS.

4. MATERIALS
- 4.1 W-SHAPES AND CHANNELS: TO CSA-G40.20/G40.21, GRADE 350W, UNLESS NOTED OTHERWISE.
- 4.2 HOLLOW STRUCTURAL STEEL MEMBERS: TO CSA G40.20/G40.21, GRADE 350 W, CLASS C, UNLESS NOTED OTHERWISE.
- 4.3 PLATES AND ANGLES: TO CSA G40.20/G40.21, GRADE 300W, UNLESS NOTED OTHERWISE.
- 4.4 ANCHOR BOLTS: TO ASTM A307.
- 4.5 BOLTS, NUTS AND WASHERS: TO ASTM A325M, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS, HOT DIPPED GALVANIZED FOR EXTERIOR MEMBERS.
- 4.6 WELDING MATERIALS: TO CSA W48 SERIES, CSA W59 AND CERTIFIED BY CANADIAN WELDING BUREAU.
- 4.7 GROUT: TO ASTM C1107/C1107M, NON-SHRINK TYPE, PREMIXED COMPOUND CONSISTING OF NON-METALLIC AGGREGATE, CEMENT, WATER REDUCING AND PLASTICIZING ADDITIVES, CAPABLE OF DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF 50 MPa at 28 DAYS.
- 4.8 HOT DIP GALVANIZING: GALVANIZE STEEL, WHERE INDICATED AND EXTERIOR, TO CAN/CSA-G164, MINIMUM ZINC COATING OF 275 G/M².
5. FABRICATION
- 5.1 FABRICATE STRUCTURAL STEEL IN ACCORDANCE WITH CAN/CSA-S16, CAN/CSA-S136, AND IN ACCORDANCE WITH THE APPROVED SHOP DRAWINGS.
- 5.2 SPLICING WILL NOT BE ALLOWED WITHOUT THE APPROVAL OF THE CONSULTANT AT THE SHOP DRAWING REVIEW STAGE. SPLICING WILL THEN ONLY BE ALLOWED IF THE LENGTH OF THE FABRICATED MEMBER REQUIRED IS LONGER THAN THAT NORMALLY PRODUCED AT THE MILL. IF A MEMBER IS SPLICED, THE FABRICATOR AND SHOP DRAWING DESIGN ENGINEER SHALL ENSURE THAT THE FULL SECTION PROPERTIES ARE CONTINUOUS OVER THE SPLICE.
- 5.3 ALL MEMBERS SHALL BE TRUE TO LENGTH SUCH THAT ASSEMBLY MAY BE DONE WITHOUT FILLERS.
- 5.4 CONTINUOUSLY SEAL JOINED MEMBERS WITH CONTINUOUS WELDS OR INTERMITTENT WELDS AND PLASTIC FILLER, WHERE FULL SEAL IS NOT POSSIBLE, PROVIDE WELD HOLES.
- 5.5 MAKE GOOD WELDS WHICH SHOW INCLUSIONS, POROSITY, OR LACK OF FUSION PENETRATION BEYOND THE TOLERANCES SET OUT IN CSA W59.
- 5.6 GRIND ALL EXPOSED WELDS SMOOTH IF NEEDED.
- 5.7 UNLESS NOTED OTHERWISE, FABRICATE CONNECTIONS FOR BOLT, NUT AND WASHER CONNECTORS.
- 5.8 TAKE CARE TO MINIMIZE DISTORTION DUE TO WELDING AND GALVANIZING PROCEDURES. STRAIGHTEN MEMBERS ARE REQUIRED TO MAINTAIN FABRICATION TOLERANCES OF CAN/CSA S-16.
- 5.9 PROVIDE HOLES FOR CONNECTING THE WORK OF OTHER TRADES, WHERE HOLE LOCATIONS CAN BE DETERMINED PRIOR TO FABRICATION, AND ONLY WHERE SUCH HOLES WILL NOT IMPAIR THE PERFORMANCE OF THE MEMBER.
- 5.10 UNLESS OTHERWISE SPECIFIED, MAKE HOLES 2 mm (3/32") LARGER THAN THE NOMINAL DIAMETER OF THE FASTENER. HOLES MAY BE PUNCHED, SUB-PUNCHED, DRILLED, OR REAMED AS PERMITTED IN CSA S16.
- 5.11 PROVIDE WELDED STRAP OR REINFORCING BAR ANCHORS FOR ATTACHMENT TO CONCRETE OR MASONRY, AS SHOWN IN THE TYPICAL DETAILS.
- 5.12 BEAR ANGLE LINTELS AS INDICATED ON DRAWINGS, BUT NOT LESS THAN 200 mm (8") AT EACH END, WELD ANGLES TOGETHER WHERE THE UPSTANDING LEGS ARE BACK TO BACK.
- 5.13 MARK MATERIALS IN ACCORDANCE WITH CSA G40.20/G40.21. DO NOT USE DIE STAMPING, WHEN STEEL IS TO BE LEFT IN UNPAINTED CONDITION, PLACE MARKING AT LOCATIONS NOT VISIBLE FROM EXTERIOR AFTER ERECTION.
6. FINISH
- 6.1 CLEAN MEMBERS, REMOVE LOOSE MILL SCALE, RUST, OIL, DIRT AND FOREIGN MATTER. PREPARE SURFACES ACCORDING TO SSPC-SP-3.
- 6.2 SHOP PRIME STRUCTURAL STEEL, EXCEPT FOR:
- 6.2.1 SURFACES TO BE IN CONTACT WITH CONCRETE OR SOIL.
- 6.2.2 SURFACES AND EDGES TO BE FIELD WELDED.
- 6.2.3 STRIP PAINT FROM BOLTS, NUTS, CORNERS, AND SHARP EDGES BEFORE PRIME COAT IS DRY.
- 6.2.4 CONFIRM PRIMER REQUIREMENTS WITH ARCHITECT FOR STEEL MEMBERS BEING FIRE-RATED (APPROVED BY CSA STANDARDS) AND PROVIDE COPY OF APPROVED SPECS TO IE DESIGN.
- 6.3 APPLY PRIMER AND TWO COATS OF COAL TAR EPOXY TO BASES OF EXTERIOR CANOPY COLUMNS OR AS SPECIFIED BY ARCHITECT.
- 6.4 HOT DIP GALVANIZING: WHERE INDICATED, GALVANIZE STEEL, TO CAN/CSA-G164, MINIMUM ZINC COATING OF 600 G/M².

7. ERECTION
- 7.1 ERECT STRUCTURAL STEEL IN ACCORDANCE WITH CAN/CSA S16 AND THE APPROVED ERECTION DRAWINGS.
- 7.2 ALLOW FOR ERECTION LOADS, AND FOR SUFFICIENT TEMPORARY BRACING TO MAINTAIN STRUCTURE SAFE, PLUMB, AND IN TRUE ALIGNMENT UNTIL COMPLETION OF PERMANENT BRACING.
- 7.3 FIELD WELD COMPONENTS AS INDICATED ON SHOP DRAWINGS.
- 7.4 FIELD CONNECT MEMBERS WITH THREADED FASTENERS; TORQUE TO REQUIRED RESISTANCE AS RECOMMENDED IN CAN/CSA S16.
- 7.5 ASSEMBLE BOLTED PARTS TOGETHER SOLIDLY, DO NOT SEPARATE WITH GASKETS OR ANY OTHER INTERPOSED COMPRESSIBLE MATERIAL.
- 7.6 DO NOT DISTORT OR ENLARGE HOLES. HOLES IN ADJACENT PARTS SHALL MATCH SUFFICIENTLY WELL TO PERMIT EASY ENTRY OF BOLTS.
- 7.7 FIELD CUTTING OR ALTERING OF STRUCTURAL MEMBERS IS NOT PERMITTED WITHOUT WRITTEN STAMPED APPROVAL FROM THE SUPPLIER'S DESIGN ENGINEER.
- 7.8 AFTER ERECTION, PRIME WELDS, ABRASIONS, AND SURFACES NOT SHOP PRIMED, EXCEPT SURFACES TO BE IN CONTACT WITH CONCRETE.
- 7.9 GROUT UNDER BASE PLATES, TROWEL GROUTED SURFACE SMOOTH, SPLAY NEATLY TO 45 DEGREES.
8. TOLERANCES
- 8.1 AS PER MOST RECENT AND APPLICABLE CAN/CSA S16.
- 8.2 MAXIMUM VARIATION FROM PLUMB: 6 mm (1/4") PER STOREY, NON-CUMULATIVE.
- 8.3 MAXIMUM VARIATION FROM TRUE ALIGNMENT: 6 mm (1/4").
9. FIELD QUALITY CONTROL
- 9.1 FIELD INSPECTION AND TESTING OF MATERIALS AND WORKMANSHIP SHALL BE CARRIED OUT BY AN INDEPENDENT INSPECTION/TESTING AGENCY. INSPECT STEEL, WELDS, AND BOLTED CONNECTIONS FOR ALIGNMENT AND STRUCTURAL INTEGRITY. SUBMIT REPORTS TO CONSULTANT WITHIN 1 WEEK OF COMPLETION OF INSPECTION.

LIGHTWEIGHT STEEL FRAMING

1. DESIGN
- 1.1 BASE DESIGN ON LIMIT STATES DESIGN PRINCIPLES USING FACTORED LOADS AND RESISTANCES. DETERMINE RESISTANCES AND RESISTANCE FACTORS IN ACCORDANCE WITH THE MOST RECENT APPLICABLE ONTARIO BUILDING CODE AND CAN/CSA-S16.
- 1.2 FOR WIND LOAD CALCULATIONS, THE REFERENCE VELOCITY PRESSURE, Q, SHALL BE BASED ON A 1 IN 50 PROBABILITY OF BEING EXCEEDED IN ANY ONE YEAR.
- 1.3 CONFORM TO THE REQUIREMENTS OF FIRE RATED ASSEMBLIES WHICH HAVE BEEN TESTED IN ACCORDANCE WITH CANULC S101 AND PROVIDE A FIRE RESISTANCE RATING AS INDICATED ON THE DRAWINGS.
- 1.4 THE MINIMUM DESIGN THICKNESS FOR STUDS AND TRACK SHALL BE 0.84 mm FOR 89 mm (3.5") AND 152 mm (6") WIDTHS. USE GREATER STUD/TRACK THICKNESSES IF REQUIRED BY THE DESIGN CRITERIA.
- 1.5 FOR WALL STUDS SUPPORTING BRICK VENEER, THE MINIMUM DESIGN THICKNESS EXCLUSIVE OF COATING SHALL BE THE GREATER OF THE DESIGN THICKNESSES LISTED ABOVE OR 1.12 mm.
- 1.7 THE MINIMUM DESIGN THICKNESS FOR BRIDGING CHANNEL SHALL BE 1.22 mm. USE GREATER BRIDGING CHANNEL DESIGN THICKNESS IF REQUIRED BY THE DESIGN CRITERIA.
- 1.8 THE MINIMUM DESIGN THICKNESS FOR CLIP ANGLES SHALL BE 1.52 mm. USE GREATER CLIP ANGLE THICKNESS IF REQUIRED BY THE DESIGN CRITERIA.
- 1.9 MAXIMUM FLEXURAL DEFLECTIONS UNDER SPECIFIED LIVE OR WIND LOADS SHALL CONFORM TO THE FOLLOWING:
- 1.9.1 WALL STUDS SUPPORTING MASONRY VENEER SHALL MEET THE REQUIREMENTS OF CSA S304.1 WITH STUD DEFLECTIONS LIMITED TO L/480
- 1.9.2 STUDS SUPPORTING OTHER FINISHES: L/180
- 1.9.3 BUILDING SWAY DUE TO ALL EFFECTS, 1/500 OF BUILDING HEIGHT OR 1/500 OF STOREY HEIGHT.
- 1.9.4 DESIGN BRIDGING TO PREVENT MEMBER ROTATION AND MEMBER TRANSLATION PERPENDICULAR TO THE MINOR AXIS. PROVIDE SECONDARY STRESS EFFECTS DUE TO TORSION BETWEEN LINES OF BRIDGING. DO NOT RELY ON COLLATERAL SHEATHING TO HELP RESTRAIN MEMBER ROTATION AND TRANSLATION PERPENDICULAR TO THE MINOR AXIS.
- 1.9.5 DESIGN ANCHORAGE AND SPLICE DETAILS FOR BRIDGING.
- 1.9.6 ALLOW FOR LOADS DUE TO ANCHORAGE OF CLADDING AND INTERIOR WALL MOUNTED FIXTURES WHERE SHOWN.
- 1.9.8 CONNECT COLD-FORMED METAL FRAMING MEMBERS BY BOLTING, WELDING OR SCREWING
- 1.9.9 ALLOW FOR APPROPRIATE END ECCENTRICITIES IN THE DESIGN OF AXIAL LOAD-BEARING MEMBERS
- 1.9.10 DESIGN INTERIOR AXIAL LOAD BEARING WALLS WITH NOMINAL LATERAL WIND LOAD OF 0.24 kPa IN COMBINATION WITH THE REQUIRED AXIAL LOADS
- 1.9.10 PROVIDE LINTEL, SILL AND JAMB MEMBERS AND CONNECTIONS IN STUD WALLS TO FRAME OPENINGS LARGER THAN 100 mm IN ANY DIRECTION.
- 1.9.11 ANCHOR TOP AND BOTTOM TRACK TO THE STRUCTURE AT A MAXIMUM SPACING OF 800 mm (32") O.C. CLOSER SPACING MAY BE REQUIRED TO SATISFY STRUCTURAL REQUIREMENTS.

2. SHOP DRAWINGS
- 2.1 SUBMIT STAMPED SHOP DRAWINGS AS SPECIFIED IN REQUIRED SUBMITTALS ON STRUCTURAL DRAWINGS.
- 2.2 INCLUDE SHOP DETAILS AND ERECTION DIAGRAMS. INDICATE MEMBER SIZE, LOCATION, THICKNESSES EXCLUSIVE OF COATINGS, COATINGS AND MATERIAL TYPES.
- 2.3 INCLUDE CONNECTION DETAILS FOR ATTACHING FRAMING TO ITSELF AND FOR ATTACHMENT TO THE STRUCTURE. SHOW SPLICE DETAILS WHERE PERMITTED.
- 2.4 INDICATE DIMENSIONS, OPENINGS, REQUIREMENTS FOR RELATED WORK AND CRITICAL INSTALLATION PROCEDURES. SHOW TEMPORARY BRACING REQUIRED FOR ERECTION PURPOSES.
- 2.5 INDICATE DESIGN LOADS
- 2.6 EACH SHOP DRAWING SUBMITTED SHALL BEAR THE STAMP AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK.
- 2.7 THE SHOP DRAWING ENGINEER WILL UNDERTAKE PERIODIC FIELD REVIEW DURING CONSTRUCTION AND SHALL SUBMIT REPORTS AS DESCRIBED BELOW:
- 2.8 INCLUDE REVIEW OF MILL TESTS REPORTS, WELDED AND SCREWED CONNECTIONS, CONNECTIONS TO THE MAIN STRUCTURE, MEMBER SIZES, LOCATION AND MATERIAL THICKNESS, COATING THICKNESS, ERECTION TOLERANCES, AND ALL FIELD CUTTING

3. QUALIFICATIONS
- 3.1 FABRICATORS DESIGN ENGINEER: A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK TO DESIGN THE COLD-FORMED METAL FRAMING SYSTEM; TO PREPARE, SEAL AND SIGN SHOP DRAWINGS; AND TO PERFORM FIELD REVIEW, STAMPED SHOP DRAWINGS TO SHOW BOTH DESIGN AND INSTALLATION REQUIREMENTS.
- 3.2 INSTALLERS: COMPANY SPECIALIZING IN INSTALLING COLD-FORMED METAL FRAMING SYSTEMS, WITH MINIMUM OF TEN YEARS EXPERIENCE AND A MEMBER IN GOOD STANDING OF THE CANADIAN SHEET STEEL BUILDINGS INSTITUTE (CSSBI)
- 3.3 WELDERS: COMPANIES CERTIFIED BY THE CANADIAN WELDING BUREAU TO CSA W47.1, AND HAVING WELDERS QUALIFIED FOR THE BASE MATERIAL TYPES AND THICKNESSES THAT ARE TO BE WELDED.
4. DELIVERY STORAGE AND HANDLING
- 4.1 STORE PRODUCTS PROTECTED FROM CONDITIONS THAT MAY CAUSE PHYSICAL DAMAGE OR CORROSION.
- 4.2 HANDLE AND LIFT PREFABRICATED PANELS CAREFULLY TO AVOID PERMANENT DISTORTION TO ANY MEMBER OR COLLATERAL MATERIAL.
5. MANUFACTURES HAVING PRODUCTS CONSIDERED ACCEPTABLE FOR USE:
- 5.1 BAILEY METAL PRODUCTS.
- 5.2 DIETRICH METAL FRAMING.
- 5.3 MITEK CANADA INC.
7. MATERIALS
- 7.1 STEEL: TO CAN/CSA-S136, IDENTIFIED ON SHOP DRAWINGS AS TO SPECIFICATION, GRADE, MECHANICAL PROPERTIES AND COATING TYPE AND THICKNESS.
- 7.2 BOLTS AND NUTS: TO ASTM A307 OR ASTM A325M; HOT-DIPPED GALVANIZED, CW WASHERS.
- 7.3 SCREWS: GALVANIZED STEEL, SELF-TAPPING, TO ASTM C1513
- 7.4 WELDING MATERIALS: TO CSA W59
- 7.5 WELDING ELECTRODES: 480MPa MINIMUM TENSILE STRENGTH SERIES, E.G. E480XX OR ER480X-X
- 7.6 TOUCH-UP PAINT: ZINC RICH PAINT FOR TOUCHING UP WELDS AND DAMAGED METALLIC COATINGS, TO CAN/CSG8-1.181.
8. MANUFACTURED ITEMS
- 8.1 LOAD-BEARING STEEL STUDS, TRACKS AND BRACING: TO ASTM C965, FINISHES, SIZE AND THICKNESSES AS IDENTIFIED ON ACCEPTED SHOP DRAWINGS
9. FABRICATION
- 9.1 EXCEPT AS NOTED HEREIN, FABRICATE WALL FRAMING COMPONENTS TO CAN/CSG8-7.1 AND IN ACCORDANCE WITH APPROVED SHOP DRAWINGS.
- 9.2 WHERE SPECIFIED, PROVIDE CUT-OUTS CENTRED IN THE WEBS OF MEMBERS TO ACCOMMODATE SERVICES AND THROUGH-THE-KNOCKOUT STYLE BRIDGING. LIMIT THE DISTANCE FROM THE CENTRELINE OF THE LAST UNREINFORCED CUT-OUT TO THE END OF THE MEMBER TO BE NOT LESS THAN 300 mm, THE EFFECT OF CUT-OUTS ON THE STRENGTH AND STIFFNESS OF THE MEMBER SHALL BE CONSIDERED.
- 9.3 LENGTH TOLERANCES OF MEMBERS:
- 9.3.1 TRACKS: NONE
- 9.3.2 AXIAL LOADBEARING STUDS: PLUS OR MINUS 1.5 mm
- 9.3.3 CROSS SECTIONAL GEOMETRY TOLERANCES FOR MEMBERS SHALL CONFORM TO THE FOLLOWING:
- 9.3.3.1 MEMBER DEPTH: MINUS 1 mm, PLUS 2 mm
- 9.3.3.2 FLANGE DEPTH: MINUS 1 mm, PLUS 2 mm; MINIMUM 31 mm WIDTH.
- 9.3.3.3 LIP LENGTH: PLUS 4 mm
- 9.3.3.4 THICKNESS: TO CSA S136
- 9.3.4 CORNER ANGLES: PLUS OR MINUS 3 DEGREES
- 9.3.5 MARK THE STEEL THICKNESS, EXCLUSIVE OR COATING, ON EACH MEMBER BY EMBOSSING, STAMPING WITH INDELIBLE INK OR BY COLOUR CODING.
10. FINISHES
- 10.1 STEEL: GALVANIZED TO ASTM A653/A653M, Z275 COATING DESIGNATION, OR ASTM A792/A792M, AZM150 COATING DESIGNATION.

11. FASTENERS AND WELDS
- 11.1 ENSURE THAT CONNECTED PARTS ARE IN CONTACT, PROVIDE CLAMPING BEFORE WELDING OR INSTALLING SCREWS AS REQUIRED.
- 11.2 WELDS: TO CAN/CSA-S136, CSA W59 AND ANSII/AWS D1.3, AS APPLICABLE.
- 11.3 SHEET METAL SCREWS SHALL BE OF THE MINIMUM DIAMETER INDICATED ON THE SHOP DRAWINGS BUT NOT LESS THAN #8.
- 11.4 PENETRATION OF SHEET METAL SCREWS BEYOND JOINED MATERIALS TO BE NOT LESS THAN 3 EXPOSED THREADS.
- 11.5 SHEET METAL SCREW THREAD TYPES, DRILLING CAPABILITY AND INSTALLATION SHALL CONFORM TO THE MANUFACTURER'S RECOMMENDATIONS.
- 11.6 PROVIDE SHEET METAL SCREWS WITH LOW PROFILE HEADS WHERE COVERED BY SHEATHING MATERIALS.
- 11.7 INSTALL CONCRETE ANCHORS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS
12. ERECTION
- 12.1 ERECT COLD-FORMED METAL FRAMING TO ASTM C107
- 12.2 ERECT COLD-FORMED METAL FRAMING TRUE AND PLUMB WITHIN THE SPECIFIED TOLERANCES.
- 12.3 EMPLOY TEMPORARY BRACING WHERE NECESSARY TO WITHSTAND ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECT DURING ERECTION AND SUBSEQUENT CONSTRUCTION. LEAVE TEMPORARY BRACING IN PLACE AS LONG AS REQUIRED FOR THE SAFETY AND INTEGRITY OF THE STRUCTURE.
- 12.4 DO NOT ALLOW GAP BETWEEN THE END OF THE STUD AND THE WEB OF THE TRACK TO EXCEED 1.5 mm IN TOP AND BOTTOM TRACKS.
- 12.5 ALIGN ADJACENT OR ABUTTING MEMBERS IN THE SAME PLANE TO WITHIN PLUS OR MINUS 0.5 mm MAXIMUM.
- 12.6 SPACE STUDS WITHIN 3 mm EITHER DIRECTION OF THE DESIGN SPACING. THE CUMULATIVE ERROR IN SPACING SHALL NOT EXCEED THE REQUIREMENTS OF THE FINISHING MATERIALS.
- 12.7 ALIGN WEB CUT-OUTS IN STUDS AND JOISTS AS REQUIRED FOR THE INSTALLATION OF THROUGH-THE-KNOCKOUT STYLE BRIDGING AND SERVICES.
- 12.8 TAKE FIELD MEASUREMENTS NECESSARY TO ENSURE THE PROPER FIT OF MEMBERS.
- 12.9 USE EITHER SAWS OR SHEARS TO CUT MEMBERS. DO NOT TORCH CUT.
- 12.10 REINFORCE CUT-OUTS WHEN THE DISTANCE FROM THE CENTRE LINE OF THE CUT-OUT TO THE END OF THE MEMBER IS LESS THAN 300 mm (12"). SUBMIT REINFORCING DETAIL TO CONSULTANT FOR APPROVAL.
- 12.11 LOCATE JOISTS, TRUSSES, AND THEIR END STIFFENERS, DIRECTLY OVER AXIAL LOAD BEARING MEMBERS. ALTERNATELY, PROVIDE A LOAD DISTRIBUTION MEMBER TO TRANSFER LOADS. DO NOT USE COLD-FORMED METAL TRACK AS A LOAD DISTRIBUTION MEMBER.
- 12.12 REPLACE MEMBERS WITH LOCALIZED DAMAGE.
- 12.13 INSTALL ADDITIONAL STUDS AT ABUTTING WALLS, OPENINGS, TERMINATIONS AGAINST OTHER MATERIALS AND ON EACH SIDE AT CORNERS UNLESS EXPLICITLY DETAILED OTHERWISE ON SHOP DRAWINGS.
- 12.14 DO NOT SPLICE AXIAL LOAD-BEARING MEMBERS.

13. TOLERANCES
- 13.1 FOR THE PURPOSE OF THIS SECTION, CAMBER IS DEFINED AS THE DEVIATION FROM STRAIGHTNESS OF A MEMBER OF ANY PORTION OF A MEMBER WITH RESPECT TO ITS MAJOR AXIS, AND SWEEP IS DEFINED AS THE DEVIATION FROM STRAIGHTNESS OF A MEMBER OR ANY PORTION OF A MEMBER WITH RESPECT TO ITS MINOR AXIS.
- 13.2 PLUMBNESS:
- 13.2.1 AXIAL LOADBEARING MEMBERS: 1/1000TH OF THE MEMBER LENGTH
- 13.3 OUT-OF-STRAIGHTNESS: INCLUDING CAMBER AND SWEEP:
- 13.3.1 AXIAL LOADBEARING MEMBERS: 1/1000TH OF THE MEMBER LENGTH
- 13.3.2 TRACK: CAMBER NOT TO EXCEED 1/1000TH OF THE MEMBER LENGTH
- 13.4 QUALIFICATIONS
- 3.1 FABRICATORS DESIGN ENGINEER: A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK TO DESIGN THE STEEL JOIST SYSTEM AND TO PREPARE, SEAL AND SIGN SHOP DRAWINGS; SHOP DRAWINGS TO SHOW BOTH DESIGN AND INSTALLATION REQUIREMENTS.
- 3.2 INSTALLER: COMPANY SPECIALIZING IN INSTALLING STEEL JOIST SYSTEMS, WITH MINIMUM OF FIVE YEARS DOCUMENTED EXPERIENCE, AND APPROVED BY THE MANUFACTURER.

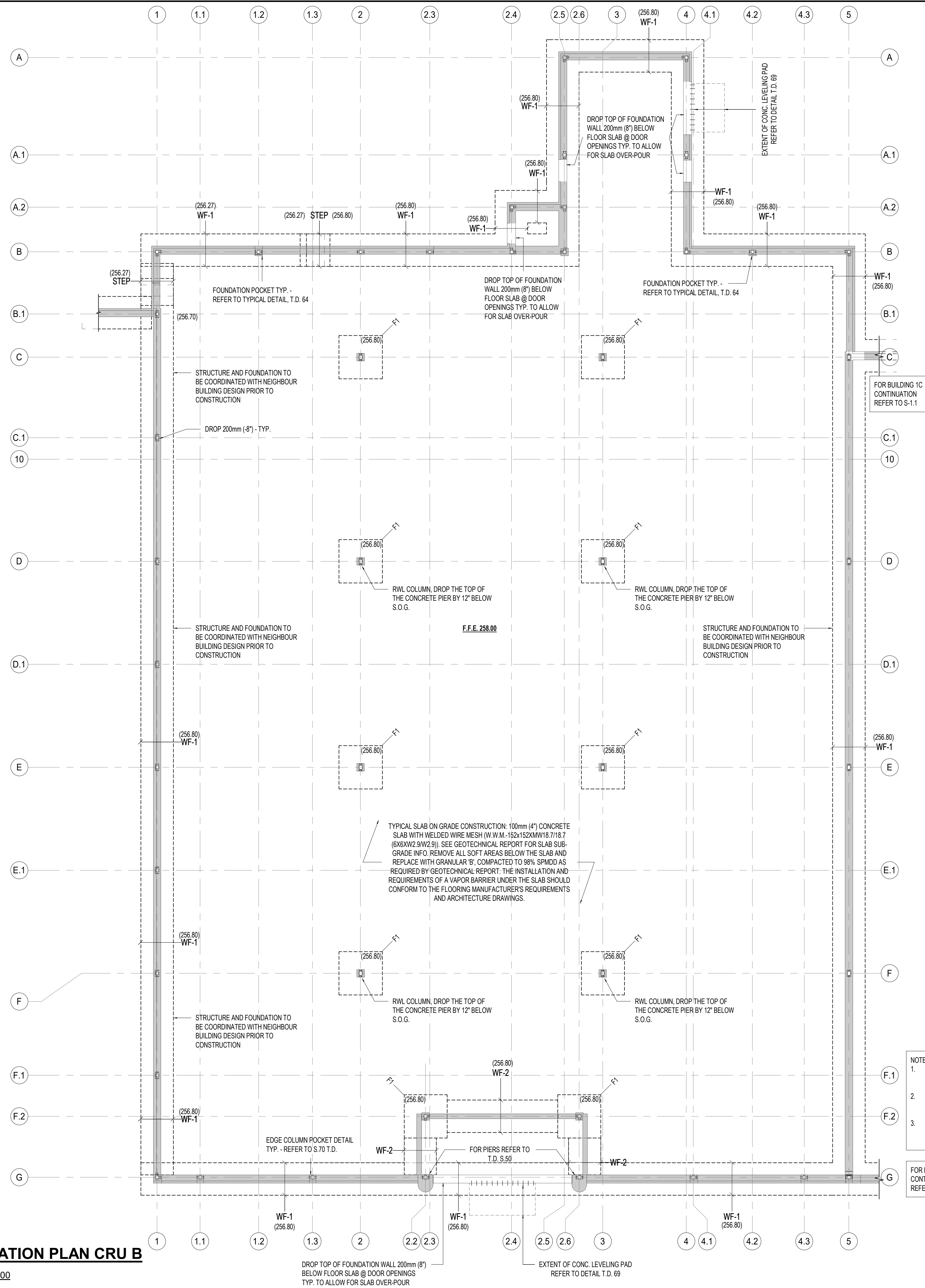
OPEN WEB STEEL JOISTS

1. DESIGN
- 1.1 BASE DESIGN ON LIMIT STATES DESIGN PRINCIPLES USING FACTORED LOADS AND RESISTANCES. DETERMINE RESISTANCES AND RESISTANCE FACTORS IN ACCORDANCE WITH THE MOST CURRENT ONTARIO BUILDING CODE AND CAN/CSA-S16.
- 1.2 FOR WIND LOAD CALCULATIONS, THE REFERENCE VELOCITY PRESSURE, Q, SHALL BE BASED ON A 1 IN 50 PROBABILITY OF BEING EXCEEDED IN ANY ONE YEAR.
- 1.3 UNLESS OTHERWISE NOTED, MAXIMUM FLEXURAL DEFLECTIONS UNDER SPECIFIED LIVE OR WIND LOADS SHALL CONFORM TO THE FOLLOWING:
- 1.3.1 ROOF JOIST DEFLECTIONS LIMITED TO L/240
- 1.3.2 FLOOR JOIST DEFLECTIONS LIMITED TO L/360
- 1.4 DESIGN BRIDGING TO PREVENT MEMBER ROTATION AND MEMBER TRANSLATION PERPENDICULAR TO THE MINOR AXIS.
- 1.5 DESIGN ANCHORAGE AND SPLICE DETAILS FOR BRIDGING.
- 1.6 DESIGN JOISTS TO CONSIDER LOAD EFFECTS DUE TO TRANSPORT, HANDLING, FABRICATION, AND ERECTION.
2. SHOP DRAWINGS
- 2.1 INDICATE MATERIAL SPECIFICATIONS, CONFIGURATION, MEMBER SIZES AND DIMENSIONS, SPACING, COATING TYPE, SHOE DEPTH, AND CAMBERS.
- 2.2 INDICATE DIMENSIONS, OPENINGS, REQUIREMENTS FOR RELATED WORK, AND CRITICAL INSTALLATION PROCEDURES. SHOW TEMPORARY BRACING REQUIRED FOR ERECTION PURPOSES.
- 2.3 INDICATE ATTACHMENTS, BRIDGING LOCATIONS AND CONNECTIONS.
- 2.4 INDICATE DESIGN LOADS AND FACTORED MEMBER LOADS.
- 2.5 EACH SHOP DRAWING SUBMITTED SHALL BEAR THE STAMP AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK.
3. QUALIFICATIONS
- 3.1 FABRICATORS DESIGN ENGINEER: A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK TO DESIGN THE STEEL JOIST SYSTEM AND TO PREPARE, SEAL AND SIGN SHOP DRAWINGS; SHOP DRAWINGS TO SHOW BOTH DESIGN AND INSTALLATION REQUIREMENTS.
- 3.2 INSTALLER: COMPANY SPECIALIZING IN INSTALLING STEEL JOIST SYSTEMS, WITH MINIMUM OF FIVE YEARS DOCUMENTED EXPERIENCE, AND APPROVED BY THE MANUFACTURER.

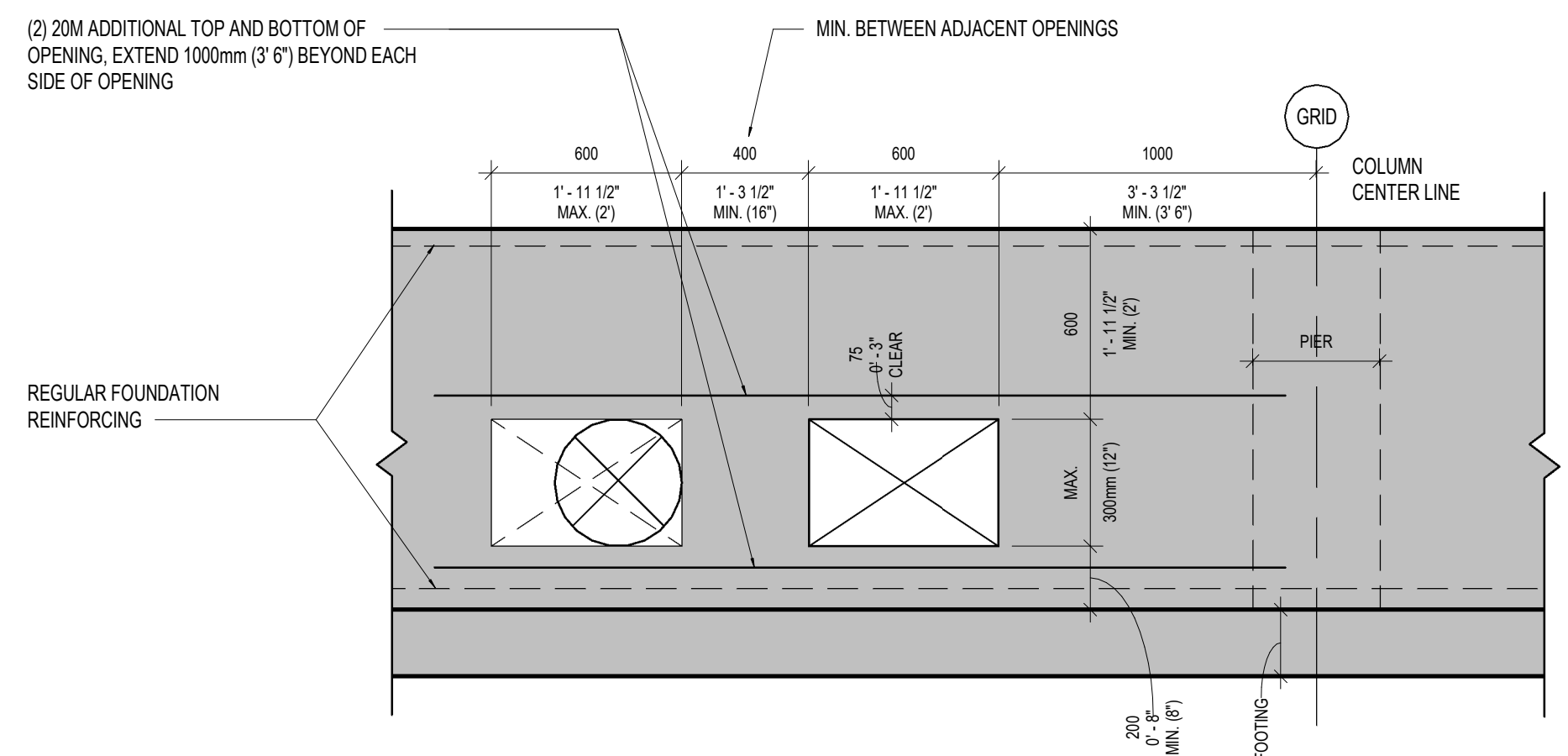
- 3.3 WELDERS: COMPANIES CERTIFIED BY THE CANADIAN WELDING BUREAU TO CAN/CSA W59, AND HAVING WELDERS QUALIFIED FOR THE BASE MATERIAL TYPES AND THICKNESSES THAT ARE TO BE WELDED.
4. DELIVERY, STORAGE, AND HANDLING
- 4.1 STORE PRODUCTS PROTECTED FROM CONDITIONS THAT MAY CAUSE PHYSICAL DAMAGE OR CORROSION.
- 4.2 HANDLE AND LIFT PREFABRICATED SECTIONS CAREFULLY TO AVOID PERMANENT DISTORTION TO ANY MEMBER OR COLLATERAL MATERIAL.
5. MATERIALS
- 5.1 STEEL: TO CAN/CSA S16 AND CAN/CSA-S136; IDENTIFIED ON SHOP DRAWINGS AS TO SPECIFICATION, GRADE, MECHANICAL PROPERTIES, COATING TYPE, AND THICKNESS.
- 5.2 BOLTS AND NUTS: TO ASTM A307 OR ASTM A325M; HOT-DIPPED GALVANIZED, CW WASHERS.
- 5.3 WELDING MATERIALS: TO CSA W59; TYPE REQUIRED FOR MATERIALS BEING WELDED.
6. FABRICATION
- 6.1 PROVIDE BOTTOM AND TOP CHORD EXTENSIONS AS INDICATED.
- 6.2 FABRICATE TO ACHIEVE END BEARING OF:
- 6.2.1 84 mm (2.5") ON STEEL.
- 6.2.2 100 mm (4") ON MASONRY.
- 6.3 FRAME SPECIAL SIZED OPENINGS IN JOIST WEB FRAMING AS DETAILED.
7. FINISHES
- 7.1 PREPARE JOIST COMPONENT SURFACES IN ACCORDANCE WITH SSPC SP 2.
- 7.2 SHOP PRIME JOISTS. DO NOT PRIME SURFACES THAT WILL BE FIREPROOFED, FIELD WELDED OR IN CONTACT WITH CONCRETE.
8. PREPARATION
- 8.1 VERIFY THAT SITE CONDITIONS ARE READY TO RECEIVE WORK AND FIELD MEASUREMENTS ARE AS INDICATED ON SHOP DRAWINGS.
- 8.2 VERIFY SUPPORTING STRUCTURE IS READY TO RECEIVE WORK.
- 8.3 PREPARE SUPPORT DEVICES FOR THE ERECTION PROCEDURE AND TEMPORARY BRACING.
9. ERECTION
- 9.1 ERECT JOIST MEMBERS TO CSA-S16
- 9.2 ERECT JOIST FRAMING TRUE AND PLUMB WITHIN THE SPECIFIED TOLERANCES.
- 9.3 ALLOW FOR ERECTION LOADS. PROVIDE TEMPORARY BRACING TO MAINTAIN FRAMING SAFE, PLUMB, AND IN TRUE ALIGNMENT.
- 9.4 COORDINATE PLACEMENT OF ANCHORS IN MASONRY AND CONCRETE CONSTRUCTION FOR SECURING BEARING PLATES AND ANGLES
- 9.5 AFTER JOIST ALIGNMENT AND INSTALLATION OF FRAMING, FIELD WELD JOIST SEAT TO BEARING PLATES OR ANGLES; WELD IN ACCORDANCE WITH CAN-CSA W59.
- 9.6 FRAME ROOF AND FLOOR OPENINGS GREATER THAN 450 mm (18") WITH SUPPLEMENTARY FRAMING AS DETAILED ON DRAWINGS.
- 9.7 DO NOT PERMIT ERECTION OF DECKING UNTIL JOISTS ARE BRACED AND SECURED, OR UNTIL INSTALLATION OF PERMANENT BRACING AND BRACING IS COMPLETE.
- 9.8 DO NOT FIELD CUT OR ALTER STRUCTURAL MEMBERS WITHOUT APPROVAL OF JOIST MANUFACTURER.
- 9.9 AFTER ERECTION, PRIME WELDS, ABRASIONS, AND SURFACES NOT SHOP PRIMED, EXCEPT SURFACES TO BE IN CONTACT WITH CONCRETE.
- 9.10 REPLACE DAMAGED MEMBERS TO SATISFACTION OF THE JOIST DESIGN ENGINEER.
10. TOLERANCES
- 10.1 MAXIMUM OFFSET FROM TRUE ALIGNMENT: 6 mm (1/4").

METAL FLOOR AND ROOF DECKING

1. DESIGN
- 1.1 BASE DESIGN ON LIMIT STATES DESIGN PRINCIPLES USING FACTORED LOADS AND RESISTANCES. DETERMINE REISTANCES AND RESISTANCE FACTORS IN ACCORDANCE WITH THE MOST CURRENT ONTARIO BUILDING CODE, CIBC 12M, AND CAN/CSA-S136.
- 1.2 FOR WIND LOAD CALCULATIONS, THE REFERENCE VELOCITY PRESSURE, Q, SHALL BE BASED ON A 1 IN 50 PROBABILITY OF BEING EXCEEDED IN ANY ONE YEAR.
- 1.3 MAXIMUM FLEXURAL DEFLECTIONS UNDER SPECIFIED LIVE OR WIND LOADS SHALL CONFORM TO THE FOLLOWING:
- 1.3.1 STEEL FLOOR DECK VERTICAL DEFLECTIONS LIMITED TO L/360
- 1.3.2 STEEL ROOF DECK VERTICAL DEFLECTIONS LIMITED TO L/240
- 1.3.3 LATERAL DEFLECTION OF DIAPHRAGM SHALL NOT EXCEED, 1/400 OF BUILDING HEIGHT OR 1/600 OF STOREY HEIGHT
2. SHOP DRAWINGS
- 2.1 PROVIDE DECK PROFILE, CHARACTERISTICS, DIMENSIONS, STRUCTURAL PROPERTIES, AND FINISHES.
- 2.2 INDICATE DECK PLAN, SUPPORT LOCATIONS, PROJECTIONS, OPENINGS, REINFORCEMENT, PERTINENT DETAILS, AND ACCESSORIES.
- 2.3 INDICATE DESIGN LOADS.
- 2.4 EACH SHOP DRAWING SUBMITTED SHALL BEAR THE STAMP AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK.
3. QUALIFICATIONS
- 3.1 FABRICATOR'S DESIGN ENGINEER: A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE OF THE WORK TO DESIGN THE STEEL DECK SYSTEM AND TO PREPARE, SEAL AND SIGN SHOP DRAWINGS; AND TO PERFORM FIELD REVIEW, SHOP DRAWINGS TO SHOW BOTH DESIGN AND INSTALLATION REQUIREMENTS.
- 3.2 INSTALLER: COMPANY SPECIALIZING IN INSTALLING METAL DECK SYSTEMS, WITH MINIMUM OF THREE YEARS DOCUMENTED EXPERIENCE, AND APPROVED BY THE MANUFACTURER.
- 3.3 WELDERS: COMPANIES CERTIFIED BY THE CANADIAN WELDING BUREAU TO CSA W59, AND HAVING WELDERS QUALIFIED FOR THE BASE MATERIAL TYPES AND THICKNESSES THAT ARE TO BE WELDED.
4. DELIVERY, STORAGE, AND HANDLING
- 4.1 STORE PRODUCTS PROTECTED FROM CONDITIONS THAT MAY CAUSE PHYSICAL DAMAGE OR CORROSION.
- 4.2 CUT PLASTIC WRAP TO ENCOURAGE VENTILATION.
- 4.3 STORE DECK ON DRY WOOD SLEEPERS, SLOPE FOR POSITIVE DRAINAGE.
- 4.4 HANDLE AND LIFT PREFABRICATED PANELS CAREFULLY TO AVOID PERMANENT DISTORTION OR COLLATERAL MATERIAL.
5. MATERIALS
- 5.1 SHEET STEEL: TO ASTM A653/A653M, STRUCTURAL QUALITY, IDENTIFIED ON SHOP DRAWINGS AS TO SPECIFICATION, GRADE, MECHANICAL PROPERTIES, COATING TYPE, AND THICKNESS.
- 5.2 BEARING ANGLES OR PLATES: ASTM A36/A36M STEEL, UNFINISHED.
- 5.3 SCREWS: SELF-TAPPING, TO ASTM C1513, SUITABLE FOR INTENDED EXPOSURE AND OF A MATERIAL FINISH TO PREVENT GALVANIC CORROSION WITH BASE MATERIALS.
- 5.4 WELDING MATERIALS: TO CSA W59, TYPE REQUIRED FOR MATERIALS BEING WELDED.
- 5.5 FLUTE CLOSURES: CLOSED CELL, PROFILED TO FIT TIGHT TO THE DECK.
6. FABRICATION
- 6.1 METAL DECK: CSSBI 10M, SHEET STEEL, CONFIGURED AS FOLLOWS:
- 6.1.1 SPAN DESIGN: TRIPLE, UNLESS NOTED OTHERWISE.
- 6.1.2 MINIMUM METAL THICKNESS EXCLUDING FINISH: 0.8MM/22 GAUGE.
- 6.1.3 NOMINAL HEIGHT: 38 mm (1.5") OR 75 mm (3") AS INDICATED ON DRAWINGS, FLUTED PROFILE.
- 6.1.4 FORMED SHEET WIDTH: 900 mm (36"), UNLESS NOTED OTHERWISE.
- 6.1.5 SLOPE: JOINTS: LAPPED, UNLESS NOTED OTHERWISE.
- 6.2 RELATED DECK ACCESSORIES: METAL CLOSURE STRIPS, WET CONCRETE STOPS, COVER PLATES, MINIMUM 0.8MM/22 GAUGE THICK.
- 6.3 CANT STRIPS: FORMED SHEET STEEL, MINIMUM 0.8MM/22 GAUGE, 45 DEGREE SLOPE, 3.5" (89mm) NOMINAL WIDTH AND HEIGHT, FLANGE FOR ATTACHMENT.
- 6.4 WELD HEADERS: MILD STEEL, UNCOATED, 19 mm (3/4") OUTSIDE DIAMETER, 3 mm (1/8") THICK.
7. INSTALLATION
- 7.1 ERECT METAL DECK TO MANUFACTURERS WRITTEN INSTRUCTIONS, CSSBI 10M, AND SDI MANUAL.
- 7.2 BEAR DECK ON MASONRY OR CONCRETE SUPPORT SURFACES WITH MINIMUM 100 mm (4") BEARING. ALIGN AND LEVEL.
- 7.3 BEAR DECK ON STEEL SUPPORTS WITH 19 mm (3/4") MINIMUM BEARING.
- 7.4 FASTEN DECK TO STEEL SUPPORT MEMBERS AT ENDS AND INTERMEDIATE SUPPORTS IN ACCORDANCE WITH DECK ATTACHMENT DETAILS PROVIDED ON DRAWINGS, UNLESS NOTED OTHERWISE. SPOT WELD DECK AT 300 mm (12") ON CENTRE MAX. PARALLEL WITH DECK FLUTES, AND AT EVERY OTHER TRANSVERSE FLUTE. WELD TO CSA- W59.
- 7.5 MECHANICALLY CLINCH MALE/FEMALE SIDE LAPS AT 600 mm (24") ON CENTRE MAXIMUM.
- 7.6 REINFORCE STEEL DECK OPENINGS FROM 150 mm (6") TO 450 mm (18") IN SIZE WITH 50X50X6 mm (2X2X1/4") STEEL. ANGLES: PLACE FRAMING ANGLES PERPENDICULAR TO FLUTES; EXTEND MINIMUM TWO FLUTES BEYOND EACH SIDE OF OPENING AND WELD TO DECK AT EACH FLUTE.
- 7.7 INSTALL 150MM MINIMUM WIDE SHEET STEEL COVER PLATES, OF SAME THICKNESS AS DECK, WHERE DECK CHANGES DIRECTION. WELD AT 300 mm (12") ON CENTRE MAXIMUM.
- 7.8 INSTALL SHEET STEEL CLOSURE AND ANGLE FLASHINGS TO CLOSE OPENINGS BETWEEN DECK AND WALLS, COLUMNS, AND OPENINGS.
- 7



FOUNDATION PLAN CRU B
SCALE: 1 : 100

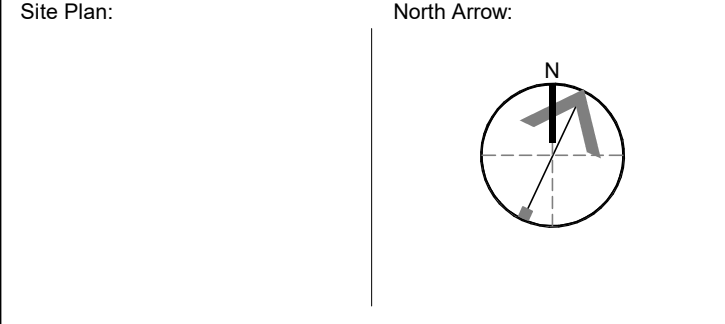
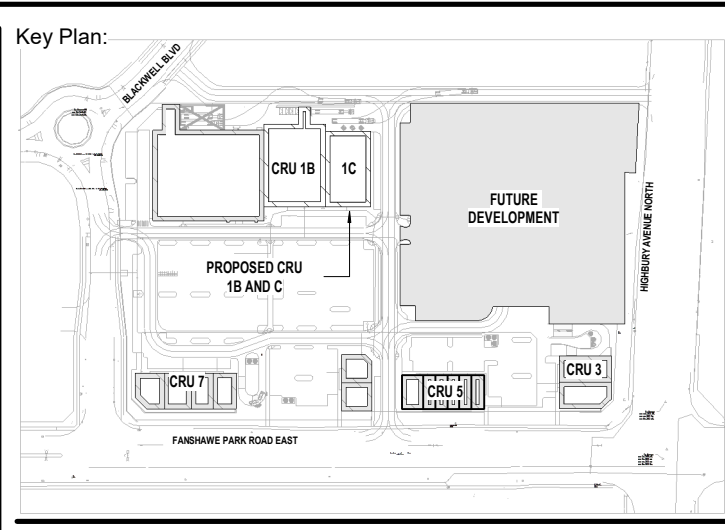


FOUNDATION WALL OPENING - TYPICAL
SCALE: NTS

FOUNDATION NOTES

- ALL FOOTINGS ARE TO BE FOUNDED AT AN ELEVATION OF MIN. 1200mm (4') BELOW FINISHED EXTERIOR GRADE AND BELOW FINISHED MAIN FLOOR ELEVATION. AS PER CIVIL DRAWINGS STAMPED AND DATED FEBRUARY 27, 2024. THE F.F.E. IS 258.00 REFER TO PLAN. THE RECOMMENDED LEVEL OF THE UNDERSIDE OF FOUNDATIONS ARE SHOWN ON PLAN ().
- G.C. TO CONFIRM THE UNDERSIDE OF FOUNDATIONS BASED ON THE LATEST GEOTECHNICAL REPORT RECOMMENDATIONS AND REPORT TO IE DESIGN IF ANY DO NOT MEET THE MINIMUM FROST DEPTH RECOMMENDED BY GEOTECHNICAL ENGINEER.
- GEOTECHNICAL INVESTIGATION REPORT #2392 BY GSPRIMO DESIGN INC. DATE: JANUARY 12, 2024. INDICATE SPREAD FOOTINGS. ULTIMATE LIMIT STATE 345kPa (7205 psf), FOR A SERVICEABILITY LIMIT STATES (SLS) GEOTECHNICAL BEARING RESISTANCE OF 190kPa (3958 psf). FOR STRIP FOOTINGS ARE CONSIDERING A ULTIMATE LIMIT STATES (ULS) GEOTECHNICAL BEARING RESISTANCE OF 260kPa (5430 psf). FOR A SERVICEABILITY LIMIT STATES (SLS) GEOTECHNICAL BEARING RESISTANCE OF 145kPa (3028 psf) - GEOTECHNICAL ENGINEER TO CONFIRM PRIOR CONSTRUCTION.
- FOUNDATION LEVELS ARE ON NATIVE SOIL AND AS PROVIDED BY THE GEOTECHNICAL REPORT. VERIFICATION OF SOIL CONDITIONS AND THE REQUIREMENT FOR CONCRETE REINFORCING STEEL PLACEMENT ARE BY GEOTECHNICAL ENGINEER AT THE TIME OF EXCAVATION. REMOVE ALL SOFT AREAS BELOW THE FOOTINGS AND REPLACE WITH CONCRETE GROUT SOLID (MIN. FC=20 MPa) BUT NOT LESS THE COMPRESSIVE STRENGTH OF FOOTINGS AND TO BE REVIEWED AND CONFIRMED BY GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION.
- ALL GROUND OR GRADE OR PARKING LEVEL CONCRETE WALLS, COLUMNS, AND PIERS EXTEND DOWN TO FOOTINGS.
- TYPICAL SLAB ON GRADE CONSTRUCTION: 100mm (4") CONCRETE SLAB WITH WELDED WIRE MESH (W.W.M.-152x152XMM18.7/18.7 (6X6XW2.9W2.9)). SEE GEOTECHNICAL REPORT FOR SLAB SUB-GRADE INFO. REMOVE ALL SOFT AREAS BELOW THE SLAB AND REPLACE WITH GRANULAR 'B' COMPACTED TO 98% SPMD AS REQUIRED BY GEOTECHNICAL REPORT. THE INSTALLATION AND REQUIREMENTS OF A VAPOR BARRIER UNDER THE SLAB SHOULD CONFORM TO THE FLOORING MANUFACTURER'S REQUIREMENTS AND ARCHITECTURE DRAWINGS.
- PROVIDE CONCRETE CONTROL JOINTS IN CONCRETE SLAB AT 4.5mX4.5m (14' 5"X14' 5") INTERVALS OR BETTER OR AS SHOWN IN TYPICAL DETAILS. COORDINATE WITH THE TILE INSTALLER IF APPLICABLE. SEE TYPICAL DETAILS FOR DEPTH AND CONSTRUCTION PRACTICE.
- ALL DIMENSIONS AND LEVELS ARE APPROXIMATE AND SHOULD BE CONFIRMED BY OWNER AND ARCHITECT. REFER TO ARCHITECTURAL DRAWINGS FOR DEPRESSIONS AND FLOORING AND SLOPES TO DRAIN - MAINTAIN STRUCTURAL THICKNESS. SLOPE PARKING FLOOR AND SLAB AS REQUIRED BY ARCHITECTURAL DRAWINGS FOR DRAINAGE - MAINTAIN STRUCTURAL THICKNESS.
- CONSTRUCTION SEQUENCE ARE TO BE DISCUSSED WITH IE DESIGN DURING THE DESIGN STAGE WITH TYPICAL SIMILAR HEIGHTS STRUCTURES BUILT AT SAME TIME. ANY CHANGES TO CONSTRUCTION SEQUENCE DURING CONSTRUCTION ARE TO BE DISCUSSED AND APPROVED BY IE DESIGN.
- IN THE EVENT THE SITE SERVICES ENTER THE BUILDING BELOW THE FOOTING DEPTHS SHOWN ON THE DRAWING, THE CONTRACTOR MUST STEP THE WALL AND FOOTING (IN 600mm (2') HIGH INCREMENTS AS PER TYPICAL DETAILS) TO ENSURE THE SERVICES ENTER THROUGH THE FOUNDATION WALL. CONCRETE CONTRACTOR AND G.C. TO COORDINATE WITH ALL TRADES THE LOCATION OF ALL PIPES SLEEVES THROUGH CONCRETE FOUNDATION WALLS. PIPE SLEEVES MAY NOT BE PLACED WITHIN FOOTINGS. REPORT ANY DISCREPANCIES TO THE STRUCTURAL CONSULTANT BEFORE PROCEEDING WITH CONSTRUCTION.
- SEE ARCHITECTURAL DRAWINGS FOR SLOPES TO DRAINS IN FLOOR AREAS. MAINTAIN ALL STRUCTURAL THICKNESS SHOWN.
- CENTER ALL CONCRETE PIERS UNDER STEEL COLUMN BASE PLATES U.N.O. SEE ARCHITECTURAL DRAWINGS FOR COLUMN OFFSETS FROM GRID LINES.
- ELEVATION OF THE UNDERGROUND WATER TABLE IS AS PER GEOTECHNICAL REPORT. BUILDING FOUNDATIONS AND SLAB-ON-GRADE HAVE NOT BEEN DESIGNED FOR HYDROSTATIC PRESSURE AND UPLIFT FORCES.
- FOUNDATIONS AND FOOTINGS HAVE NOT BEEN DESIGNED BY CRANE LOADING - CONTACT IE DESIGN IF CRANE LOADING IS TO BE INCORPORATED. LOCATION OF CRANE TO BE DETERMINED BY G.C./ARCHITECT/OWNER.
- DO NOT PLACE BACKFILL AGAINST WALLS RETAINING EARTH UNTIL FLOORS AT TOP AND BOTTOM OF WALLS ARE PLACED AND HAVE ATTAINED SPECIFIED DESIGN STRENGTH CRITERIA U.N.O. FILL REQUIRED ON SIDES OF FOUNDATION WALL SHALL BE PLACED AND COMPACTED SIMULTANEOUSLY ON BOTH SIDES TO EQUALIZE SOIL PRESSURE.
- REFER TO GEOTECHNICAL REPORT FOR RECOMMENDED FOUNDATION FROST PROTECTION, FOUNDATION FROST DEPTH, AND BACKFILL - WHERE FOUNDATION NEED TO BE RAISED OR WHERE FOUNDATIONS FOR HEATED BUILDING DO NOT HAVE THE MINIMUM REQUIRED FROST DEPTH OF SOIL COVER FROST PROTECTION. FOOTINGS SHOULD BE PROTECTED FROM FROST WITH A COMBINATION OF SOIL COVER AND RIGID POLYSTYRENE INSULATION, SUCH AS DOW STYROFOAM OR APPROVED EQUIVALENT PRODUCT - TO BE REVIEWED AND APPROVED BY GEOTECHNICAL AND STRUCTURAL ENGINEER PRIOR TENDER AND CONSTRUCTION.

NOTE:
1. F.F.E. TO BE CONFIRMED BY ARCH. AND CIVIL DRAWINGS AND REPORT TO IE DESIGN IF DIFFERENT - TYPICAL
2. TOP OF INTERIOR PIERS AND FOUNDATION WALLS TO BE SET 200mm (8") BELOW UNDERSIDE OF S.O.G. - TYPICAL
3. STRUCTURE AND FOUNDATION TO BE COORDINATED WITH NEIGHBOUR BUILDING DESIGN PRIOR CONSTRUCTION.



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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 23 2024	ISSUED FOR COORDINATION

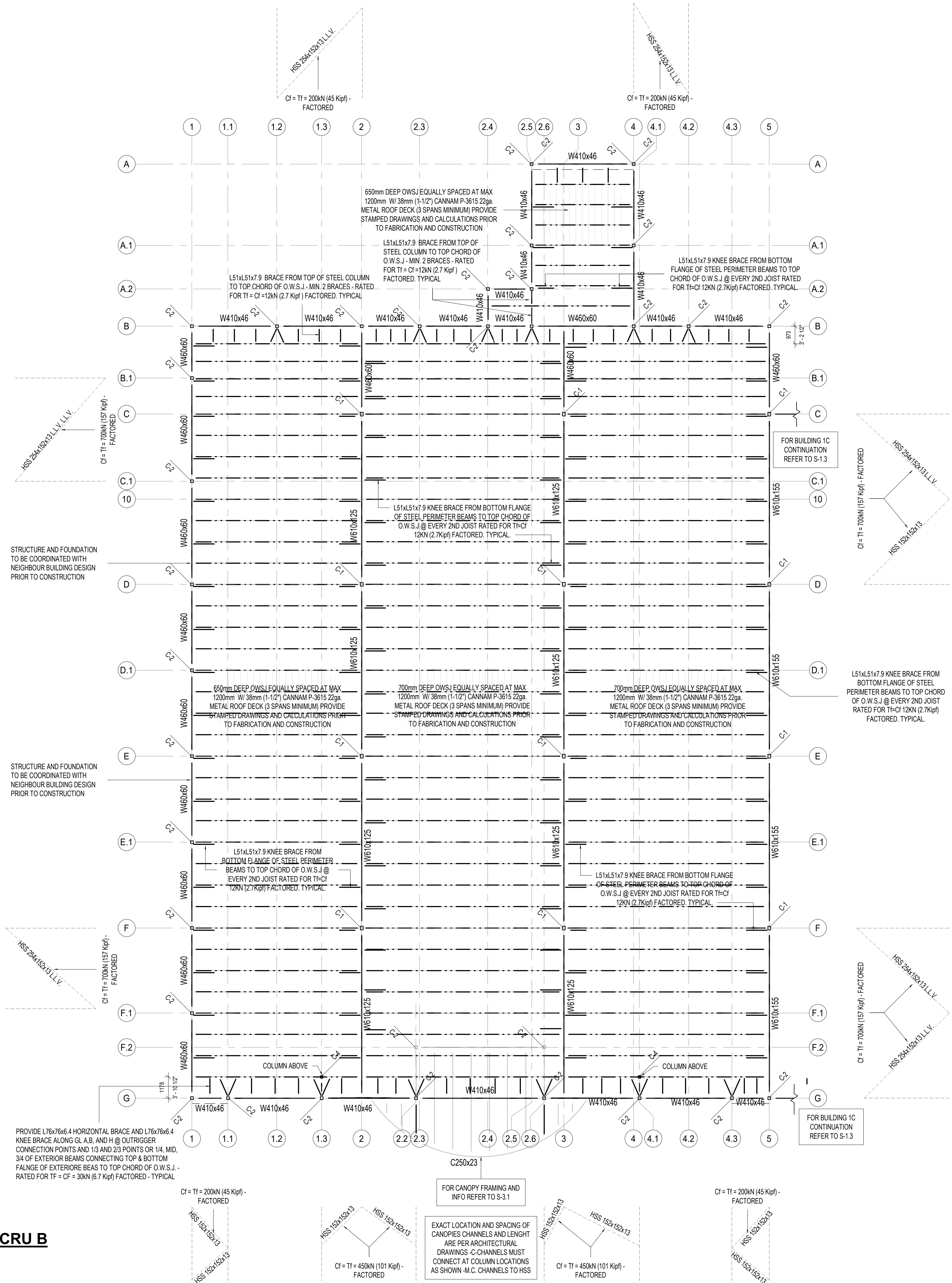
ISSUES/REVISION TABLE

Project:
WESTDELL
DEVELOPMENT CORP
1300 FANSHAWE PARK RD. EAST. - CRU #1B AND C
1300 FANSHAWE PARK RD. EAST. LONDON, ON

Drawing Title:
FOUNDATION PLAN - CRU 1B

Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102

Drawing No: **S-1.0** Revision: **2**

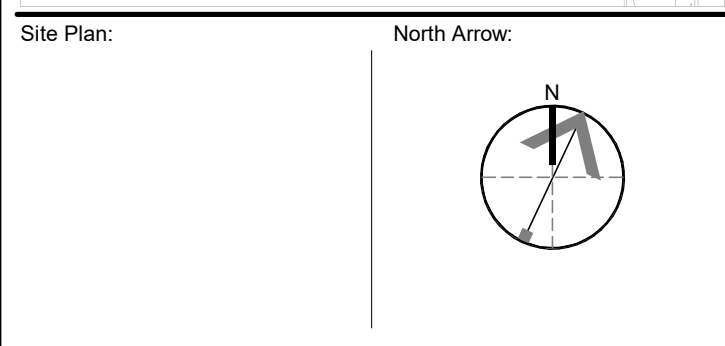
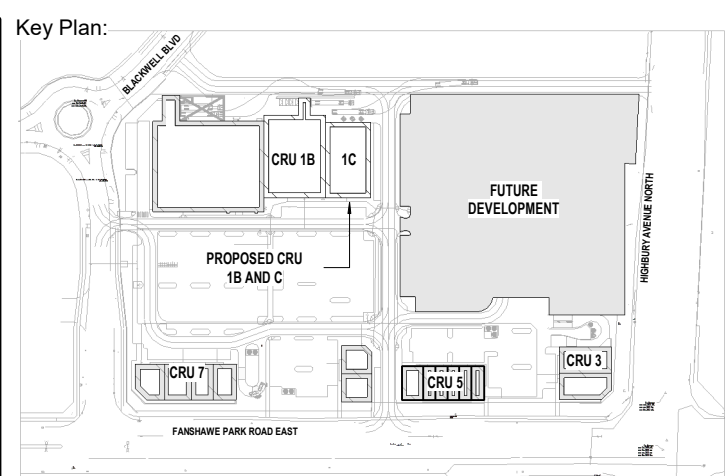


ROOF PLAN CRU B

SCALE: 1 : 120

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1	Feb 23 2024	ISSUED FOR COORDINATION

ISSUES/REVISION TABLE

Project:

WESTDELL
DEVELOPMENT CORP

**1300 FANSHAWE PARK RD.
EAST. - CRU #1B AND C**

1300 FANSHAWE PARK RD. EAST. LONDON, ON

Drawing Title:

**ROOF FRAMING PLAN -
CRU 1B**

Drawn By: **D.H./S.D./D.K.** Scale: **AS INDICATED**

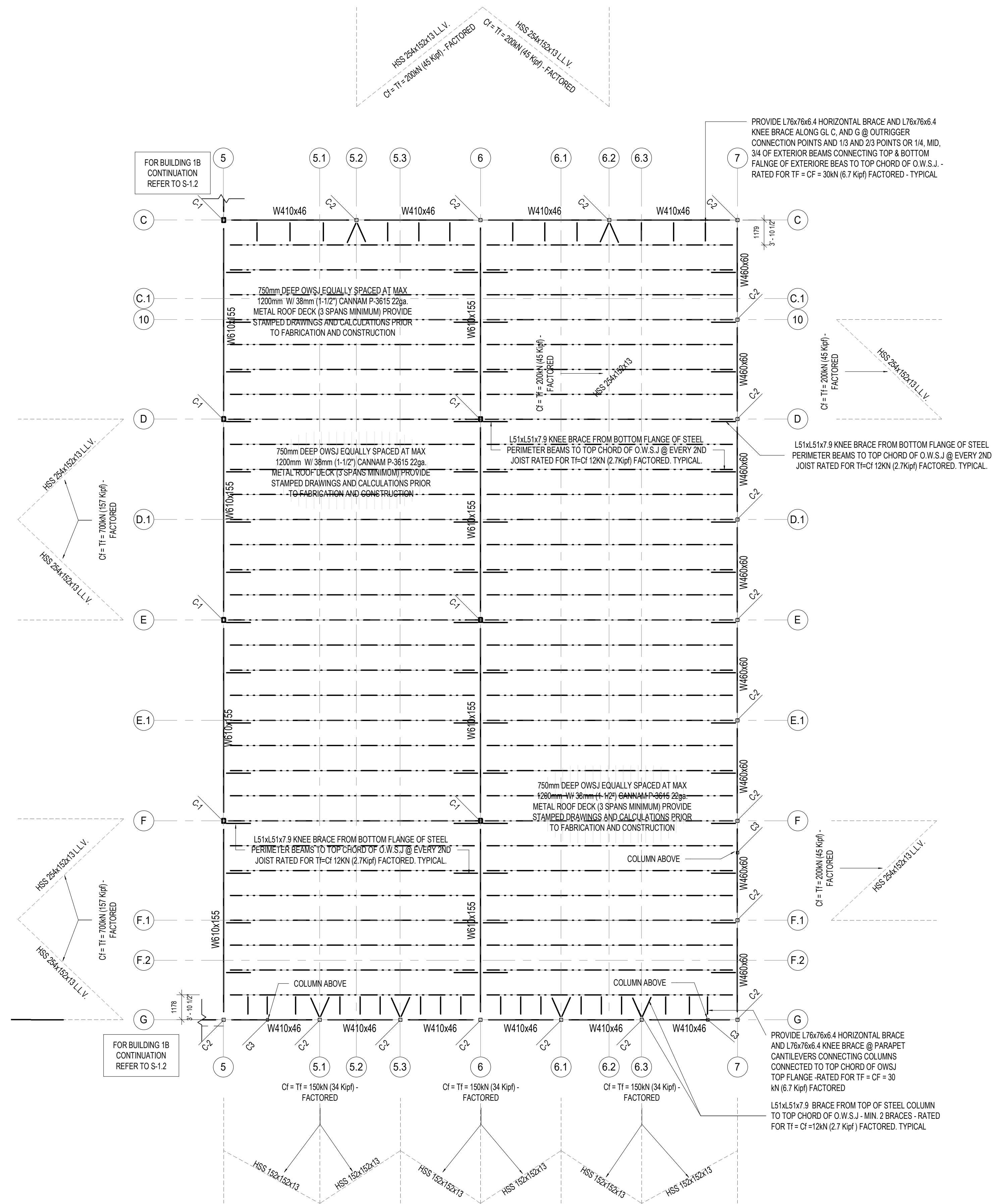
Checked By: **M.A.F./J.G.** Plot Date: **APR. 03-2024**

Project Date: **AUG. 2023**

Project No: **2023-102**

Drawing No: _____ Revision _____

S-1.2 **2**



ROOF PLAN CRU C

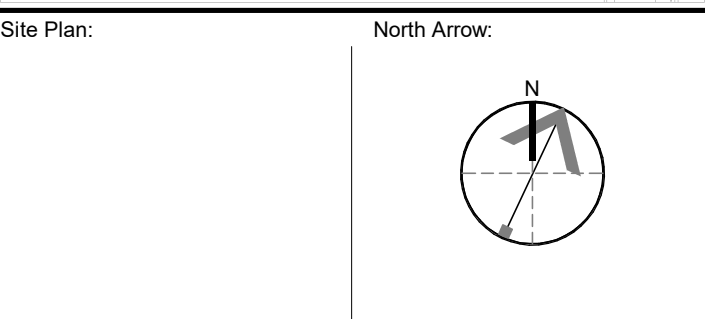
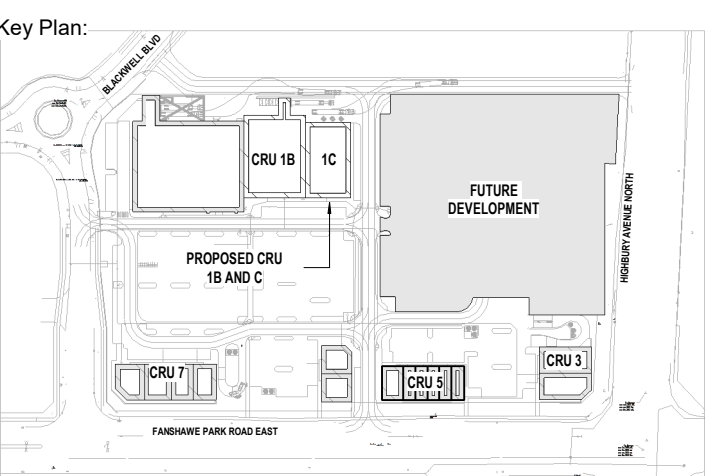
SCALE: 1 : 120

KEYNOTE LEGEND

M.C. ◀ = FLEXURAL MOMENT
 35 kN.m (7.87 kip-ft.) FACTORED U.N.O.
 TORSIONAL MOMENT
 20kN.m (15 kip-ft) FACTORED U.N.O.

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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER

ISSUES/REVISION TABLE

Project:

WESTOELL
 DEVELOPMENT CORP

1300 FANSHAWE PARK RD. EAST. - CRU #1B AND C

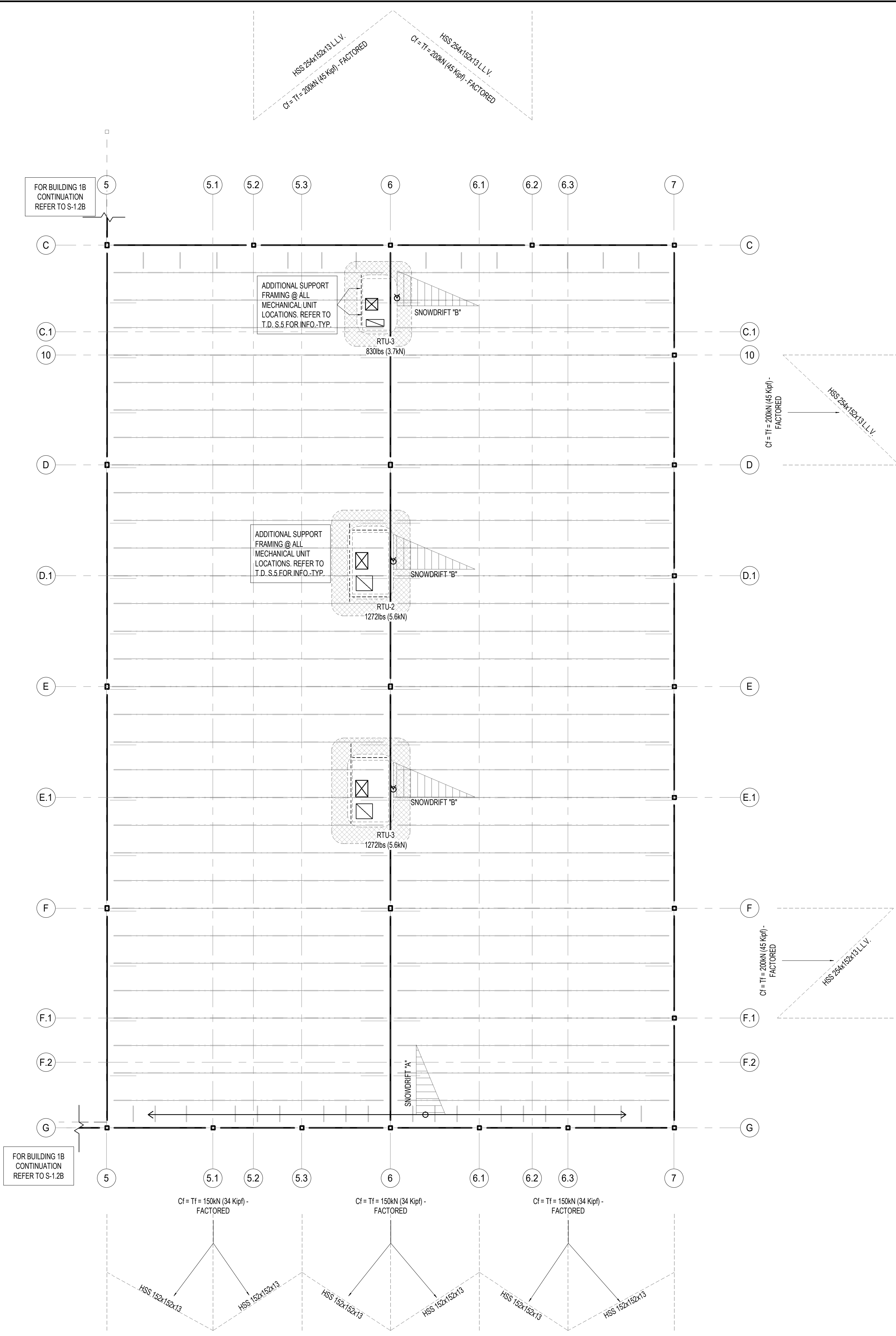
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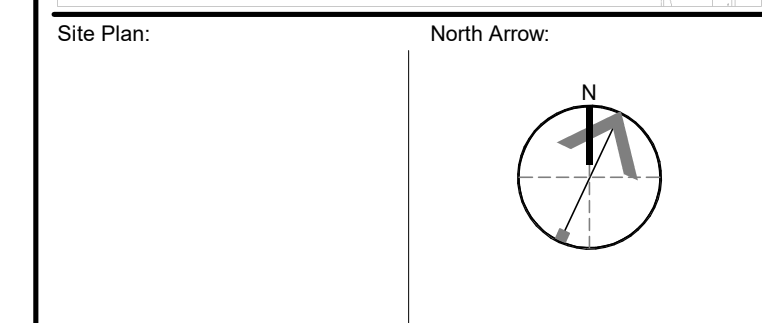
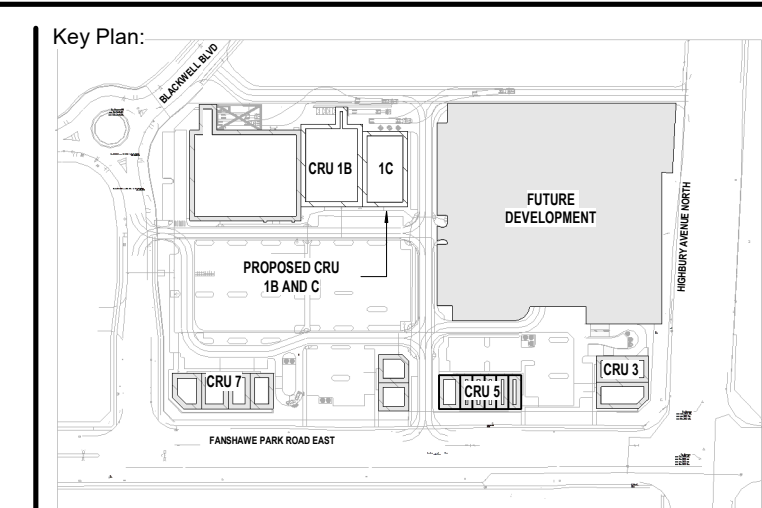
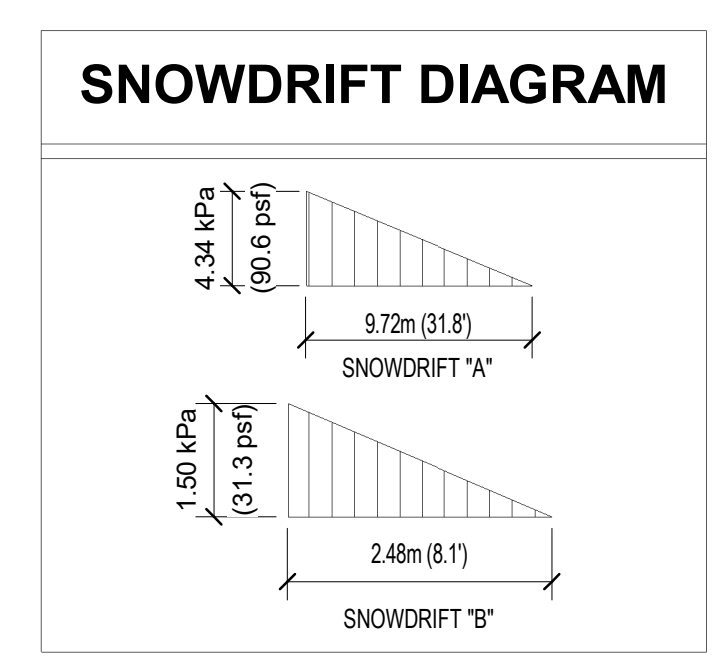
ROOF FRAMING PLAN - CRU 1C

Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
 Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
 Project Date: AUG. 2023
 Project No: 2023-102
 Drawing No: _____ Revision

S-1.3 **2**



SNOW DRIFT AND MECHANICAL CRU C
SCALE: 1 : 100



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2	Apr 03 2024	ISSUED FOR TENDER

ISSUES/REVISION TABLE

Project:

WESTDELL
DEVELOPMENT CORP

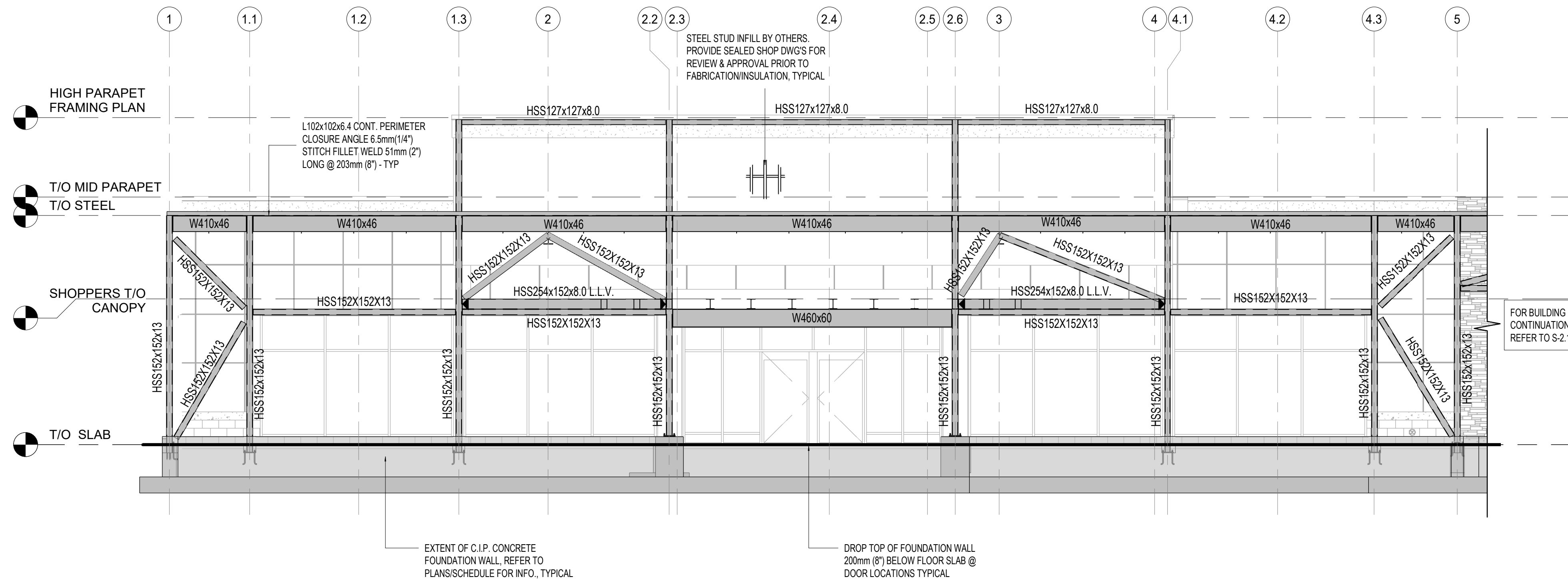
1300 FANSHAWE PARK RD. EAST. - CRU #1B AND C
1300 FANSHAWE PARK RD. EAST. LONDON, ON

Drawing Title:

SNOWDRIFT AND MECHANICAL PLAN - CRU 1C

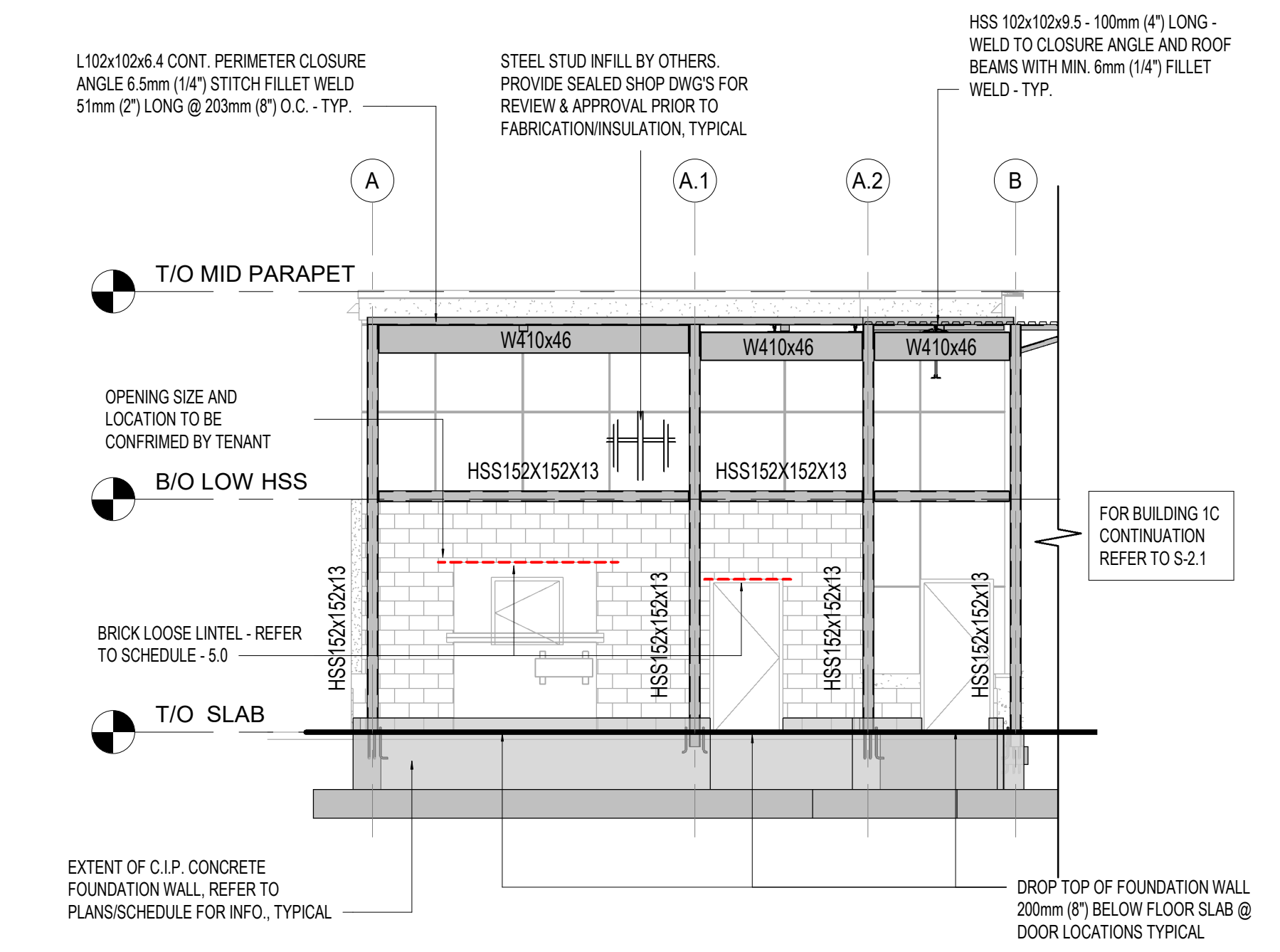
Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102

Drawing No: **S-1.5** Revision: **2**



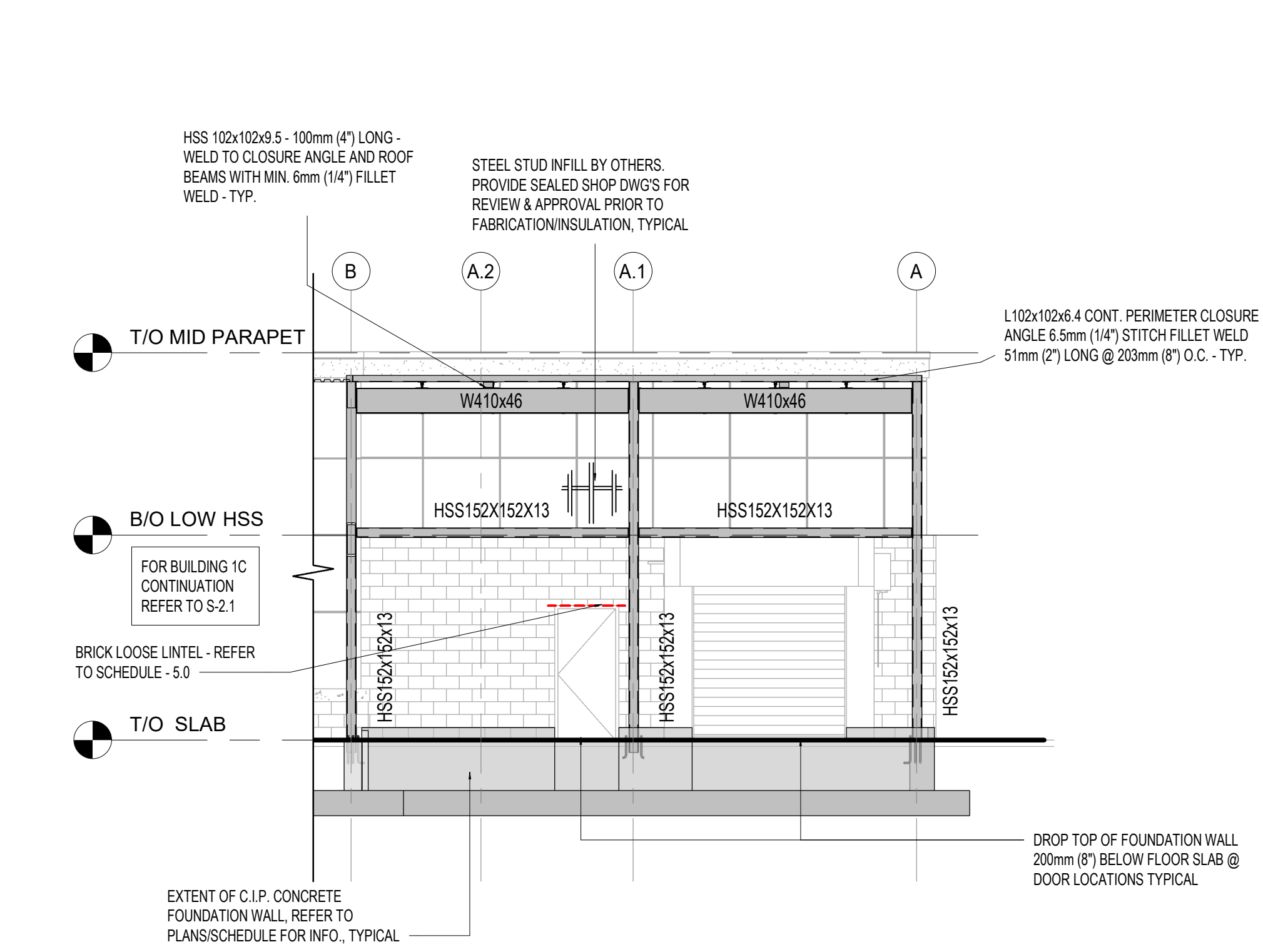
SOUTH ELEVATION-B

SCALE: 1 : 75



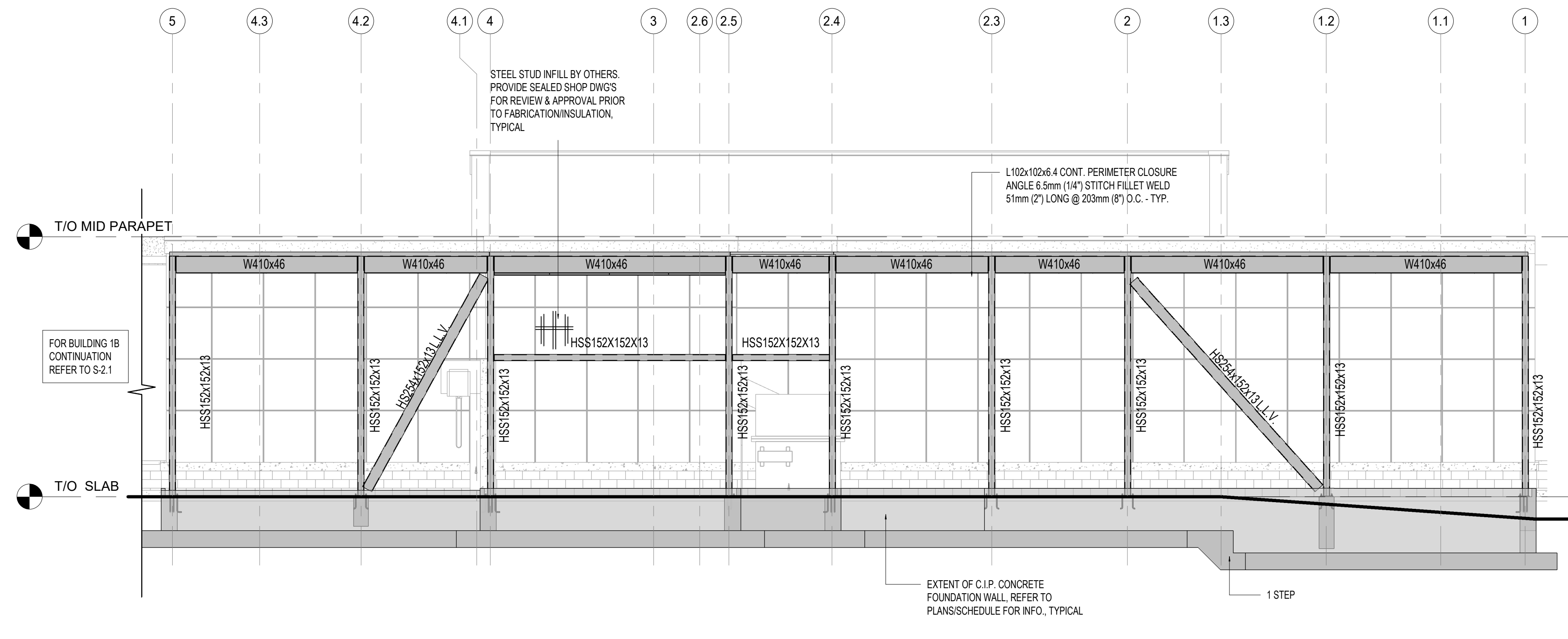
PARTIAL WEST ELEVATION-B

SCALE: 1 : 75



PARTIAL EAST ELEVATION-B

SCALE: 1 : 75



NORTH ELEVATION-B

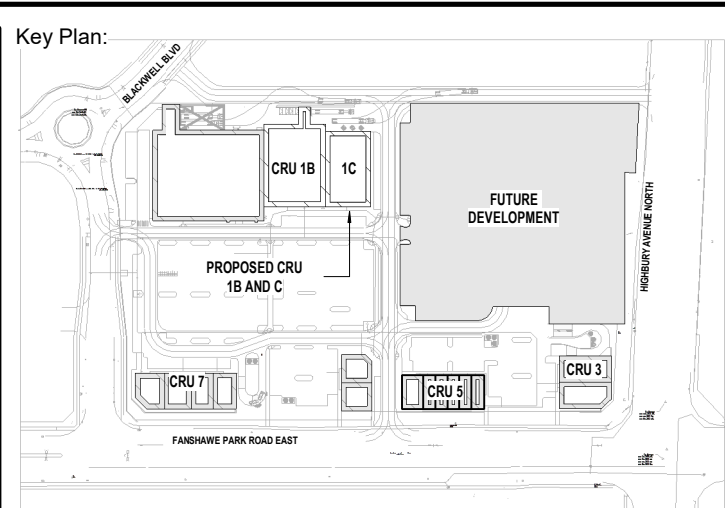
SCALE: 1 : 75

KEYNOTE LEGEND

- M.C. ◀ MOMENT CONNECT
- 35 kN.m (7.87 kip-ft) FACTORED U.O.
- TORSION MOMENT
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Site Plan: North Arrow:

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No.	Date	Revision

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DEVELOPMENT CORP

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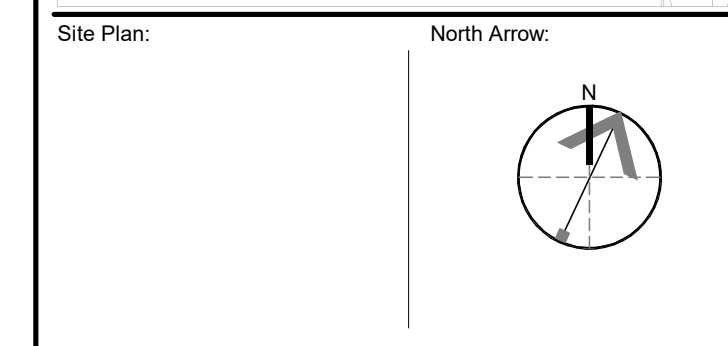
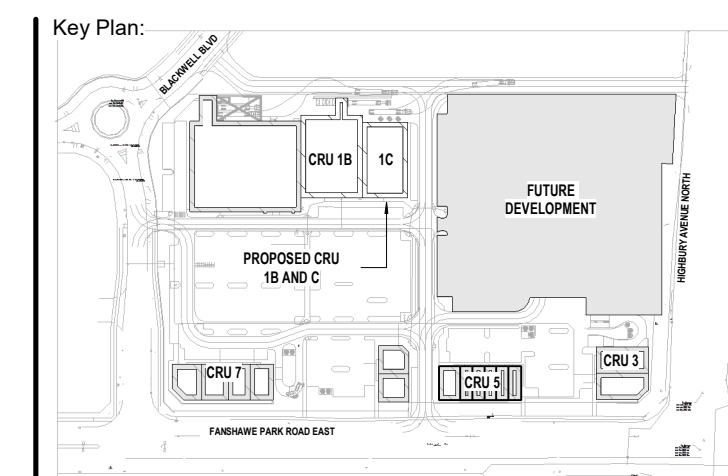
1300 FANSHAWE PARK RD. EAST. LONDON, ON

Drawing Title:

ELEVATIONS-CRU 1B

Drawn By:	D.H./S.D./D.K.	Scale:	AS INDICATED
Checked By:	M.A.F./J.G.	Plot Date:	APR. 03-2024
Project Date:	AUG. 2023		
Project No:	2023-102		
Drawn No:		Revision	

S-2.0 **2**



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No.	Date	Revision
2	Apr 03-2024	ISSUED FOR TENDER

ISSUES/REVISION TABLE

Project:

WESTDELL
DEVELOPMENT CORP

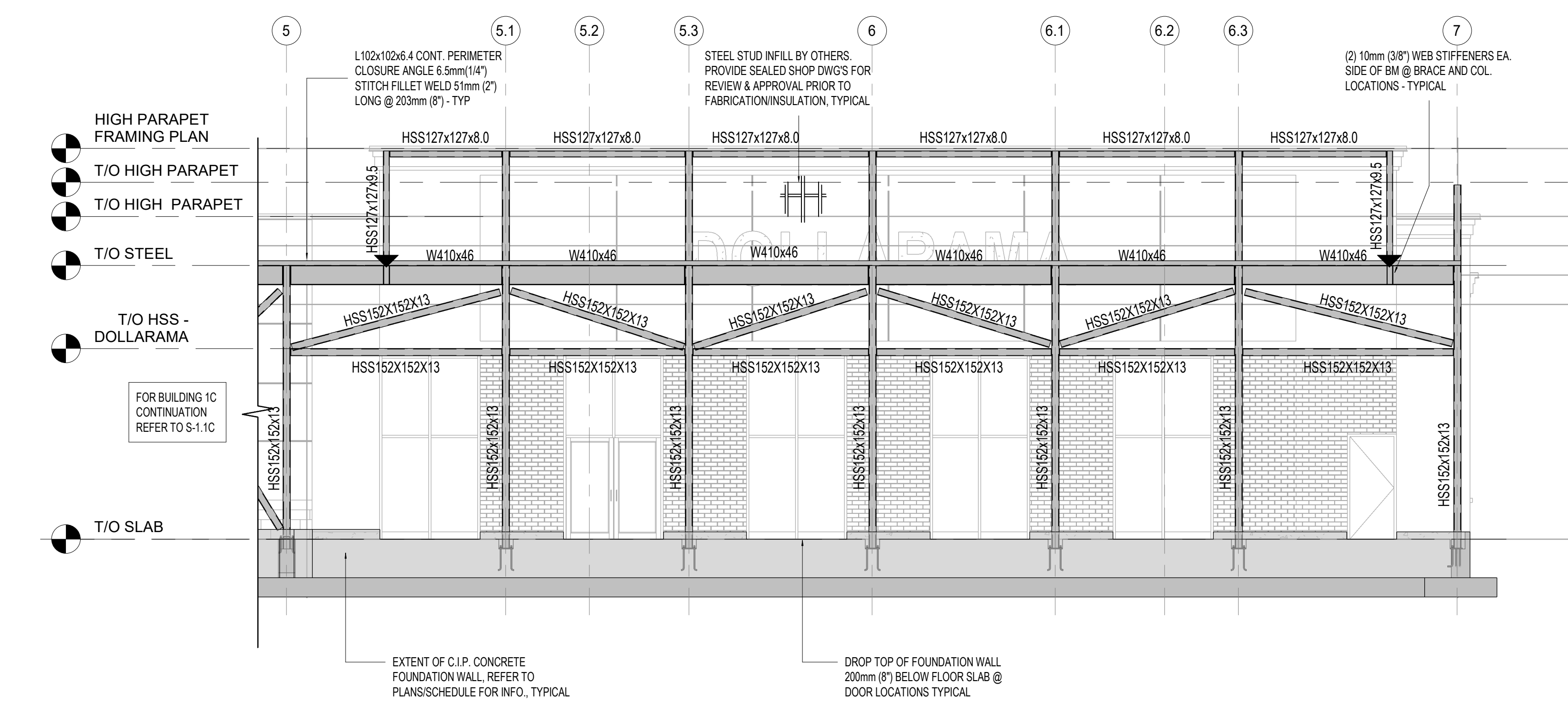
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1300 FANSHAWE PARK RD. EAST. LONDON, ON

Drawing Title:

ELEVATIONS-CRU 1C

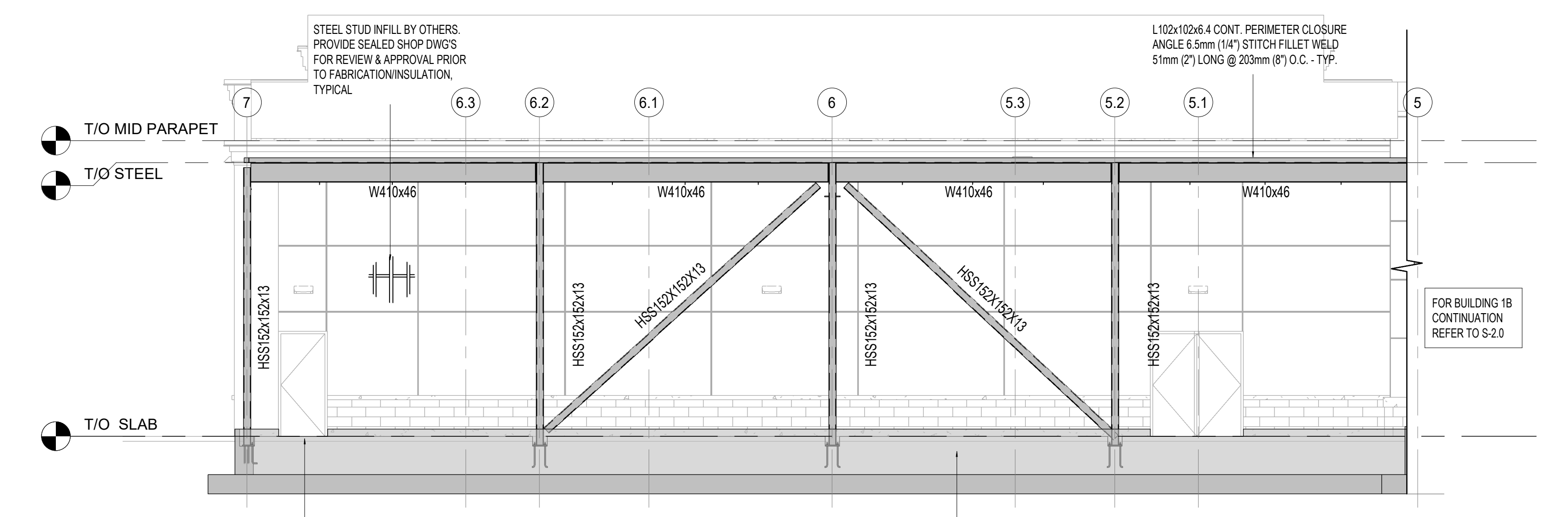
Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102

Drawing No: **S-2.1** Revision: **2**



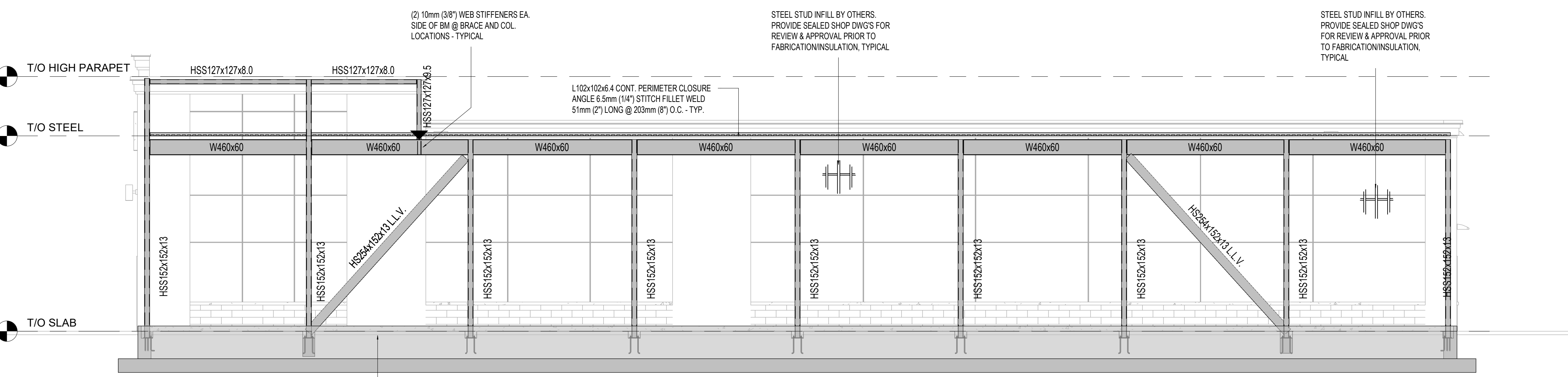
SOUTH ELEVATION-C

SCALE: 1 : 75



NORTH ELEVATION-C

SCALE: 1 : 75



EAST ELEVATION-C

SCALE: 1 : 75

KEYNOTE LEGEND

M.C. ◀= MOMENT CONNECT
20 kN.m (15 kip-ft) FACTORED U.N.O.
TORSIONAL MOMENT
10kN.m (7.5 kip-ft) FACTORED U.N.O.

LOADING INFORMATION

ROOF LOADING:

LIVE LOAD: SNOW 1.92 kPa (40.10 psf) (BASIC) + DRIFT
 WATER RETENTION 1.00 kPa (21 psf 100mm OF WATER)
 (WATER RETENTION NOT TO ACT SIMULTANEOUSLY WITH SNOW AS PER O.B.C. 4.1.6.4(3))

ROOF LIVE LOAD 1.0 kPa (21 psf) + MECHANICAL UNITS

DEAD LOAD: SELF-WEIGHT VARIES kPa
 METAL DECK 0.15 kPa (3 psf)
 STEEL BEAMS (ROOF) 0.35 kPa (7 psf)
 MECH/ELEC/PLUMBING 0.25 kPa (5 psf)
 SUPERIMPOSED / ROOF FINISH 0.50 kPa (10 psf)
 CEILING / FINISH 0.25 kPa (5 psf)

WIND LOADING: q(1/50) 0.47 kPa (9.81 psf)
 OPEN TERRAIN
 TOPOGRAPHIC FACTOR 1
 TOTAL UPLIFT ON ROOF 1.04 kPa (21.7 psf) (INTERMEDIATE)
 1.31 kPa (27.4 psf) (EDGES)
 2.54 kPa (53 psf) (CORNERS)
 CLADDING/PARAPET WIND 1.01 kPa (21 psf) INTERMEDIATE
 1.21 kPa (25 psf) EDGES

THE CLASSIFICATION OF THIS BUILDING IS ASSUMED TO BE NORMAL AN IMPORTANCE COEFFICIENT OF 1.0 WAS USED IN THE DESIGN UNDER SNOW AND WIND LOADS.

PRE-ENGINEERED ROOF/FLOOR JOIST/CLADDING SUPPLIER TO PROVIDE LOADING INFORMATION, DESIGN, SUPPLY, AND INSTALL ALL REQUIRED UPLIFT ANCHORS - PROVIDE STAMPED DRAWINGS.

SEISMIC LOADING:

AS PER OBC 2012 PART 4 (4.1.8.7) THE EQUIVALENT STATIC FORCE PROCEDURE CAN BE USED.

SOIL CLASS	D
BRACED FRAMES SFRS - CONVENTIONAL	
PGA	0.064
Sa (0.2)	0.108
Sa (0.5)	0.070
Sa (1.0)	0.041
Sa (2.0)	0.021
Sa (5.0)	0.0052
Sa (10.0)	0.0021
Rd	1.5
Ro	1.3
F(0.2) = Fa	1.24
F(0.5)	1.47
F(1.0) = Fv	1.55
F(2.0)	1.57
F(5.0)	1.58
F(10.0)	1.49
I _f , S _w (0.2)	0.134 < 0.35
T _x =	0.244 SEC
T _y =	0.244 SEC

THE CLASSIFICATION OF THIS BUILDING IS ASSUMED TO BE NORMAL AN IMPORTANCE COEFFICIENT OF 1.0 WAS USED IN THE DESIGN UNDER SEISMIC LOADS.

REQUIRED SUBMITTALS

1. THE FOLLOWING ITEMS REQUIRE TESTING OR INSPECTION BY A CERTIFIED INDEPENDENT TESTING OR INSPECTION AGENCY UNLESS NOTED OTHERWISE. THE AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS TO THE ENGINEER FOR REVIEW.

ITEM	REQUIRED	COMMENTS
SOIL BEARING CAPACITY	YES	BY SOILS ENGINEER
SOIL COMPACTION	YES	BY SOILS ENGINEER
ENGINEERED FILL	YES	BY SOILS ENGINEER
REINFORCING STEEL PLACEMENT	YES	INSPECT FINAL PLACEMENT
CONCRETE COMPRESSIVE TESTS	YES	MIN 3 SETS/100m ³
CONCRETE SLUMP	YES	MIN 3 SETS/100m ³
STRUCTURAL STEEL BOLTING	YES	INSPECT ALL BOLTS
STRUCTURAL STEEL WELDING	YES	INSPECT ALL FIELD WELDS

2. THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SUBMIT ONE DIGITAL COPY UNLESS NOTED OTHERWISE

ITEM	REQUIRED SUBMITTAL	P. ENG. STAMP REQUIRED
REBAR SHOP DRAWINGS	YES	NO
CONCRETE MIX DESIGNS	YES	YES
STRUCTURAL STEEL SHOP DRAWINGS	YES	YES (CONNECTIONS)
JOIST/TRUSS SHOP DRAWINGS	YES	YES
JOIST/HAMBRO SHOP DRAWINGS	YES	YES
MISC. STEEL HAND/GUARD RAILS/LADDER	YES	YES
PRECAST STAIRS	YES	YES
STAIRS	YES	YES
EXTERIOR DOORS/WINDOWS/GLASS/CLADDING	YES	YES
GLASS OVER CANOPY	YES	YES
STEEL SERVICE PLATFORM	YES	YES
PRECAST ERECTION AND SHOP DRAWINGS	YES	YES
STEEL STUD SHOP DRAWINGS	YES	YES
ACCESS LADDER	YES	YES

EXTERIOR BRICK LOOSE LINTEL SCHEDULE

WALL TYPE	MAX MASONRY R/O	MATERIAL	COMMENTS
102mm BRICK	150mm TO 900mm	L8x8x8x6.4 HOT DIP GALV.	SEE NOTES
102mm BRICK	900mm TO 1372mm	L127x89x6.4 L.L.V. HOT DIP GALV.	SEE NOTES
102mm BRICK	1397mm TO 2286mm	L152x102x9.5 L.L.V. HOT DIP GALV.	SEE NOTES
102mm BRICK	2311mm TO 3048mm	L203x102x11 L.L.V. HOT DIP GALV.	SEE NOTES
102mm BRICK	3048mm TO 3500mm	L203x102x13 or L178x102x15 L.L.V. HOT DIP GALV.	SEE NOTES

NOTES:

- ALL LINTELS TO HAVE A MINIMUM BEARING LENGTH OF 150mm (6") EACH SIDE.
- REFER TO PLANS FOR LINTEL SIZES FOR OPENINGS LARGER THAN THOSE INDICATED ABOVE.
- ALL EXTERIOR BRICK LINTELS ARE TO BE HOT DIP GALVANIZED OR COATED WITH A CORROSION INHIBITING PRIMER BY OTHERS.
- BASED ON BRICK HEIGHT, LOOSE LINTELS SHALL NOT BE INSTALLED WHERE HEIGHT OF SUPPORTED BRICK EXCEEDS 1800 mm (6') ABOVE OPENING.
- LINTELS MIN. F_y = 300 MPA U.N.O. ON DRAWINGS

STEEL BEAM SCHEDULE

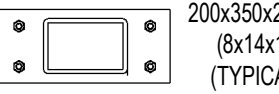
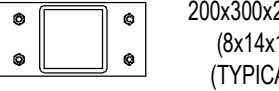
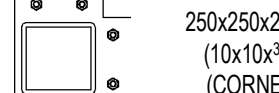
MARK	SIZE	COMMENTS
W250x49	W250x49	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.
W410x46	W410x46	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.
W460x60	W460x60	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.
W610x125	W610x125	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.
W610x155	W610x155	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.
W360x39	W360x39	(2) 3/8" (9.5mm) WEB STIFFENERS BOTH SIDES IF REQUIRED BY CONNECTION DESIGNER, AT COLUMNS ABOVE, AND AT JOIST CONNECTIONS - PROVIDE SHORING DURING CONST. FOR STABILITY AND TO ELIMINATE TORSION.

NOTES:

- ALL BEAMS ARE DROPPED BELOW FLOOR FRAMING UNLESS NOTED OTHERWISE.
- ALL STEEL TO STEEL CONNECTIONS ARE BY THE SUPPLIER. PROVIDE SEALED DRAWINGS, REFER TO PROJECT SPECIFICATIONS FOR REQUIREMENTS.
- ALL STEEL BEAMS CONNECTED TO SLABS SHALL HAVE 19M, 75mm (3") LONG WELDABLE BARS @ 600mm (24") O.C. @ EACH END, WELD ALONG THE CENTER LINE OF THE TOP FLANGE OF BEAM (CENTER LINE OF SUPPORTED WALL TO MATCH CENTER LINE OF BEAM).
- ALL BEAMS SUPPORTING CONCRETE OR CMU WALLS AND CONCRETE FLOOR SHALL HAVE WELDED DOWELS TO MATCH THE VERTICAL REINFORCEMENT AND EXTEND LAP SPlice LENGTH IN THE WALL (REFER TO LAP SPlice LENGTH SCHEDULE FOR DOWELS LENGTH), FULLY WELD ALONG THE CENTER LINE OF THE TOP FLANGE OF BEAM (CENTER LINE OF SUPPORTED WALL TO MATCH CENTER LINE OF BEAM).
- PROVIDE (1) 9.5mm (3/8") STIFFENER EACH SIDE OF BEAM:
 - BELOW ALL STEEL COLUMNS SUPPORTED ON BEAM
 - ABOVE ALL COLUMNS WHERE BEAM CANTILEVERS
 - ABOVE ALL BEARING PLATES FOR BEAMS
 ALL WEB STIFFENERS ARE AS FOLLOWS:

BEAM DEPTH	STIFFENER THICKNESS
<203mm	6.4mm
<610mm	9.5mm
>610mm	13mm

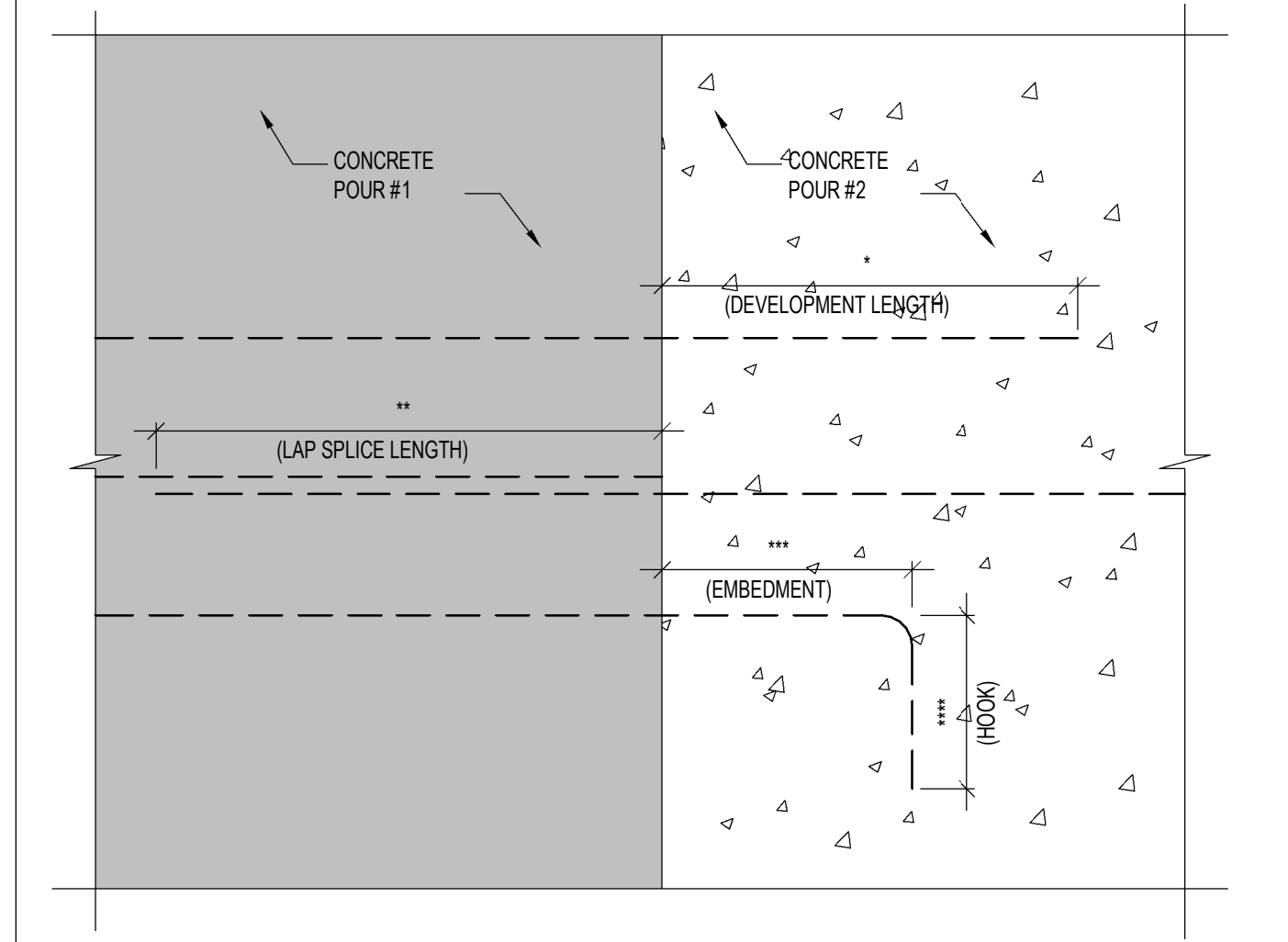
STEEL COLUMN SCHEDULE

MARK	SIZE	TOP PLATE	BOTTOM PLATE	COMMENTS
C ₁	HSS 203x152x13mm	NOTE: 1 AND 2	 200x350x25mm (8x14x1") (TYPICAL)	TYPICAL (4) 20mm(1/2") Ø 600mm (24") EMBED. + 51mm(2") HOOK ANCHOR BOLTS
C ₂	HSS 152x152x13mm	NOTE: 1 AND 2	 200x300x25mm (8x14x1") (TYPICAL)  250x250x25mm (10x10x1") (CORNER)	TYPICAL (4) 20mm(1/2") Ø 600mm (24") EMBED. + 51mm(2") HOOK ANCHOR BOLTS CORNER (4) 20mm (3/4") Ø 600mm (24") EMBED. + 51mm (2") HOOK ANCHOR BOLTS
C ₃	HSS 127x127x9.5mm	NOTE: 1 AND 2	REFER TO TYPICAL DETAIL T.D.54	REFER TO TYPICAL DETAIL T.D.54
C ₄	HSS 102x102x9.5mm	NOTE: 1 AND 2	SEE CANOPY FRAMING PLAN	SEE CANOPY FRAMING PLAN

- TOP/BOTTOM PLATES SHALL BE FULLY WELDED TO COLUMN USING MIN. 6.0mm (1/4") FILLET WELDS OR AS DEFINED BY SUPPLIER.
- PROVIDE 20mm (3/4") THICK STEEL TOP PLATE (MIN.) = TO LAW OF COLUMN U.N.O., TYPICAL.
- PROVIDE 40mm (1-1/2") THICK OF HIGH STRENGTH NON-SHRINK GROUT BELOW ALL COLUMN BASEPLATES. USE SIKAGROUT 212 OR EQUIVALENT W/ MIN COMPRESSIVE STRENGTH OF 40MPa.
- PROVIDE 9.5mm (3/8") TOP PLATE EQUAL TO LAW OF COLUMN, IF BEAM CANTILEVERS OR BEARS ON TOP PLATE, FULLY WELD US OF BEAM TO PLATE W/ CONT. 6.4mm (1/4") FILLET, TYPICAL.
- BASEPLATE BOLTED OR WELDED TO TOP FLANGE OF BEAM. CONNECTION BY STEEL SUPPLIER. PROVIDE (1) 13mm (1/2") WEB STIFFENER EACH SIDE OF BEAM AT COLUMN BEARING LOCATION. SEE BEAM SCHEDULE FOR THICKNESS
- ALL COLUMNS BEARING ON BEAMS SHALL HAVE A BASEPLATE EQUAL TO THE WIDTH OF THE BEAM BY THE WIDTH OF THE COLUMN +76mm (3") EA. SIDE, AND THE THICKNESS GREATER THAN OR EQUAL TO THE COLUMN WEB THICKNESS.
- STEEL COLUMN MAY BE SPliced @ FLOOR LEVEL IF REQUIRED FOR EASE OF INSTALLATION. CONNECTION TO BE DESIGNED BY STEEL SUPPLIER FOR FULL CAPACITY. PLATES TO FIT WITHIN WALL CAVITY, IF APPLICABLE.
- SEE TYPICAL DETAILS FOR COLUMN BRACING AND BEARING.
- ALL ANCHOR EXTEND INTO FOOTINGS AND HOOK IF NEEDED - ENSURE 75mm CONCERT COVER- TYPICAL ALL LOCATIONS FOR LONG ANCHOR BOLTS

REBAR DEVELOPMENT SCHEDULE

SIZE	STRAIGHT REINFORCING		90° STANDARD HOOKS	
	DEVELOPMENT LENGTH (')	LAP SPlice LENGTH (")	EMBEDMENT (")	HOOK (")
10M	380mm (15')	500mm (20')	200mm (8')	150mm (6')
15M	580mm (23')	750mm (30')	300mm (12')	210mm (8 1/2')
20M	760mm (30')	1000mm (40')	400mm (16')	260mm (10 1/2')
25M	1200mm (48')	1560mm (62')	500mm (20')	340mm (13 1/2')
30M	1440mm (57')	1875mm (74')	600mm (24')	410mm (16 1/2')
35M	1680mm (66')	2185mm (86')	700mm (28')	490mm (19 1/2')



NOTES:

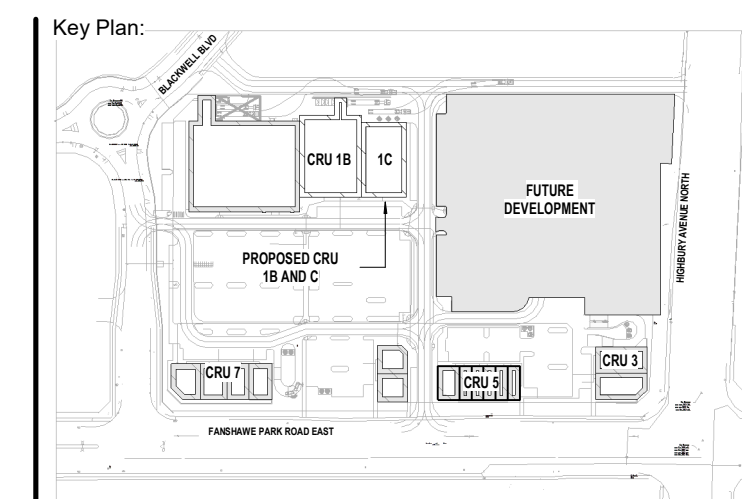
- INCREASE ABOVE BY 1.5 TIMES FOR ALL EPOXY COATED REBARS.
- THESE NUMBERS ARE ONLY FOR NORMAL WEIGHT CONCRETE.
- CONCRETE COVER MUST BE EQUAL OR MORE THAN 1.5 REBAR DIAMETER.
- CLEAR SPACING NOT LESS THAN 2 REBAR DIAMETERS.
- YIELD STRESS FOR REBARS (F_y) = 400 MPa.

FOUNDATION SCHEDULE

MARK	WALL WIDTH/PIER SIZE	WALL / PIER REINFORCING	STRIP / PAD FOOTING SIZE	FOOTING REINFORCING	DETAIL
WF-1	WALL SIZE 385mm (15 1/4") - SEE PLAN AND ARCH. DRAWINGS PIER SIZE 400mm x 400mm (16"x16") - SEE PLAN	WALL REINF. (2)20M CONT. TOP AND BOTTOM ENSURE FULL SPlice AS PER REBAR DEVELOPMENT SCHEDULE PIER REINF. (4) 15M VERT. + 10M TIES @ 200mm (8" O.C.)	1500mm x 400mm DP (3'-3" x 1'-7" DP)	FOOTING REINF. 15M @ 250MM E.W. BOTTOM ENSURE FULL SPlice AS PER REBAR DEVELOPMENT SCHEDULE	---
WF-2	WALL SIZE 225mm (9") - SEE PLAN AND ARCH. DRAWINGS PIER SIZE 400mm x 400mm (16"x16") - SEE PLAN	WALL REINF. (2)20M CONT. TOP AND BOTTOM ENSURE FULL SPlice AS PER REBAR DEVELOPMENT SCHEDULE PIER REINF. (4) 15M VERT. + 10M TIES @ 200mm (8" O.C.)	1500mm x 400mm DP (3'-3" x 1'-7" DP)	FOOTING REINF. 15M @ 250MM E.W. BOTTOM ENSURE FULL SPlice AS PER REBAR DEVELOPMENT SCHEDULE	---
F-1	PIER SIZE 400mm x 400mm (16"x16")	PIER REINF. (4) 15M VERT. + 10M TIES @ 200mm (8" O.C.)	2000mm x 2000mm x 600mm DP (6'6" x 6'6" x 2'5" DP)	FOOTING REINF. 20M @ 250MM E.W. BOTTOM	---

NOTES:

- PROVIDE 75mm (3") CLEAR COVER FOR REINFORCEMENT IN FOOTINGS AND PILES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. REFER TO GENERAL NOTES FOR CONCRETE AND REINFORCING SPECIFICATIONS, TYPICAL.
- ALL FOOTINGS TO HAVE MIN. 1200mm (4'-0") FROST PROTECTION. G.C. TO CONFIRM THE UNDERSIDE OF FOUNDATIONS BASED ON THE LATEST GEOTECHNICAL REPORT RECOMMENDATIONS.
- PROVIDE DOWELS FROM FOOTINGS INTO CONCRETE WALLS/COLUMNS ABOVE. MATCH VERTICAL WALL/COLUMN REINFORCING BAR SIZE AND SPACING/NUMBER. DOWELS SHALL HAVE STANDARD 90° HOOKS, BE TIED TO THE BOTTOM MAT IN FOOTING, AND HAVE BAR EXTENSIONS ABOVE FOOTINGS FOR A TYPICAL LAP SPlice.
- PROVIDE DOWELS FROM FOUNDATION WALLS INTO STRIP FOOTINGS. MATCH VERT. WALL REINFORCING BAR SIZE AND SPACING. EXTEND INTO MIDDLE OF UNREINFORCED FOOTING OR TO THE BOTTOM MAT OF REINFORCED FOOTING. HOOK REINFORCEMENT IN ACCORDANCE WITH THE REBAR LAP/HOOK SCHEDULE. WHERE WALL ABOVE HAS NO VERTICAL REINFORCEMENT, PROVIDE PROVIDE 15M DOWELS FROM FOUNDATION WALL INTO STRIP FOOTING, 750mm (30") LONG W/ 200mm (8") HOOK @ 600mm (24") O.C. MAX. ENSURE 152mm (6") EMBEDMENT MIN., TYPICAL.
- INSTALL (2)10M TIES IN THE TOP 100mm (4") OF ALL PIERS. AROUND ALL DOOR OPENINGS INSTALL (2)10M DIAGONAL CORNER BARS (1 E.F.) 600mm (24") LONG, TYPICAL.
- REFER TO PLAN AND SCHEDULES FOR CONCRETE COLUMN REINFORCING INFORMATION. REINFORCING SPECIFIED IS TO EXTEND BELOW TO SLAB TO TOP OF PAD FOOTING, TYPICAL.
- REFER TO PLAN AND SCHEDULES FOR CONCRETE WALL REINFORCING INFORMATION. REINFORCING SPECIFIED IS TO EXTEND BELOW T/O SLAB TO TOP OF STRIP/PAD FOOTINGS, TYPICAL.



Site Plan: North Arrow:

Consultant:

IE DESIGN
 Intelligent Engineering Design Ltd.
STRUCTURAL ENGINEERS
 iedesign@iedesign.ca www.iedesign.ca

Seal: Seal:

General Notes:
 ALL DRAWINGS AND SPECIFICATIONS ARE THE PROPERTY OF INTELLIGENT ENGINEERING DESIGN LTD. (IE DESIGN) AND ARE NOT TO BE DUPLICATED OR DISTRIBUTED WITHOUT IE DESIGN CONSENT AND MUST BE RETURNED UPON COMPLETION OF THIS PROJECT.
 THESE DRAWINGS AND ALL DETAILS ARE FOR THIS PROJECT ONLY AND SHOULD NOT BE USED FOR ANY OTHER WORK.
 IT IS THE RESPONSIBILITY OF THE APPROPRIATE CONTRACTOR TO CHECK AND VERIFY ALL DIMENSIONS ON SITE AND REPORT ALL ERRORS AND OMISSIONS TO INTELLIGENT ENGINEERING DESIGN LTD. PRIOR TO COMMENCING WORK. ALL DIMENSIONS AND LEVELS ARE APPROXIMATE AND ANY PRICES BASED ON THESE DRAWINGS MUST INCLUDE ALLOWANCES FOR THIS WITH NO LIABILITY ON INTELLIGENT ENGINEERING DESIGN LTD.
 ALL CONTRACTORS MUST COMPLY WITH ALL PERTINENT BUILDING CODE REGULATIONS AND BYLAWS HAVING JURISDICTION.
 THIS DRAWING MUST NOT BE USED FOR CONSTRUCTION UNTIL IT HAS BEEN STAMPED BY INTELLIGENT ENGINEERING DESIGN LTD. AND A BUILDING PERMIT HAS BEEN ISSUED AND MARKED "ISSUED FOR CONSTRUCTION". THE DRAWINGS SHALL NOT BE USED FOR PRICING, COSTING, OR TENDER UNLESS INDICATED IN THE REVISION COLUMN AND THESE DRAWINGS ARE NOT COMPLETE AND ANY PRICES BASED ON THESE DRAWINGS MUST INCLUDE ALLOWANCES FOR THIS WITH NO LIABILITY ON INTELLIGENT ENGINEERING DESIGN LTD.
 CONSTRUCTION IS TO BE ACCORDING TO BEST COMMON PRACTICE.
 DO NOT SCALE DRAWINGS. WHEN REQUIRED REQUEST WRITTEN VERIFICATION OF DIMENSIONS FROM INTELLIGENT ENGINEERING DESIGN LTD. - USE LATEST REVISED DRAWINGS.
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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 03 2024	ISSUED FOR COORDINATION

ISSUES/REVISION TABLE

Project:

WESTDELL
 DEVELOPMENT CORP

1300 FANSHAWE PARK RD.
 EAST. - CRU #1B AND C

1300 FANSHAWE PARK RD. EAST. LONDON, ON.

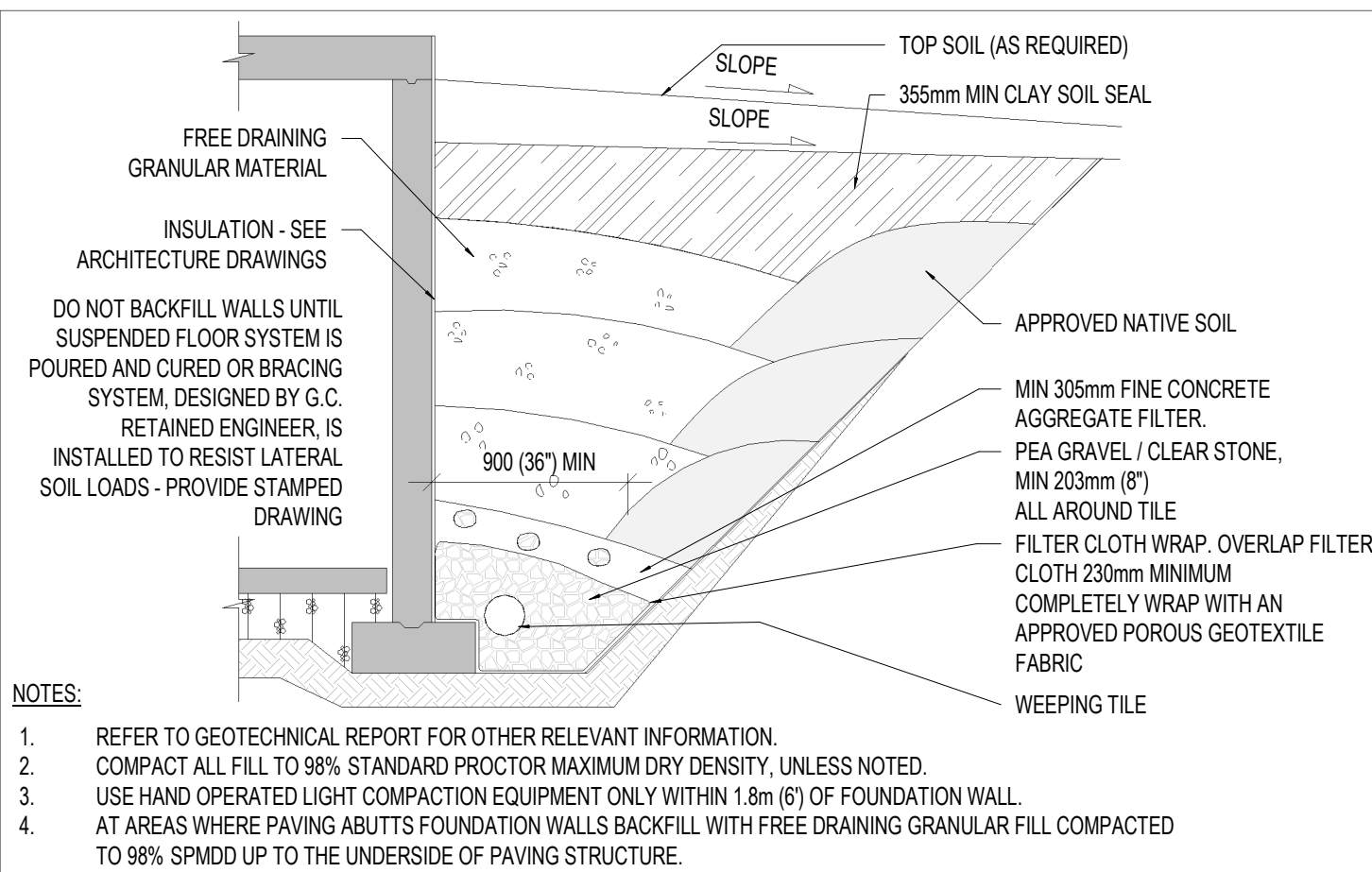
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LOADING AND SCHEDULE

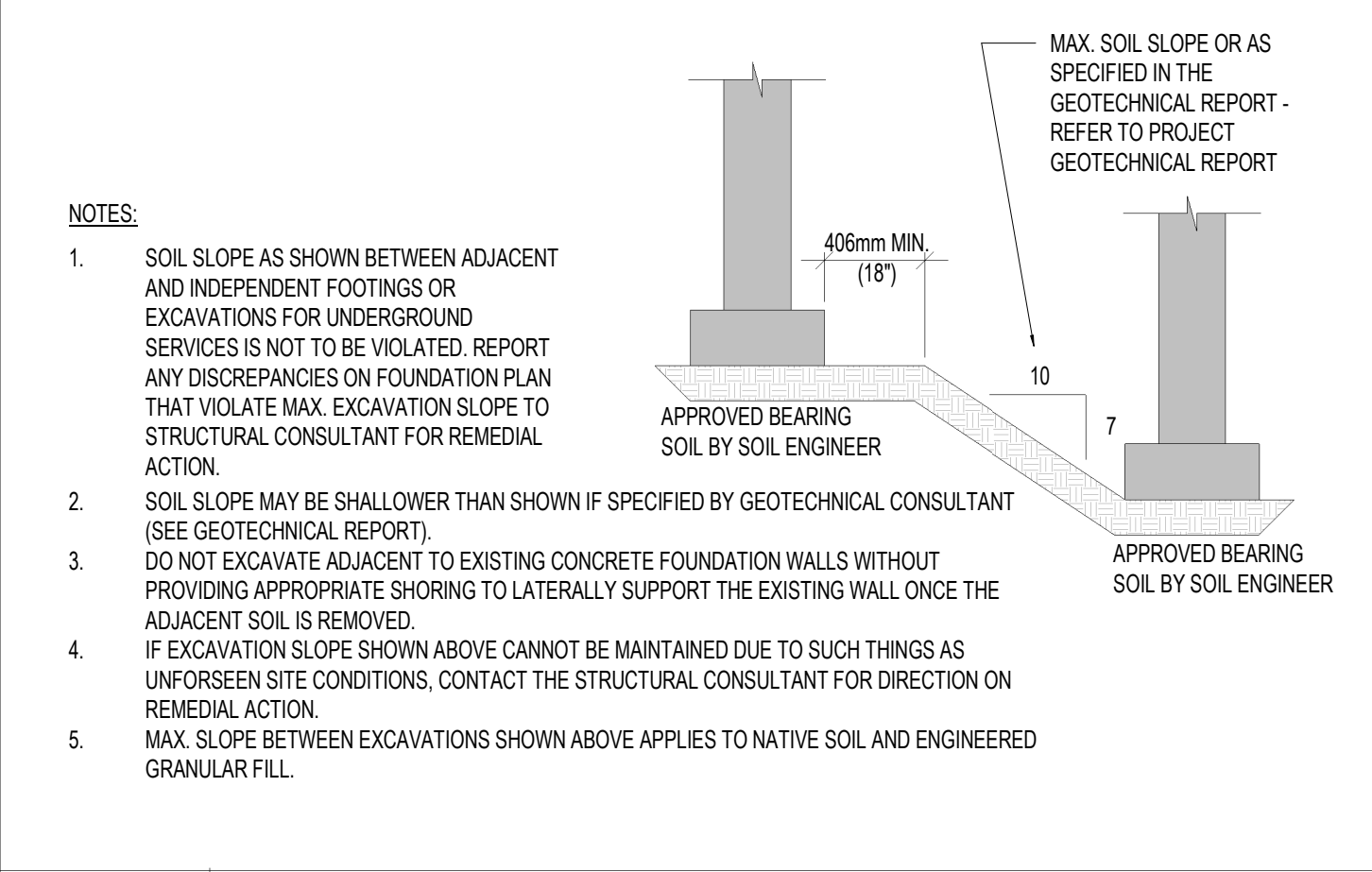
Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
 Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
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Drawing No: Revision

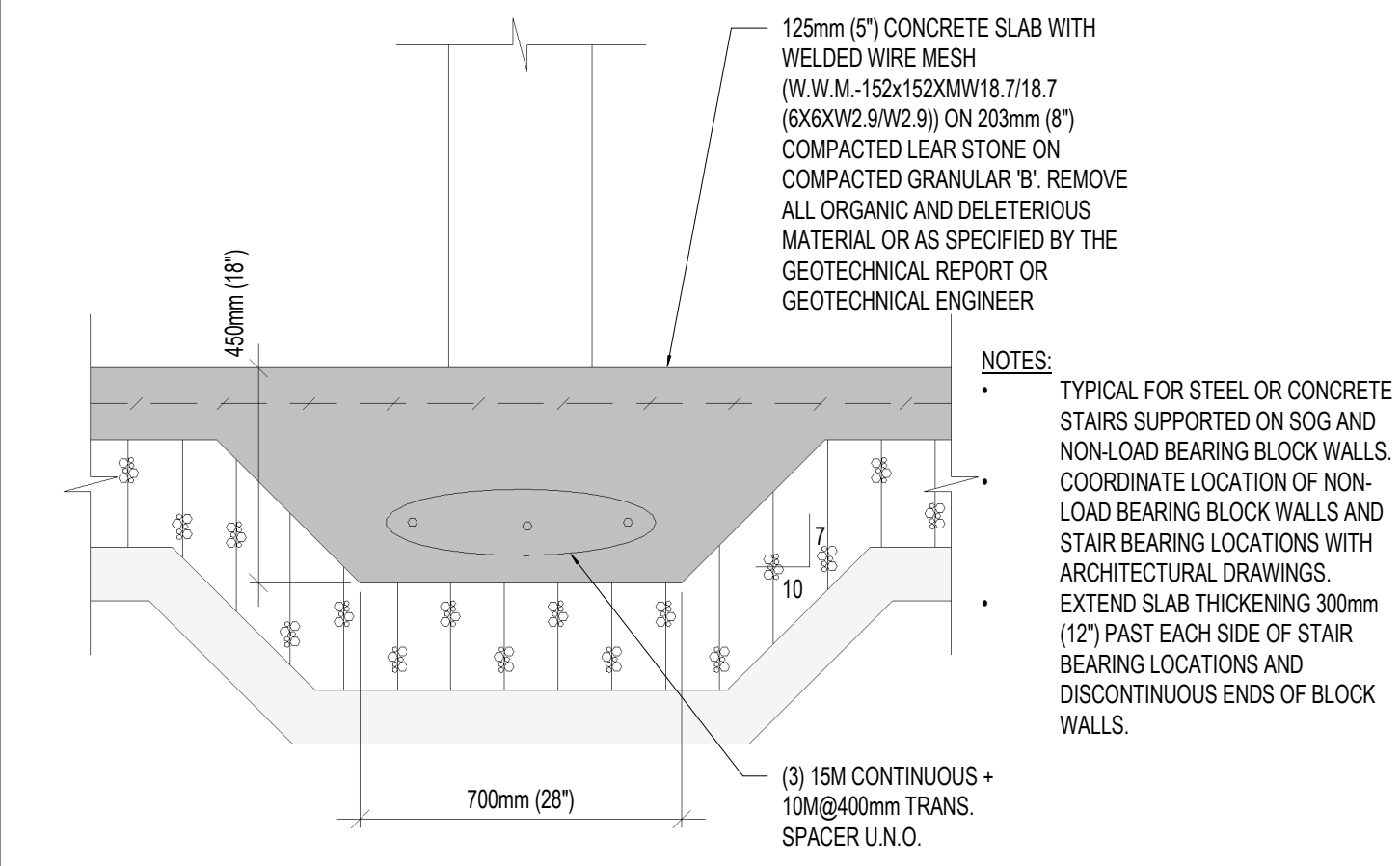
S-5.0 **2**



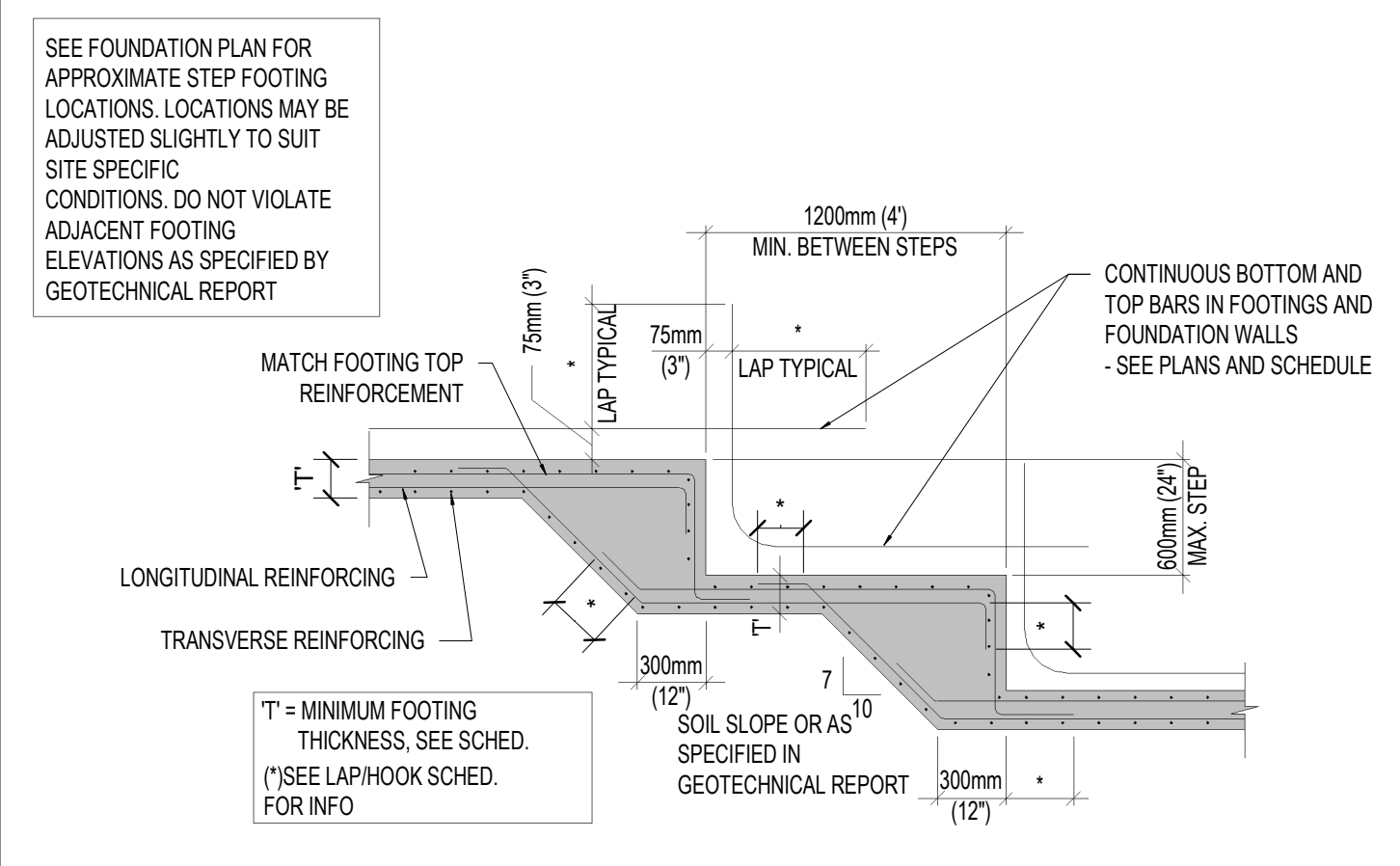
T.D.1 BACKFILL FOR EXTERIOR FDN. WALLS @ LANDSCAPED AREAS



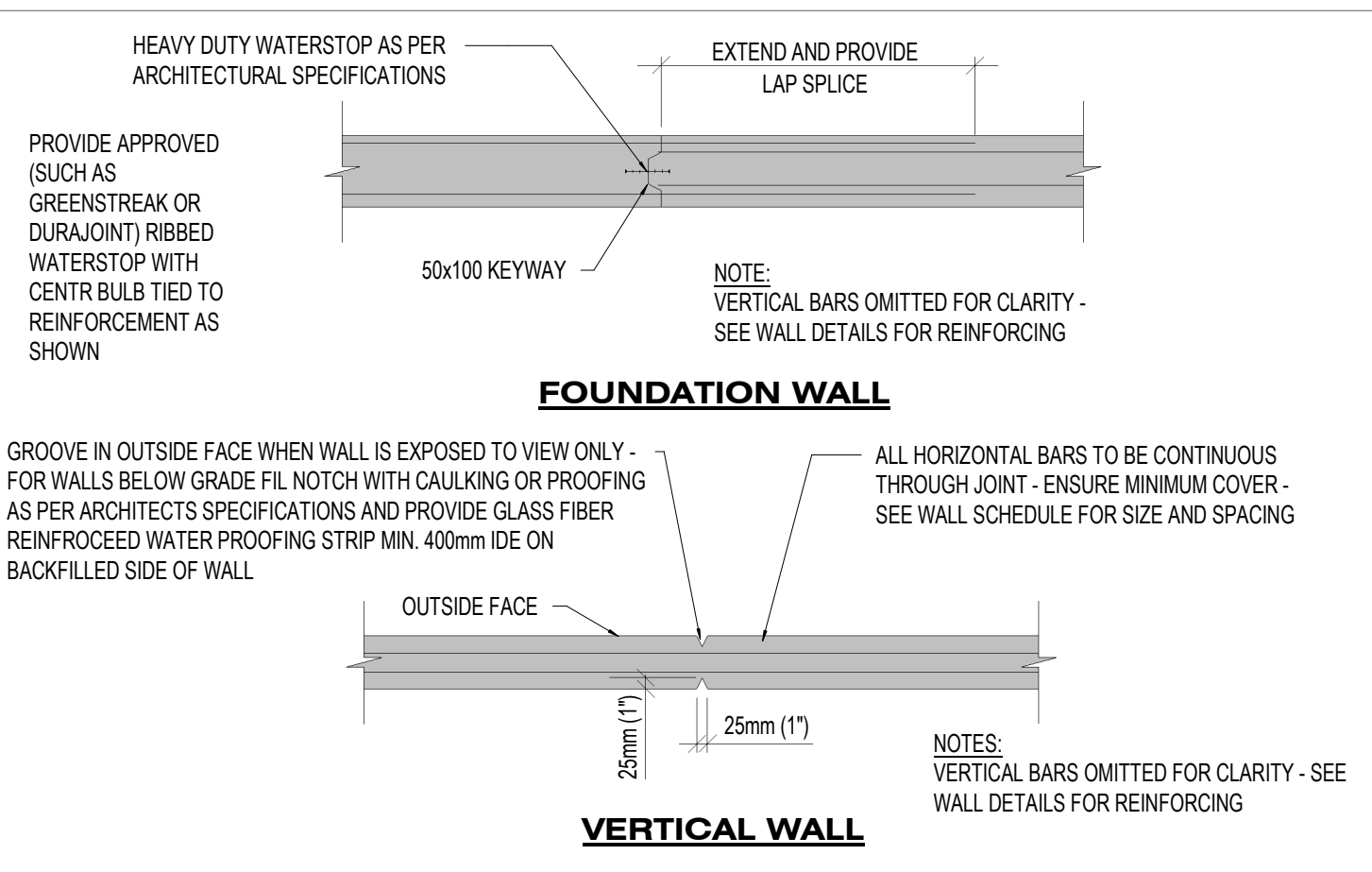
T.D.2 CONTROL JOINTS



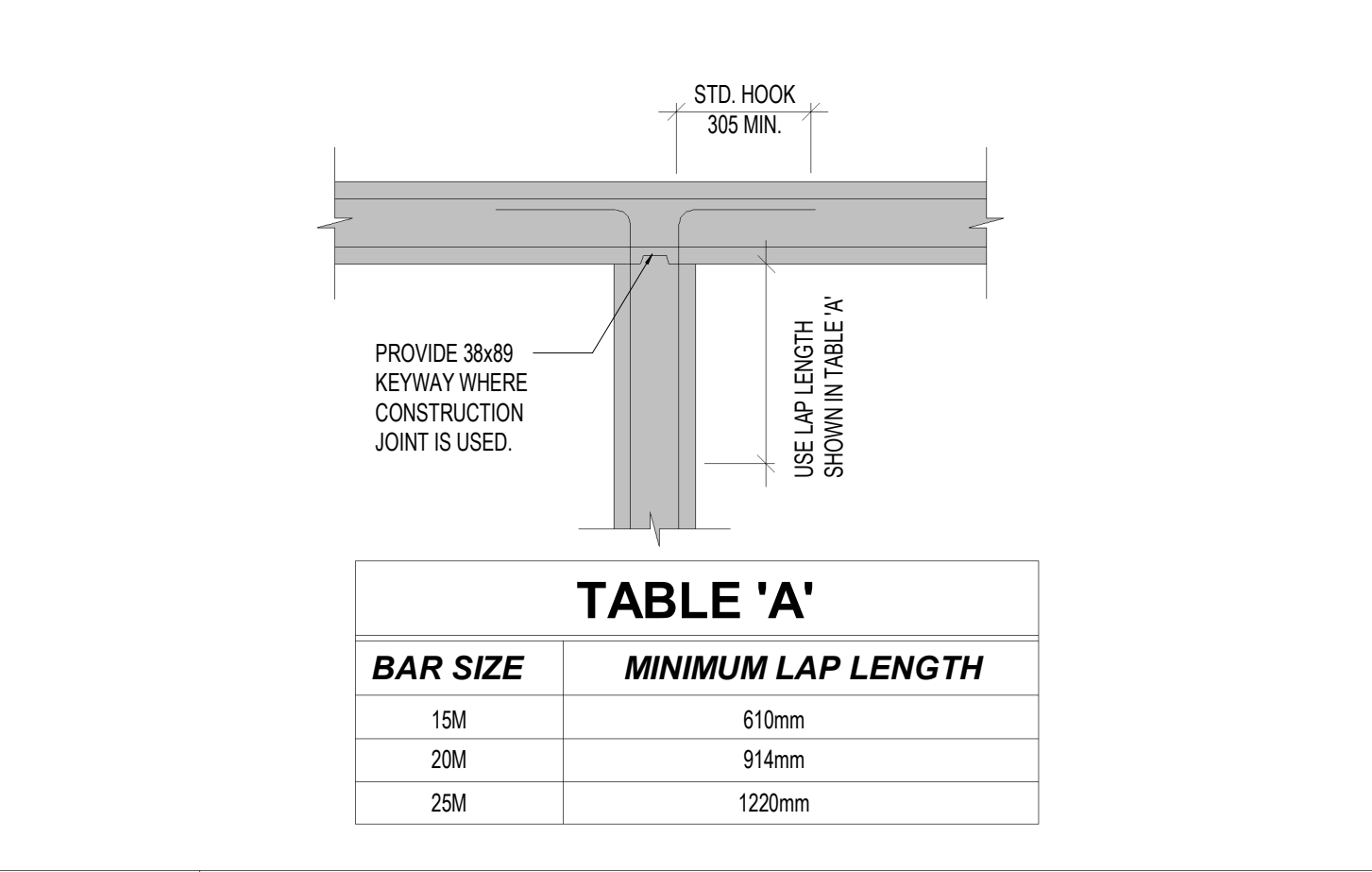
T.D.3 FOOTING TRANSITION



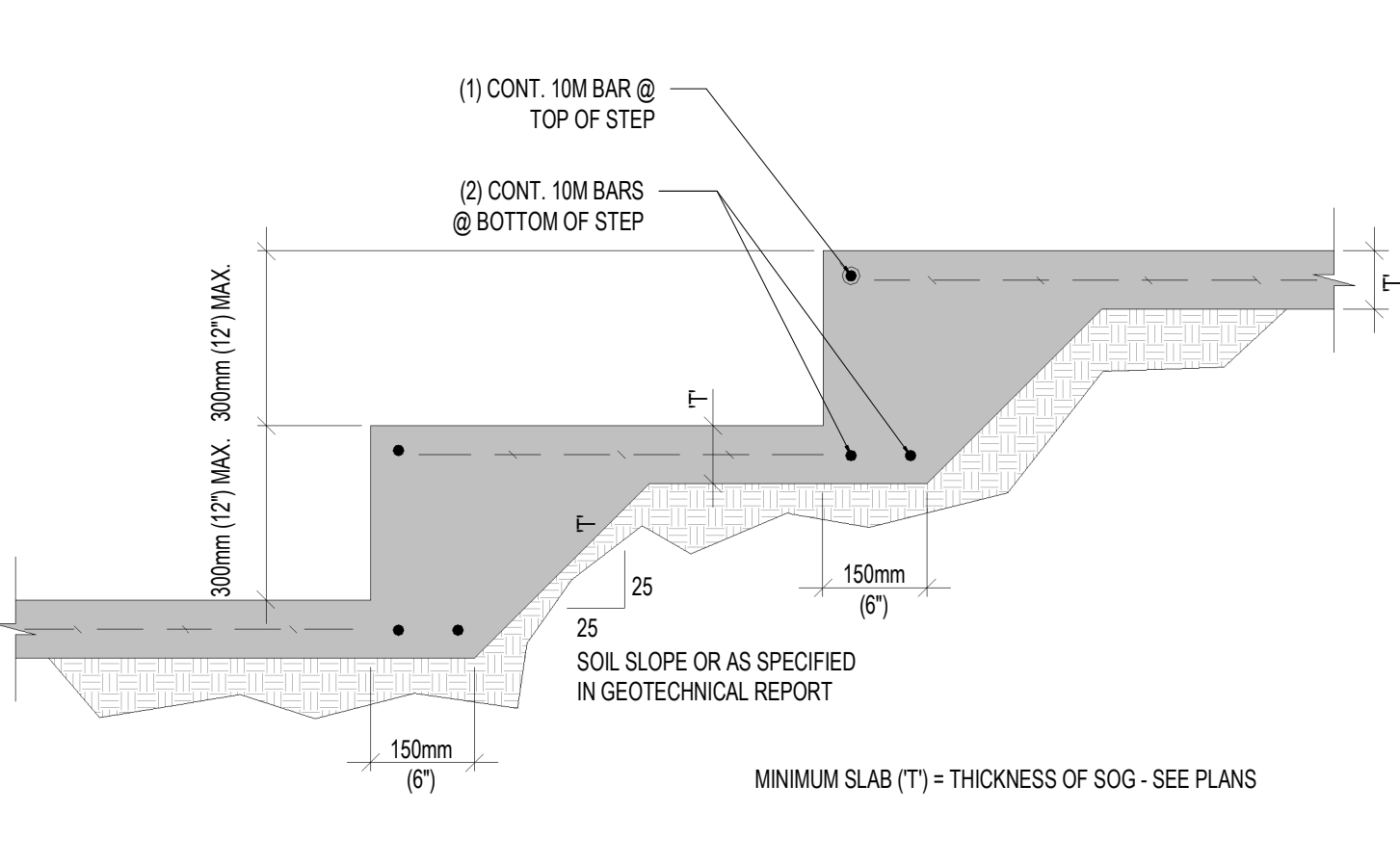
T.D.4 FOUNDATION STRIP FOOTING CONSTRUCTION JOINT



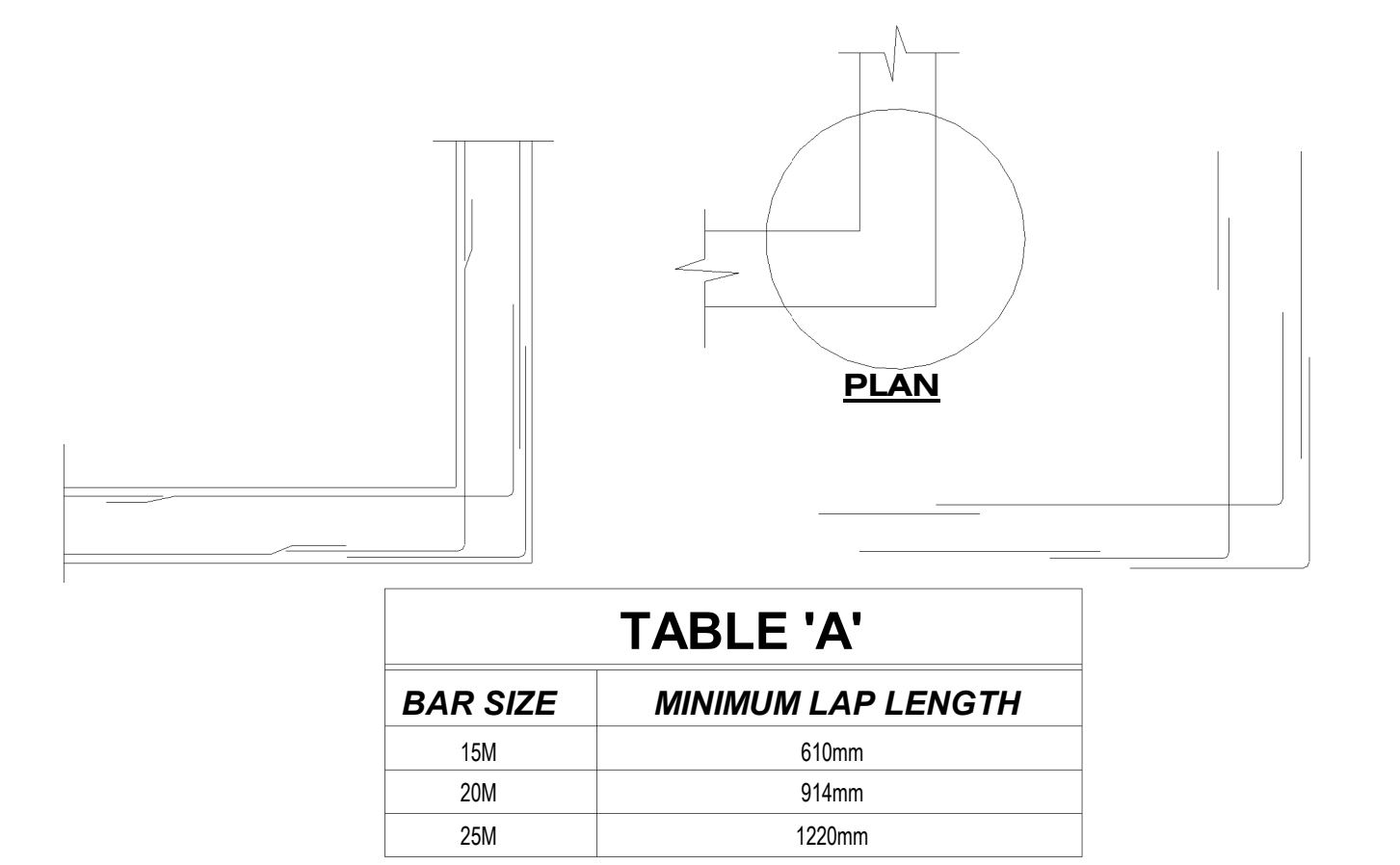
T.D.5 SLOPE OF ADJACENT FOOTING EXCAVATIONS



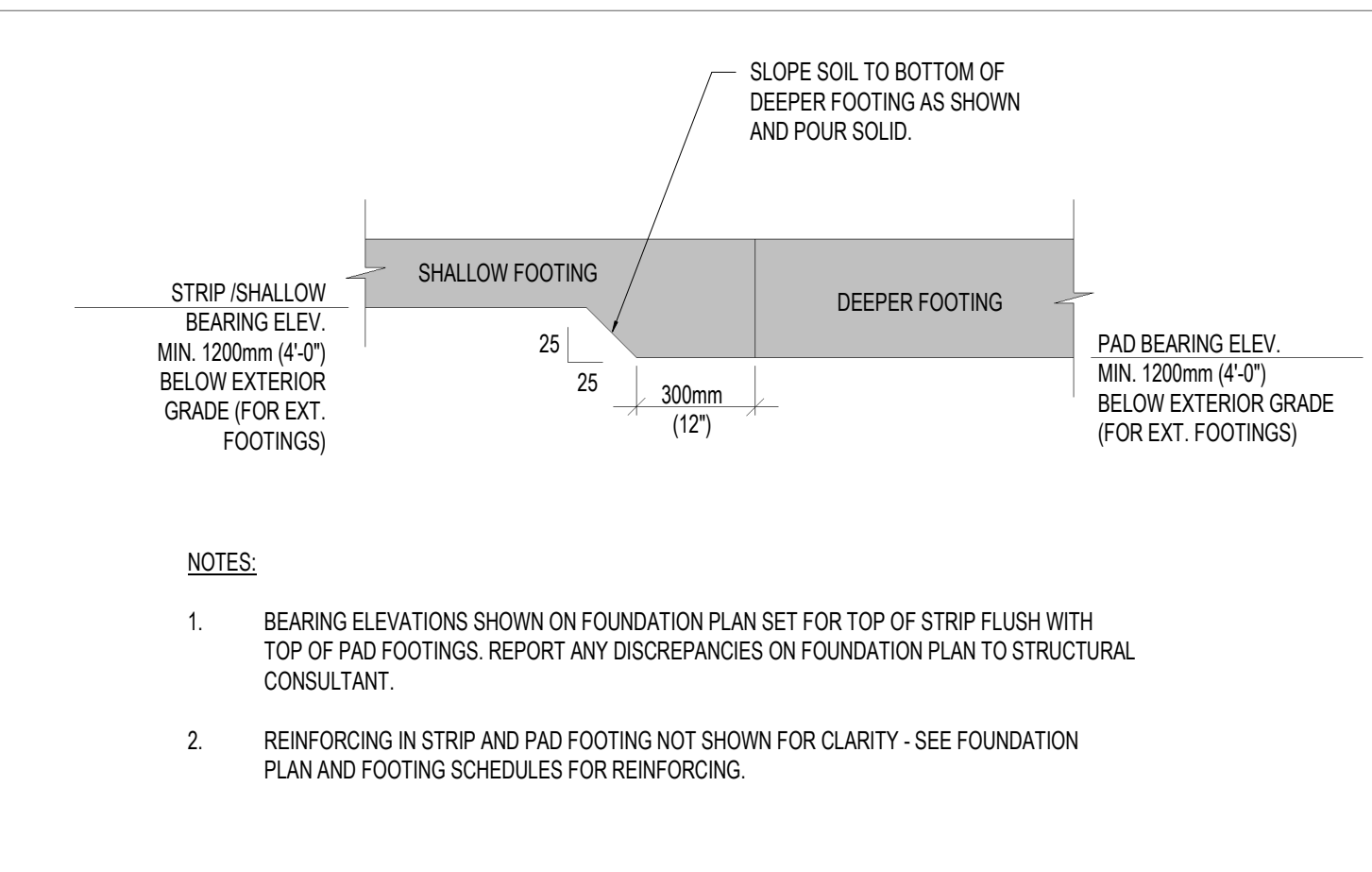
T.D.6 REINFORCING DETAILS FOR WALL INTERSECTION



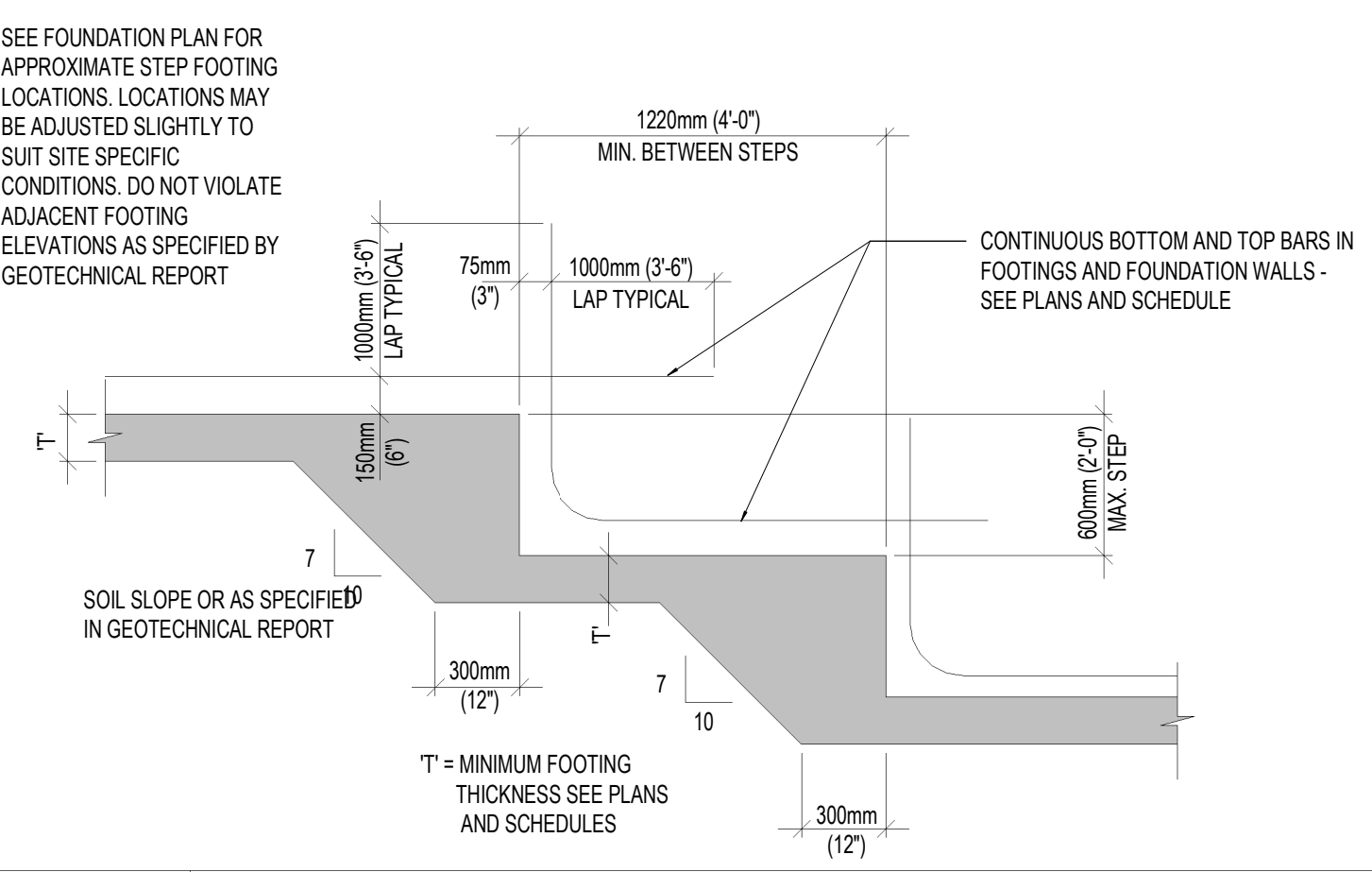
T.D.7 TYPICAL STEP FOOTING



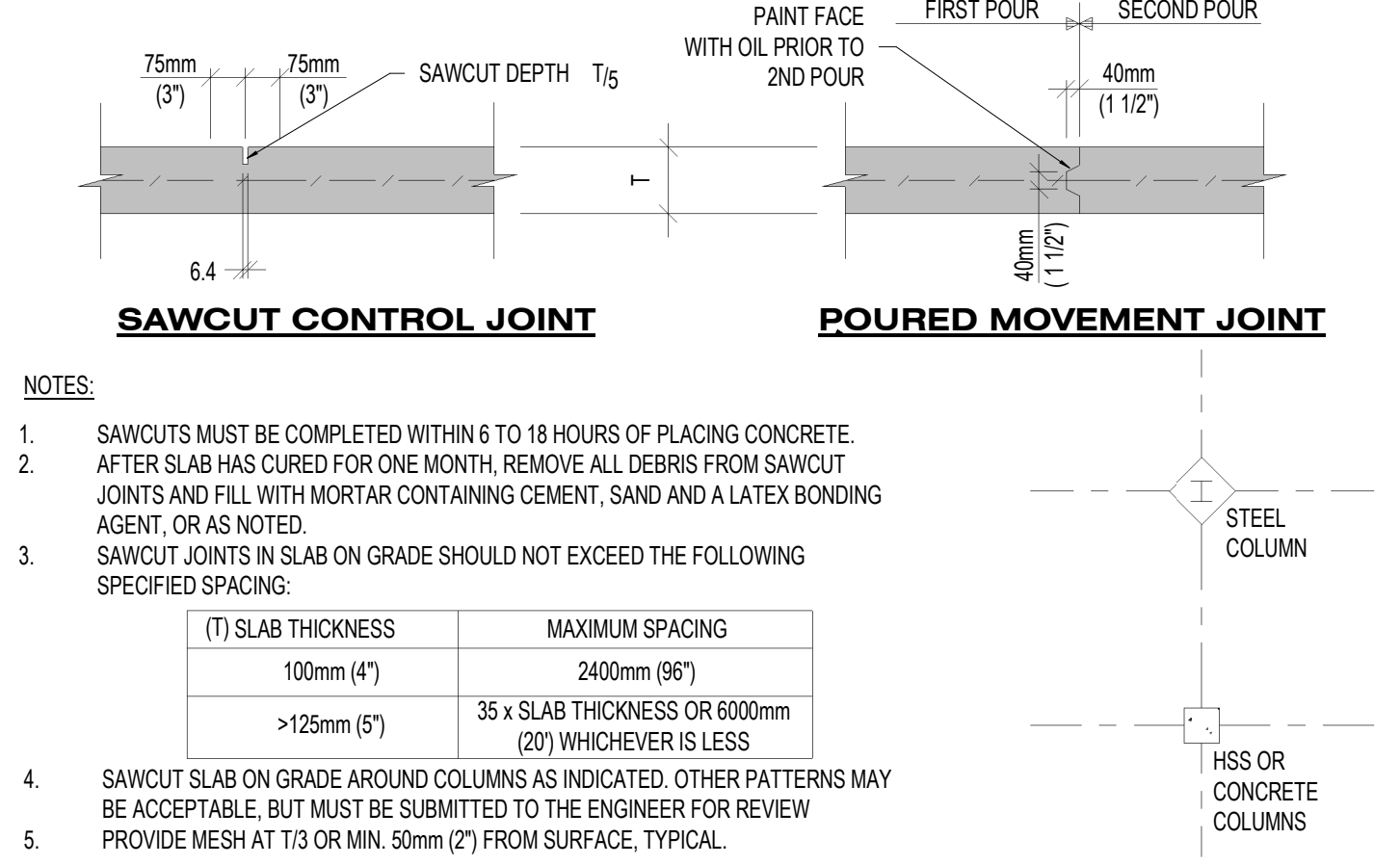
T.D.8 FOUNDATION WALL CONSTRUCTION JOINT



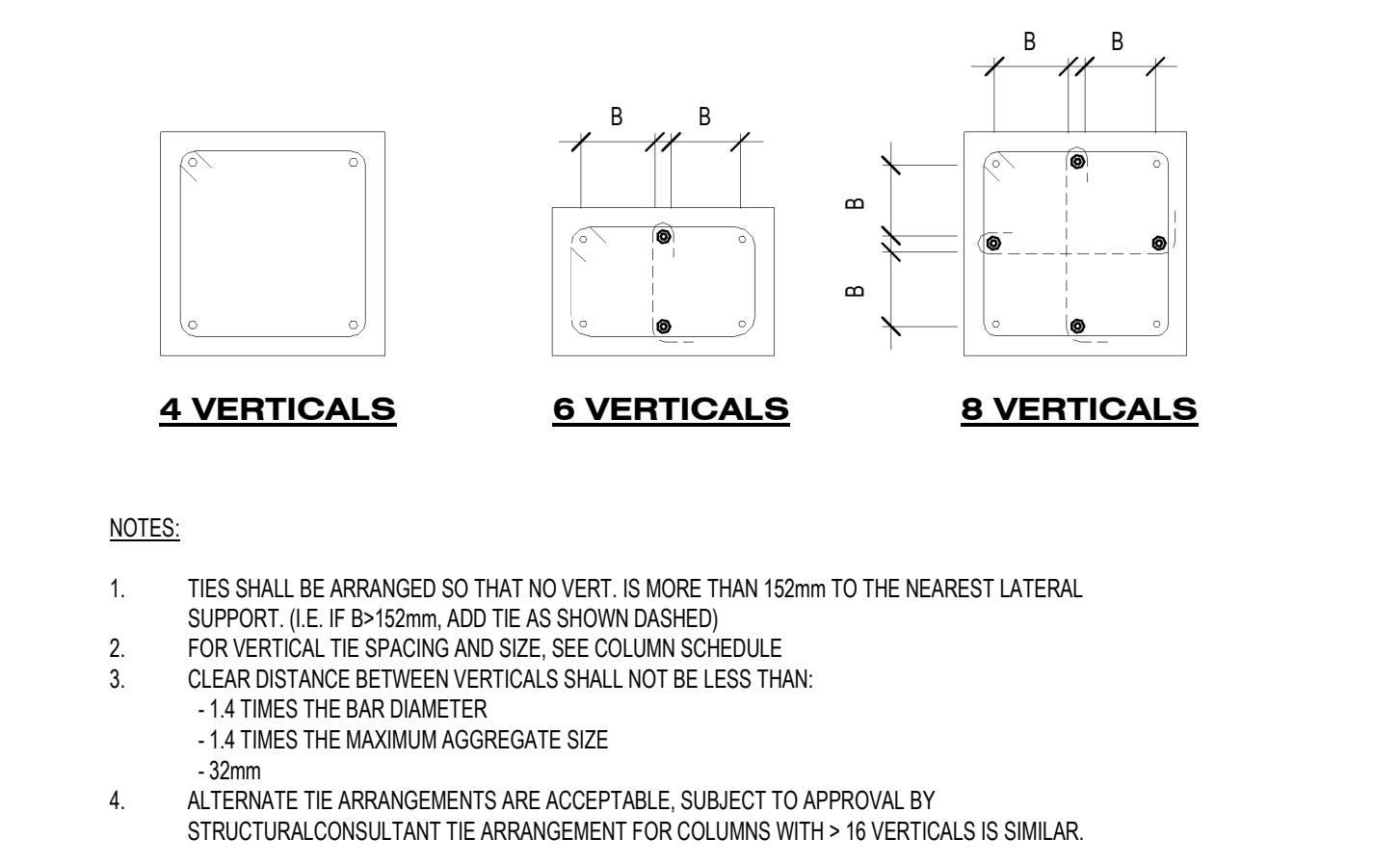
T.D.9 TYPICAL SLAB THICKENING



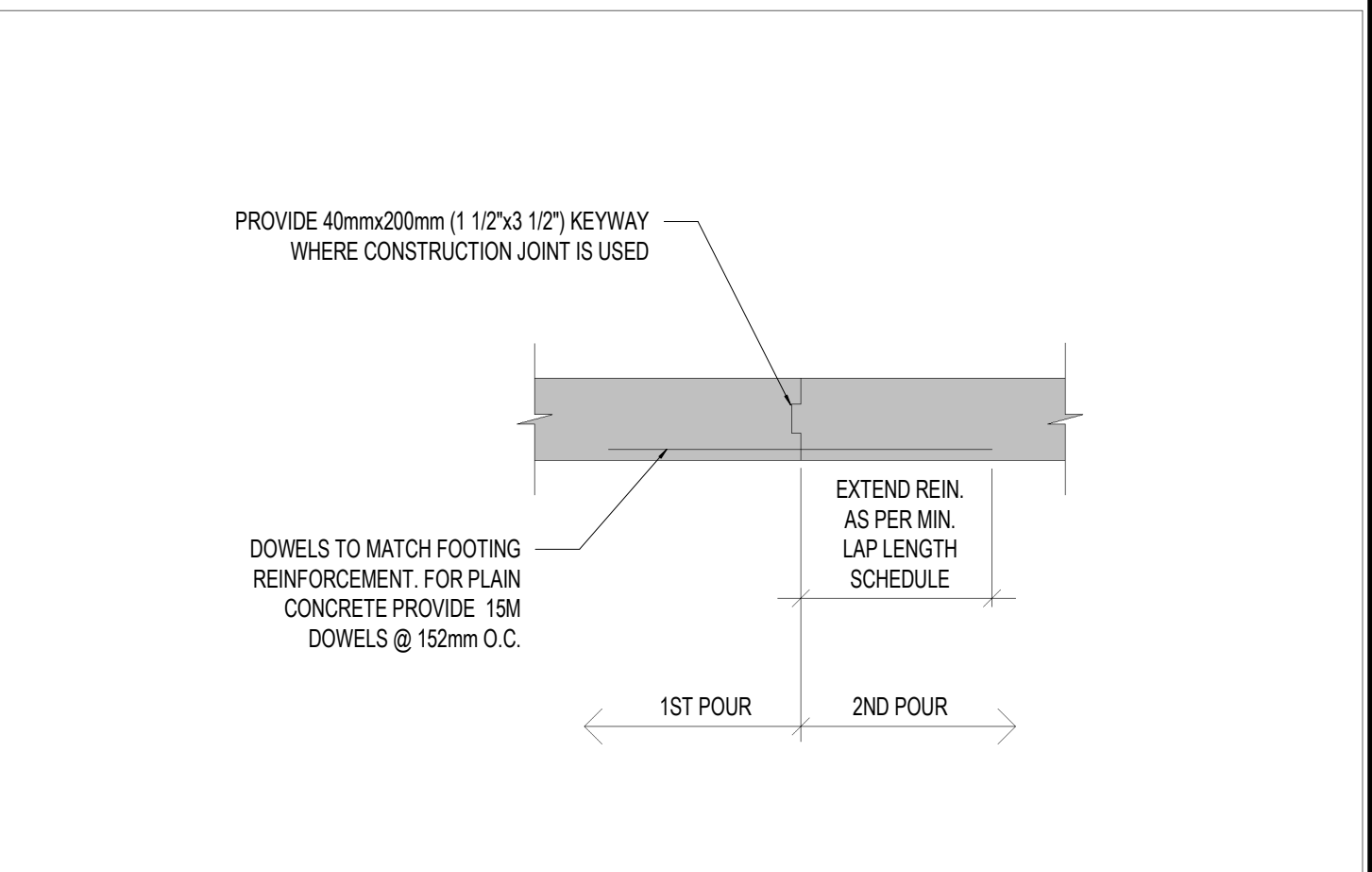
T.D.10 STEP SLAB-ON-GRADE



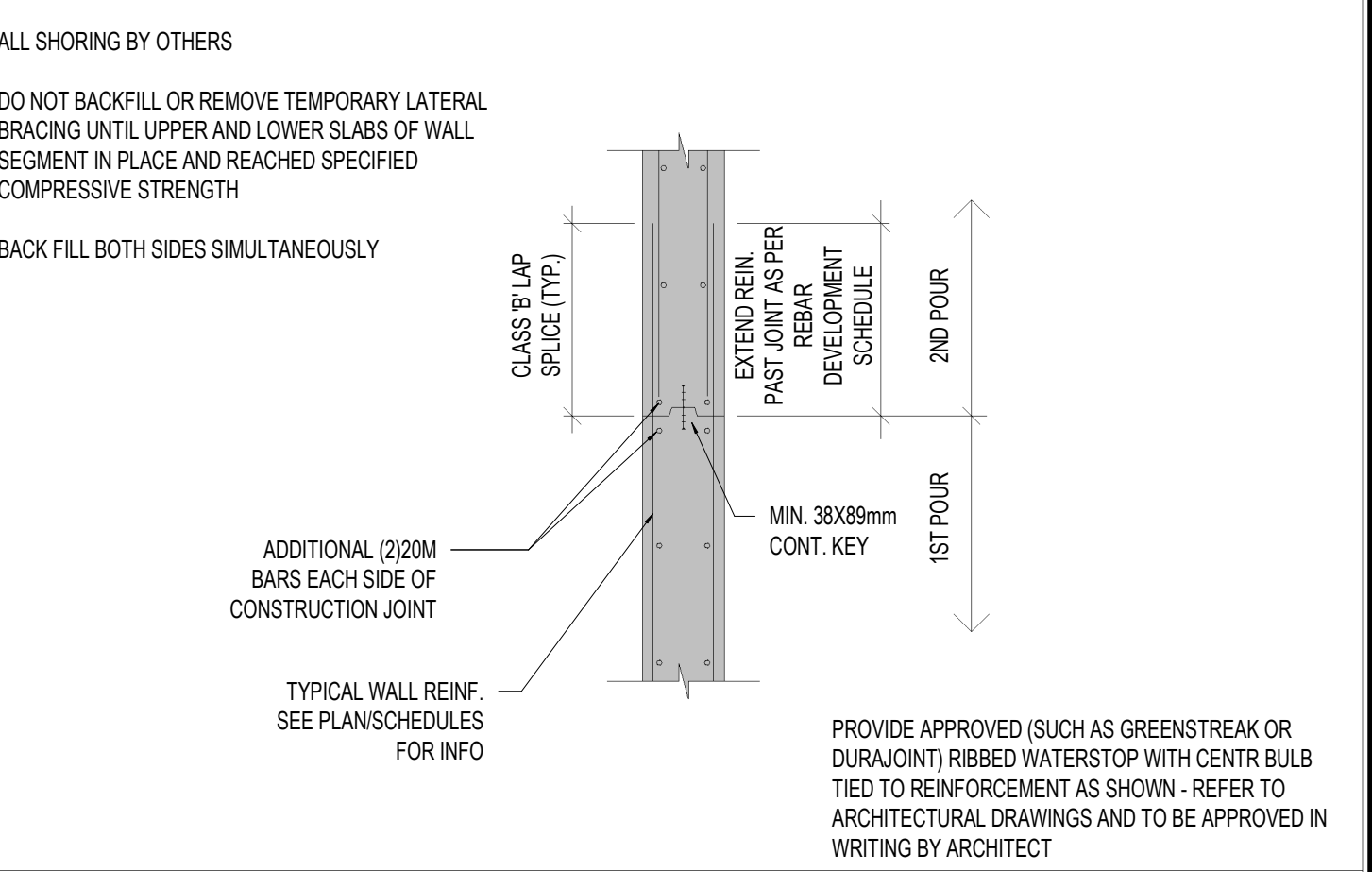
T.D.11 TYPICAL FLOOR SLAB CONTROL JOINT



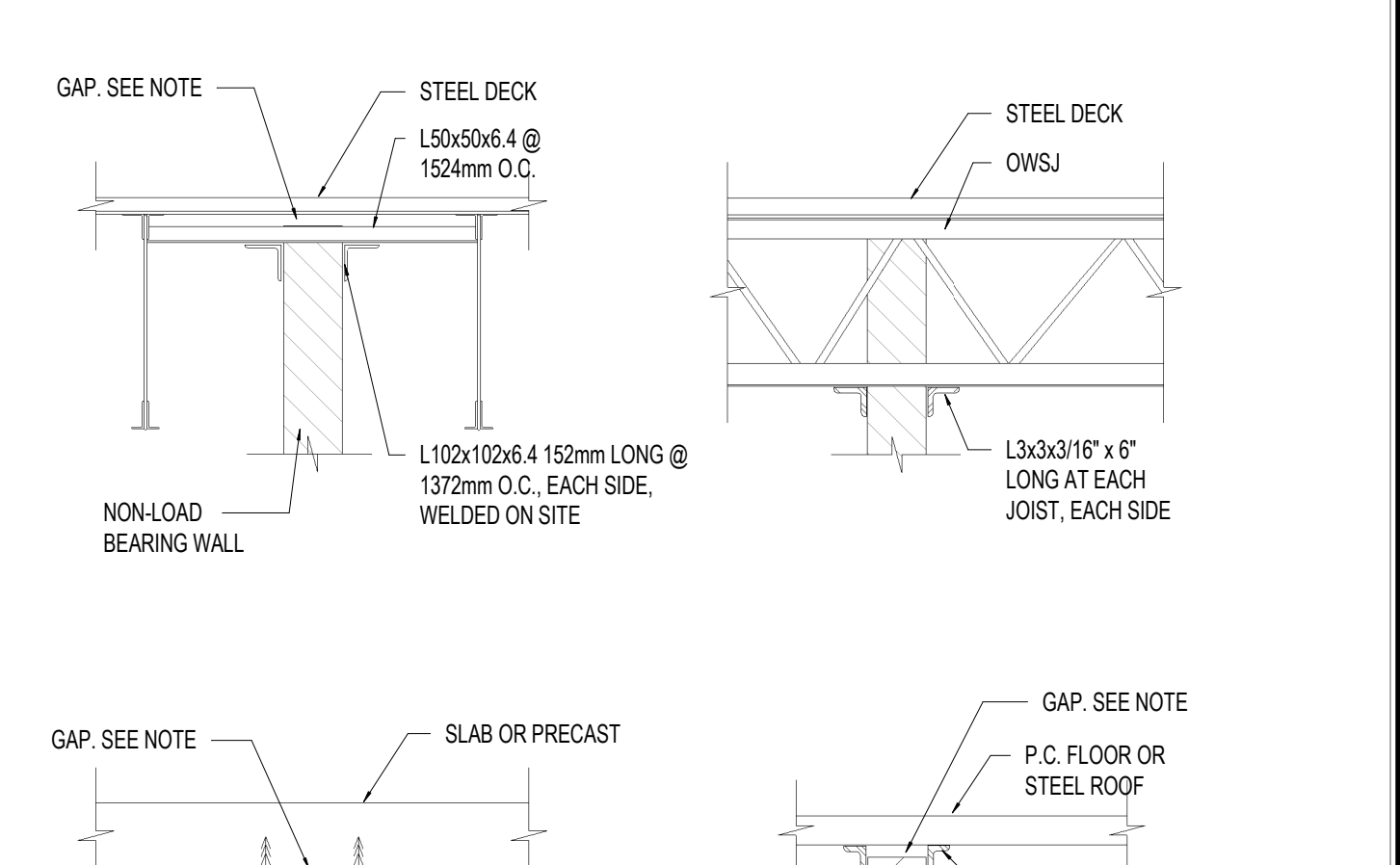
T.D.12 TYPICAL REINFORCED STEP FOOTING



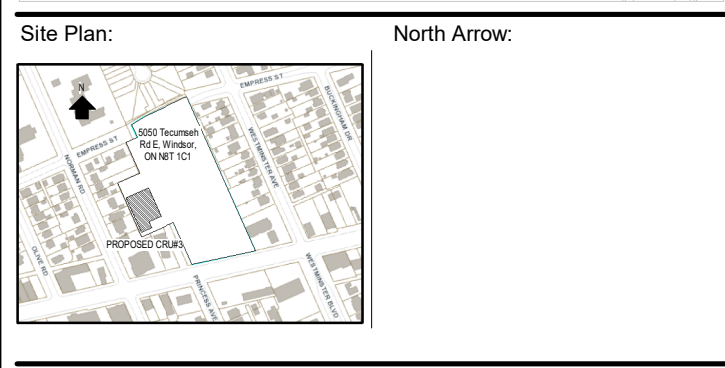
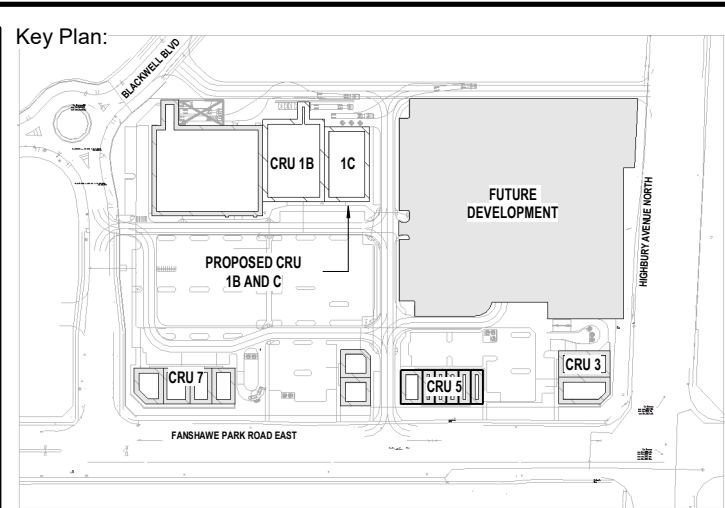
T.D.13 HORIZONTAL CORNER REINFORCING DETAILS FOR WALLS



T.D.14 TIE ARRANGEMENT IN CONCRETE COLUMNS AND PIERS



T.D.15 TYPICAL MASONRY PARTITION SUPPORT



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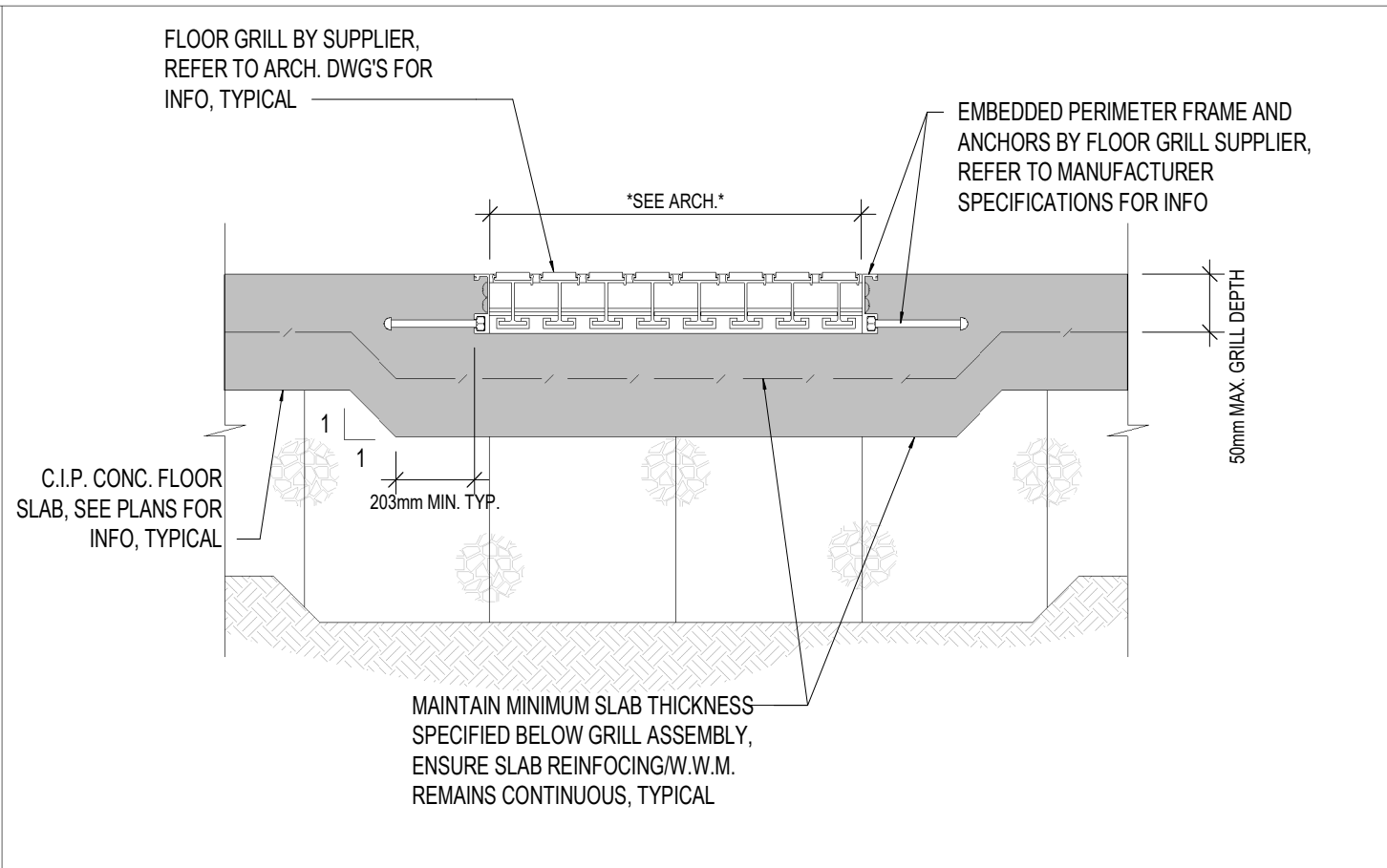
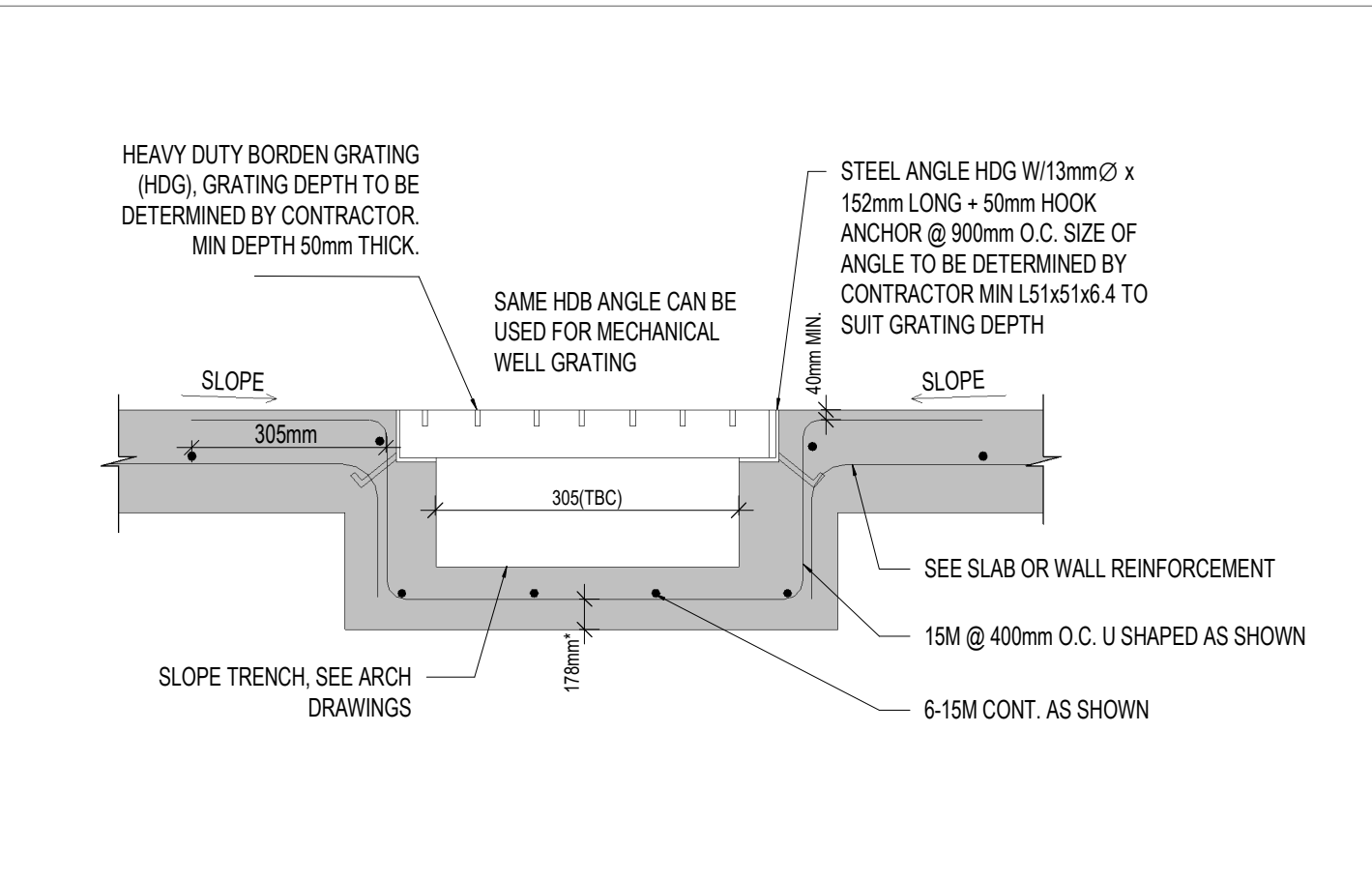
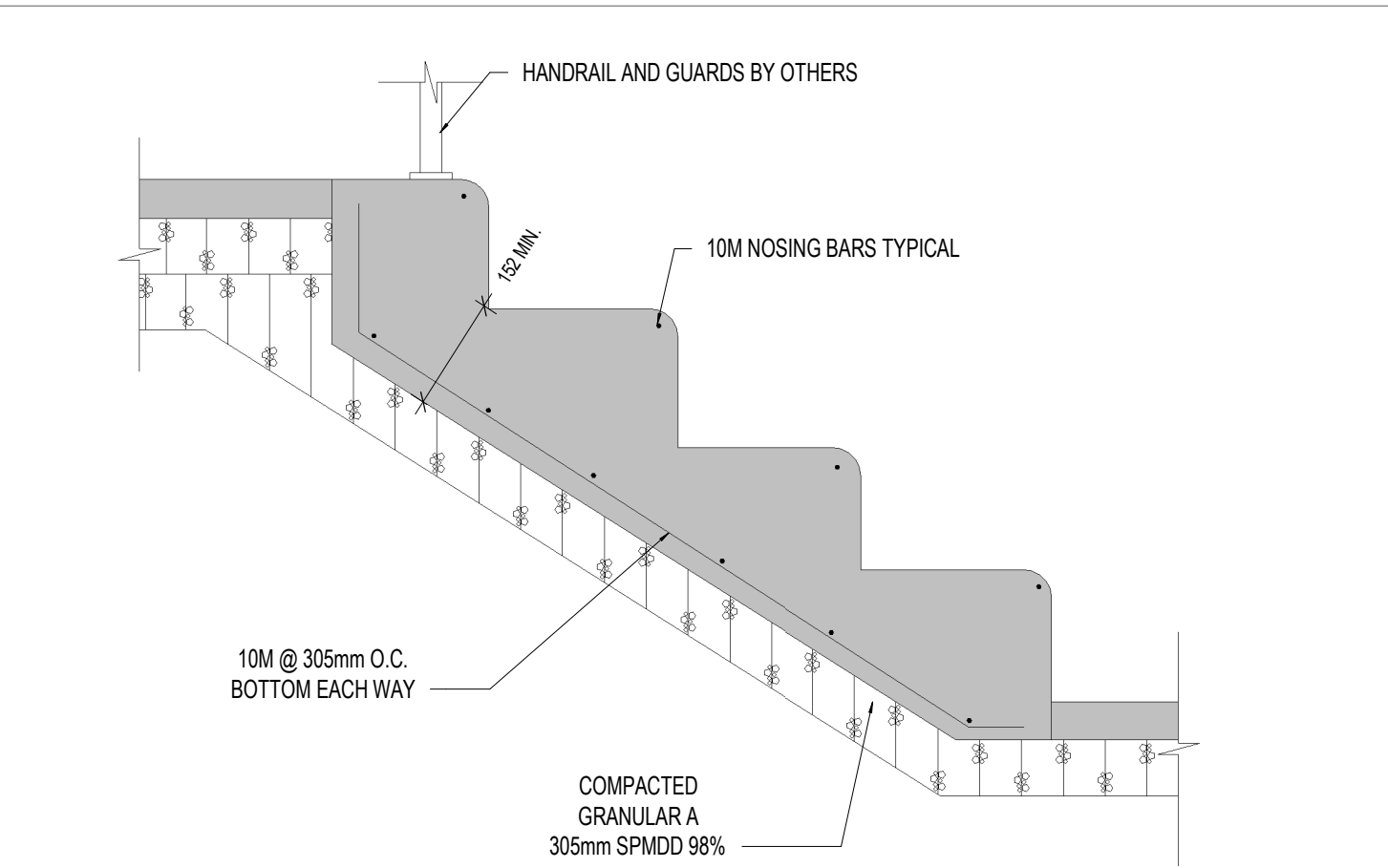
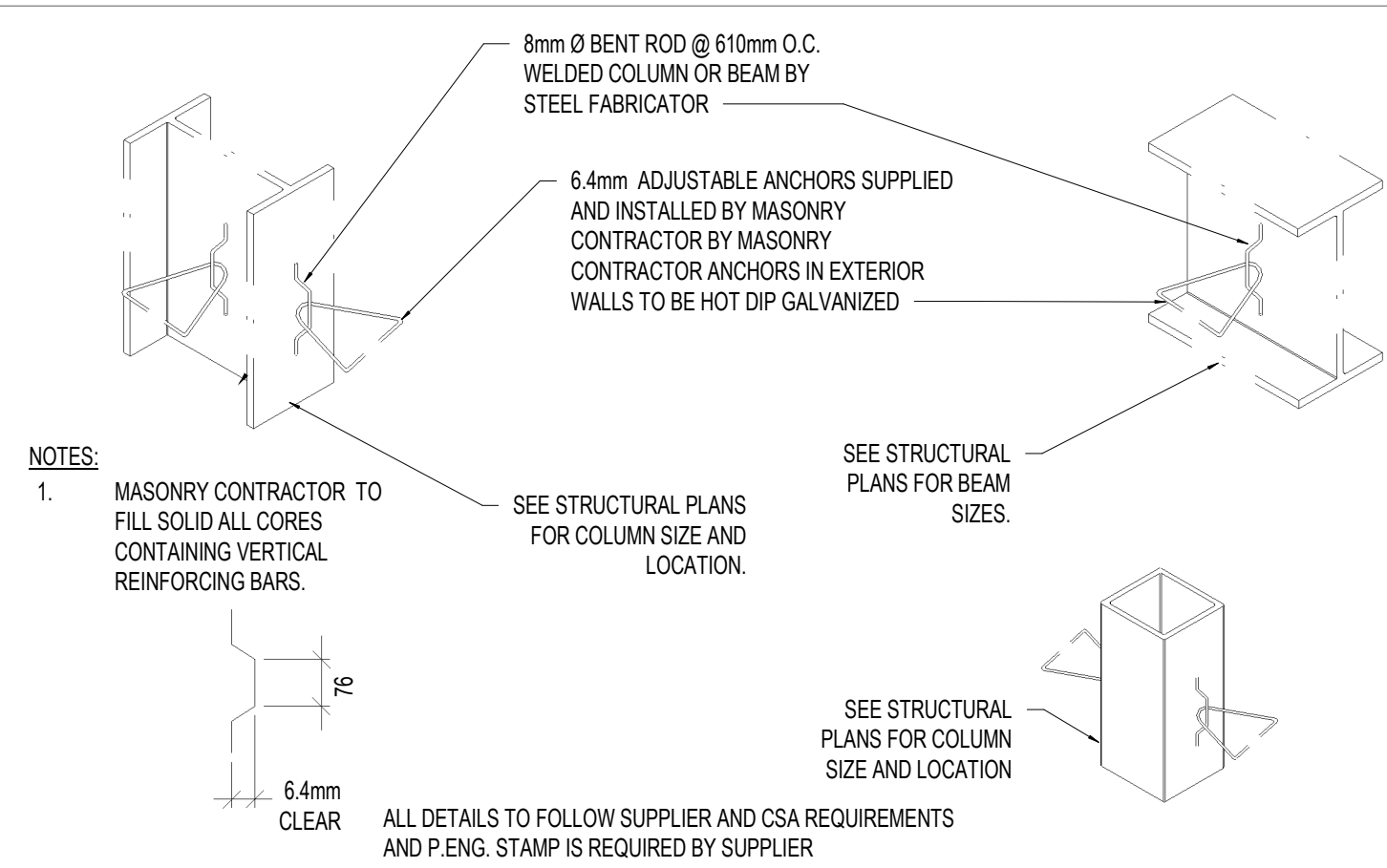
No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Apr 23 2024	ISSUED FOR CONSTRUCTION

ISSUES/REVISION TABLE		
No.	Date	Revision

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DEVELOPMENT CORP
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TYPICAL DETAILS I

Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102
Drawing No: Revision

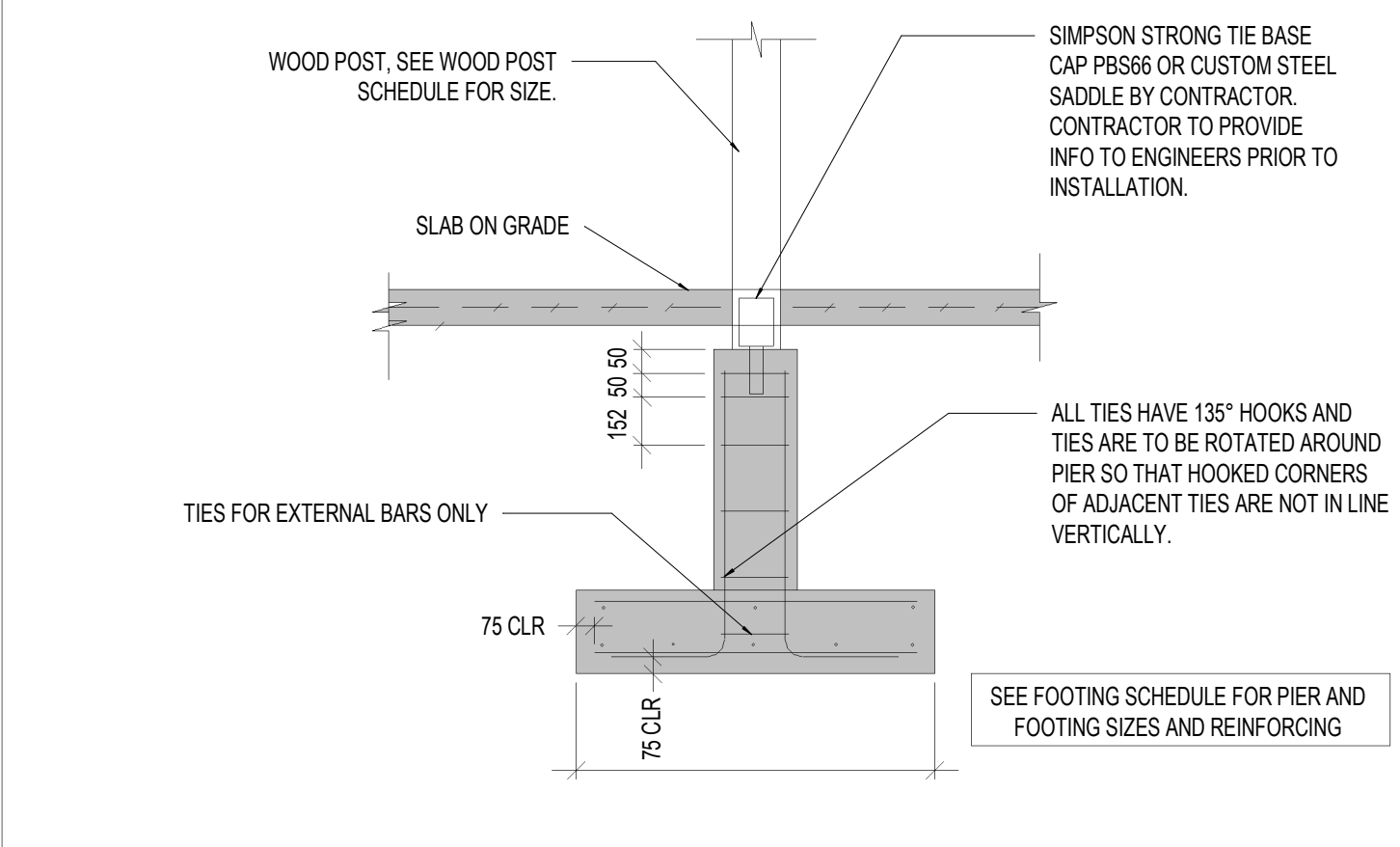
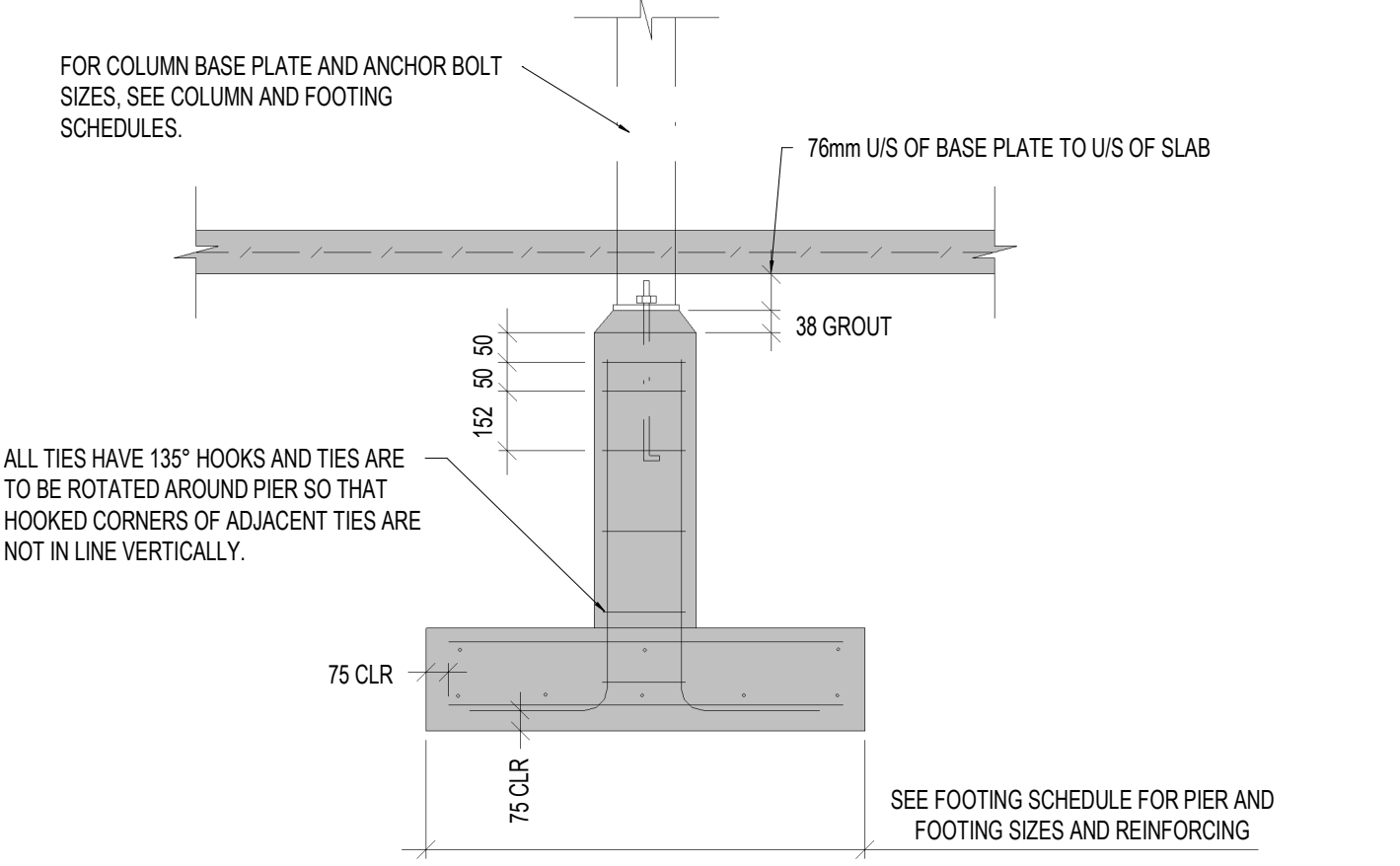
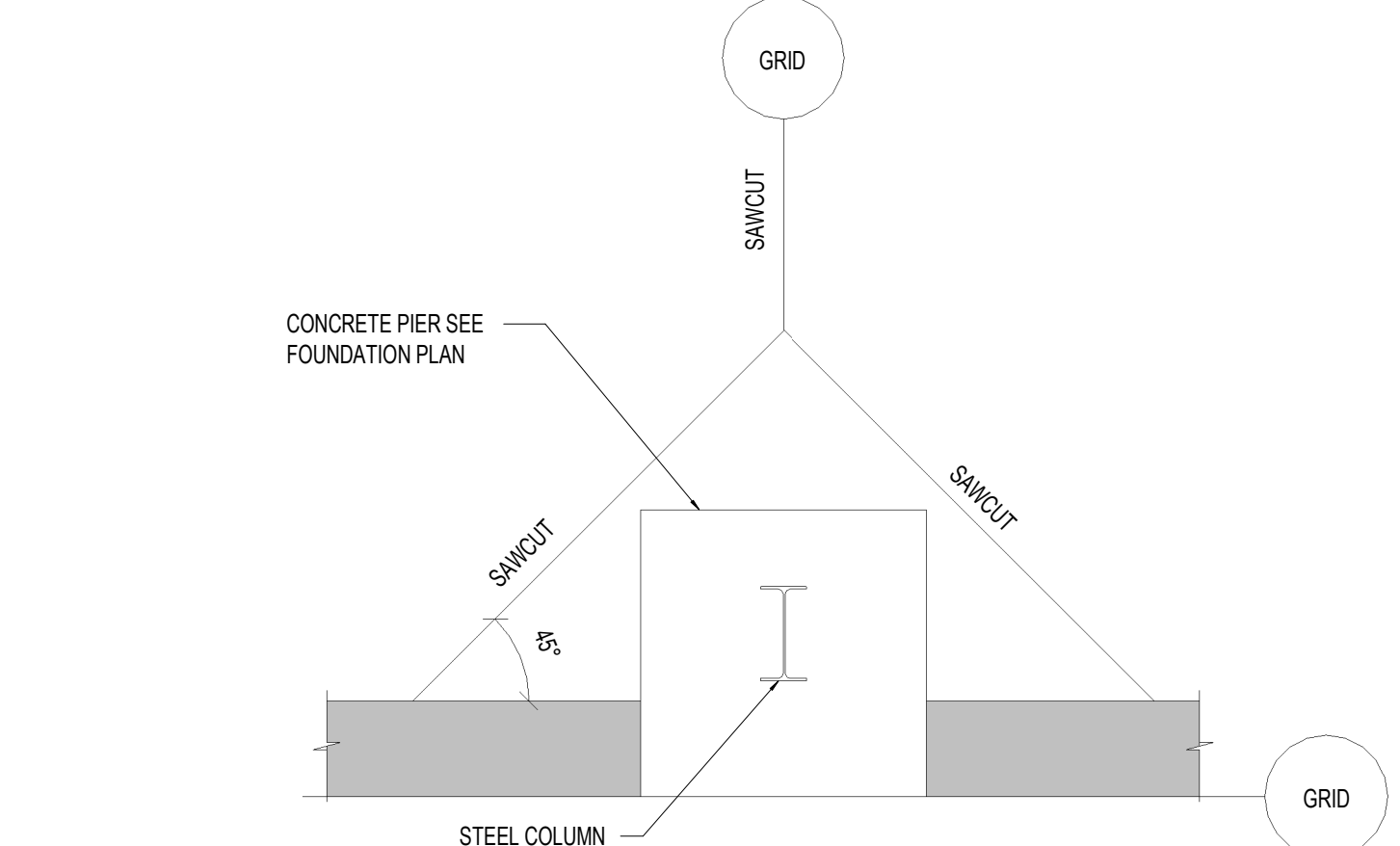
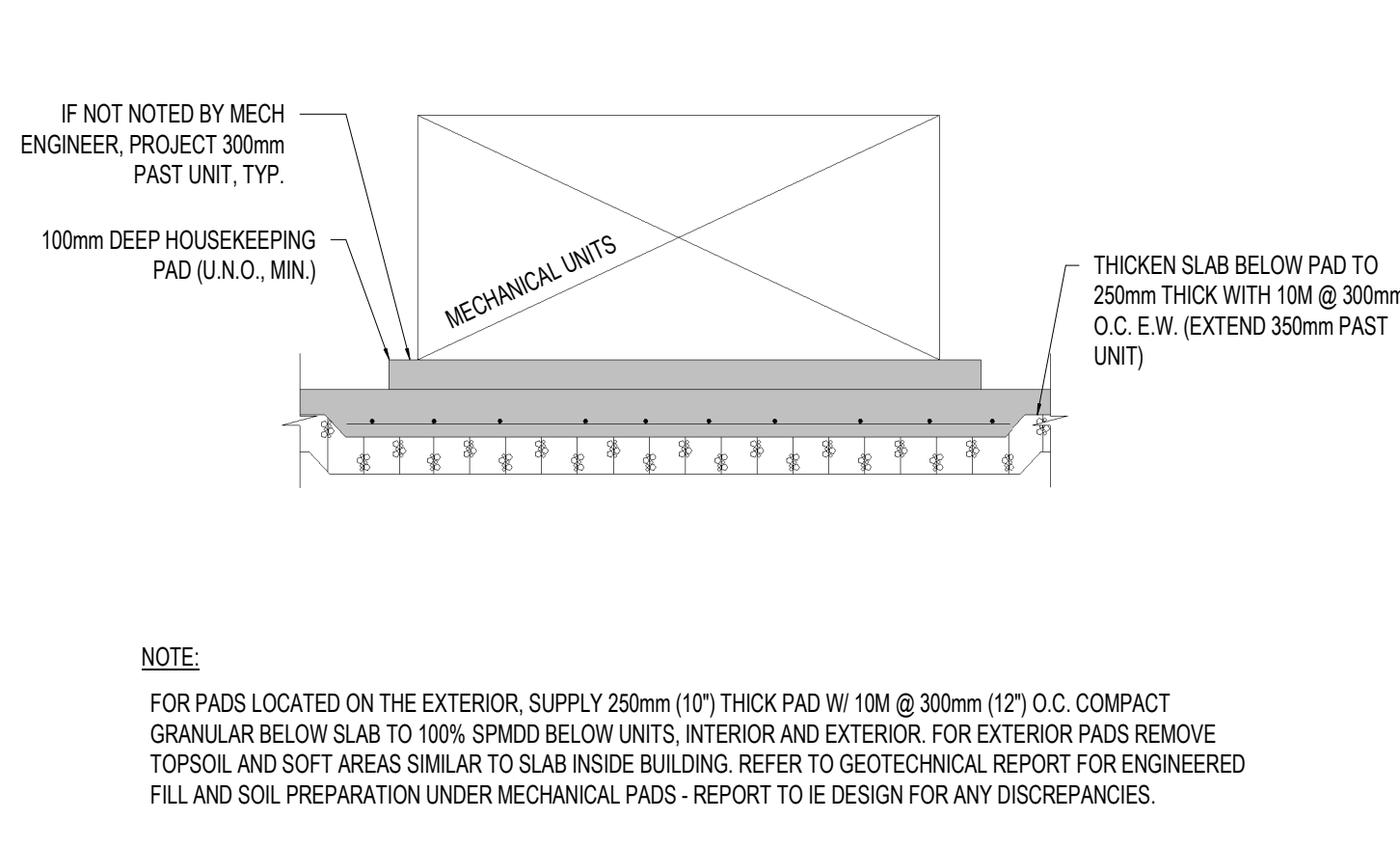


T.D.18 **TYPICAL MASONRY ANCHORAGE TO STEEL**

T.D.19 **CONCRETE STEP/STAIR REINFORCING**

T.D.23 **RECESSED FLOOR DRAIN**

T.D.27 **RECESSED FLOOR ENTRY GRILL**

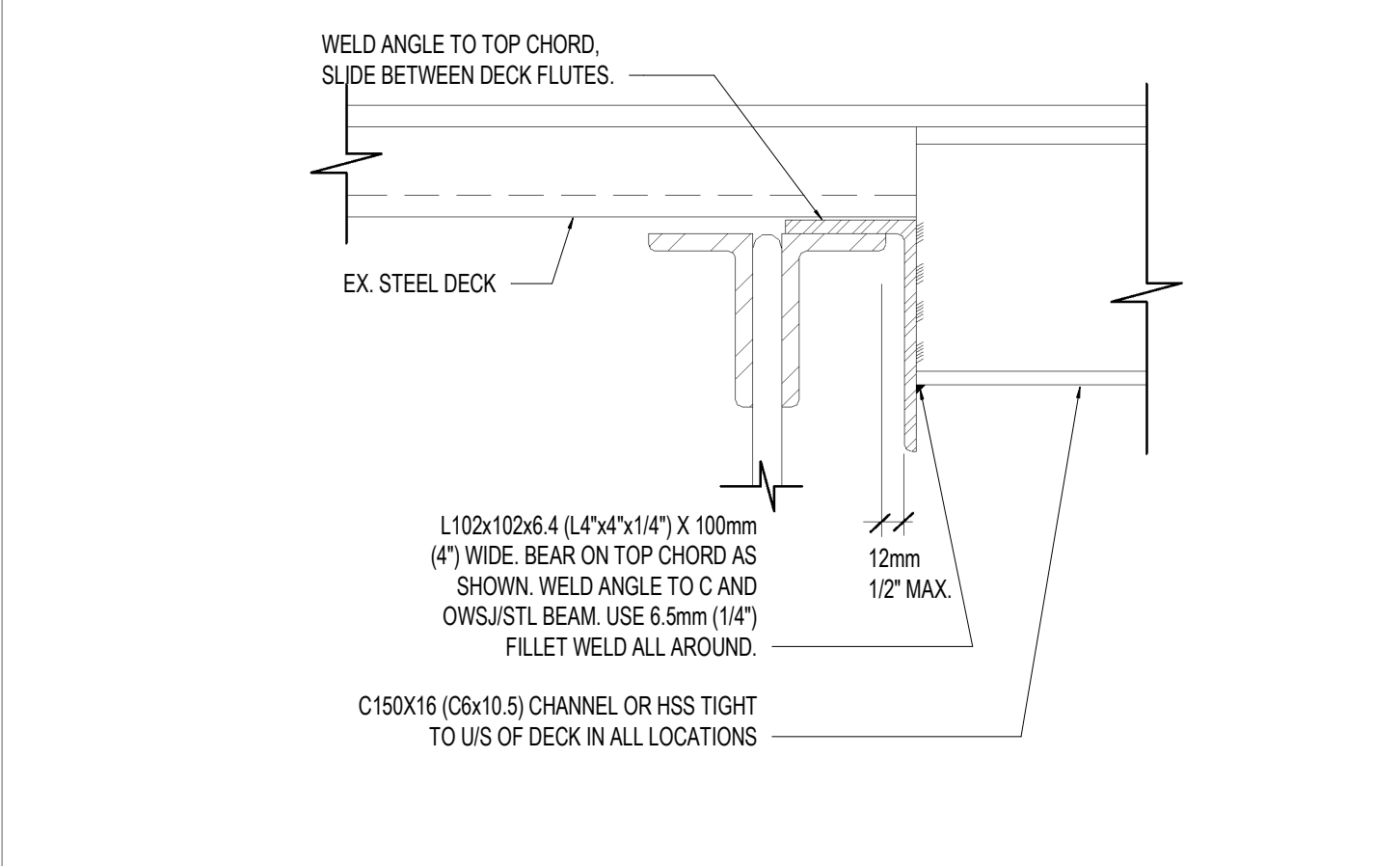
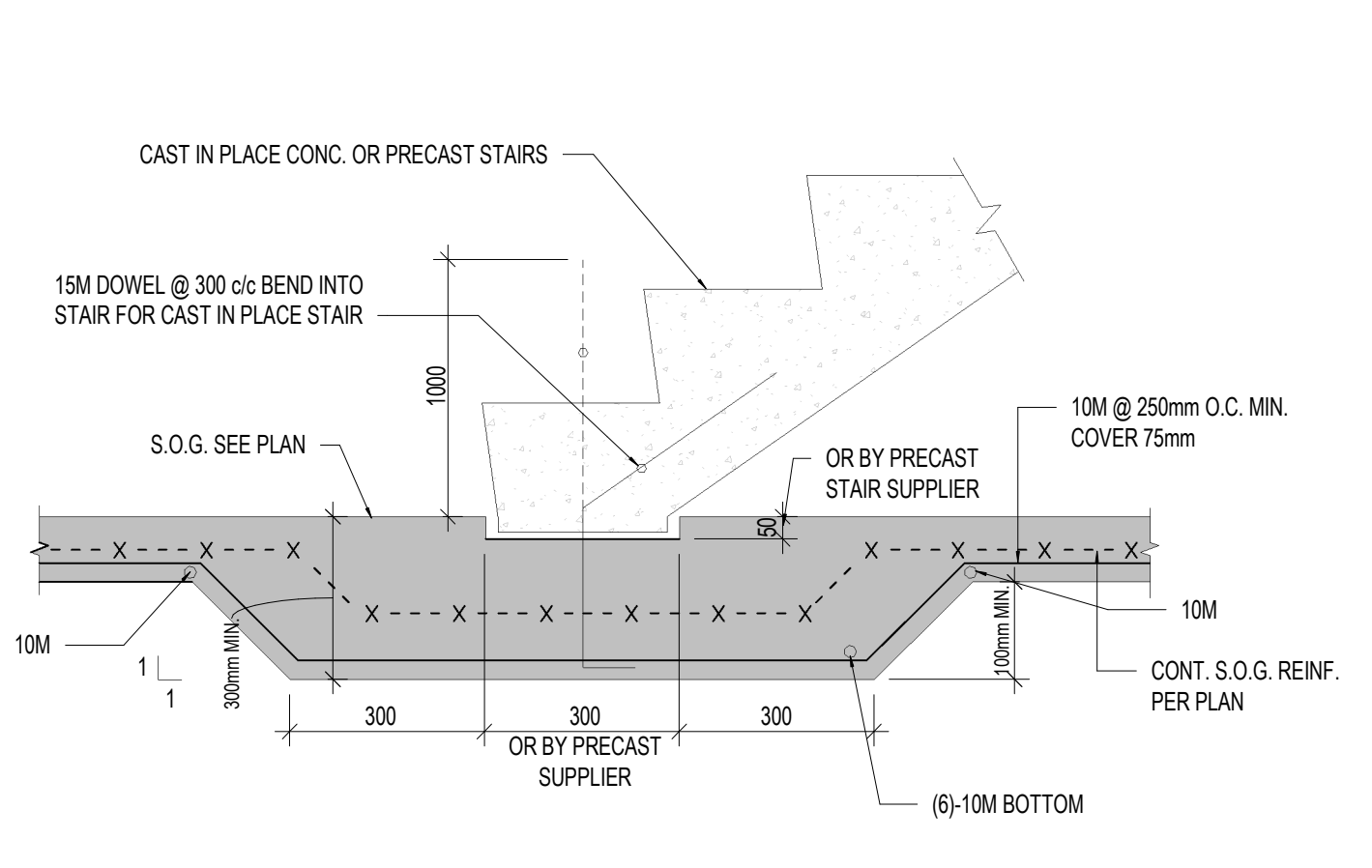
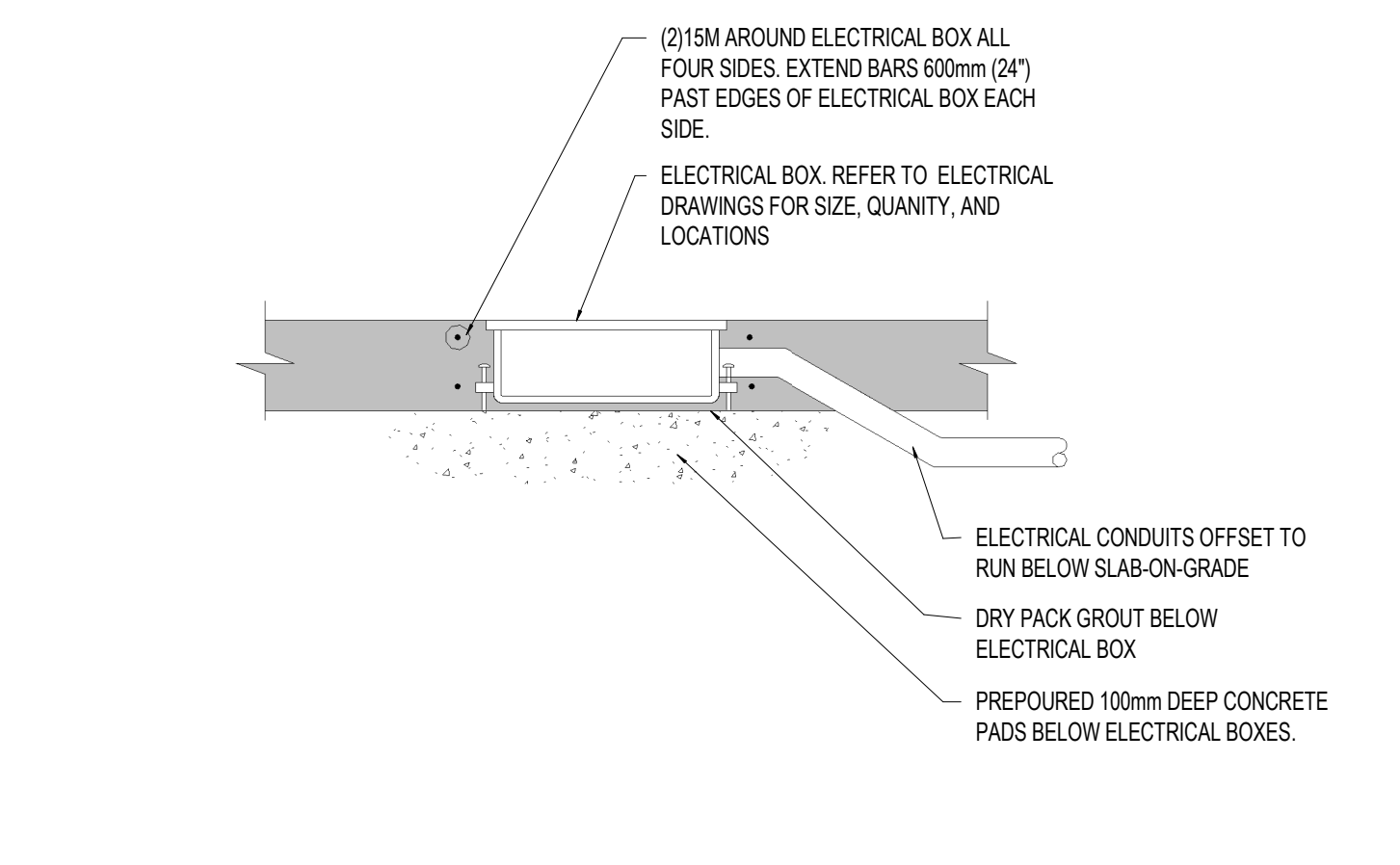
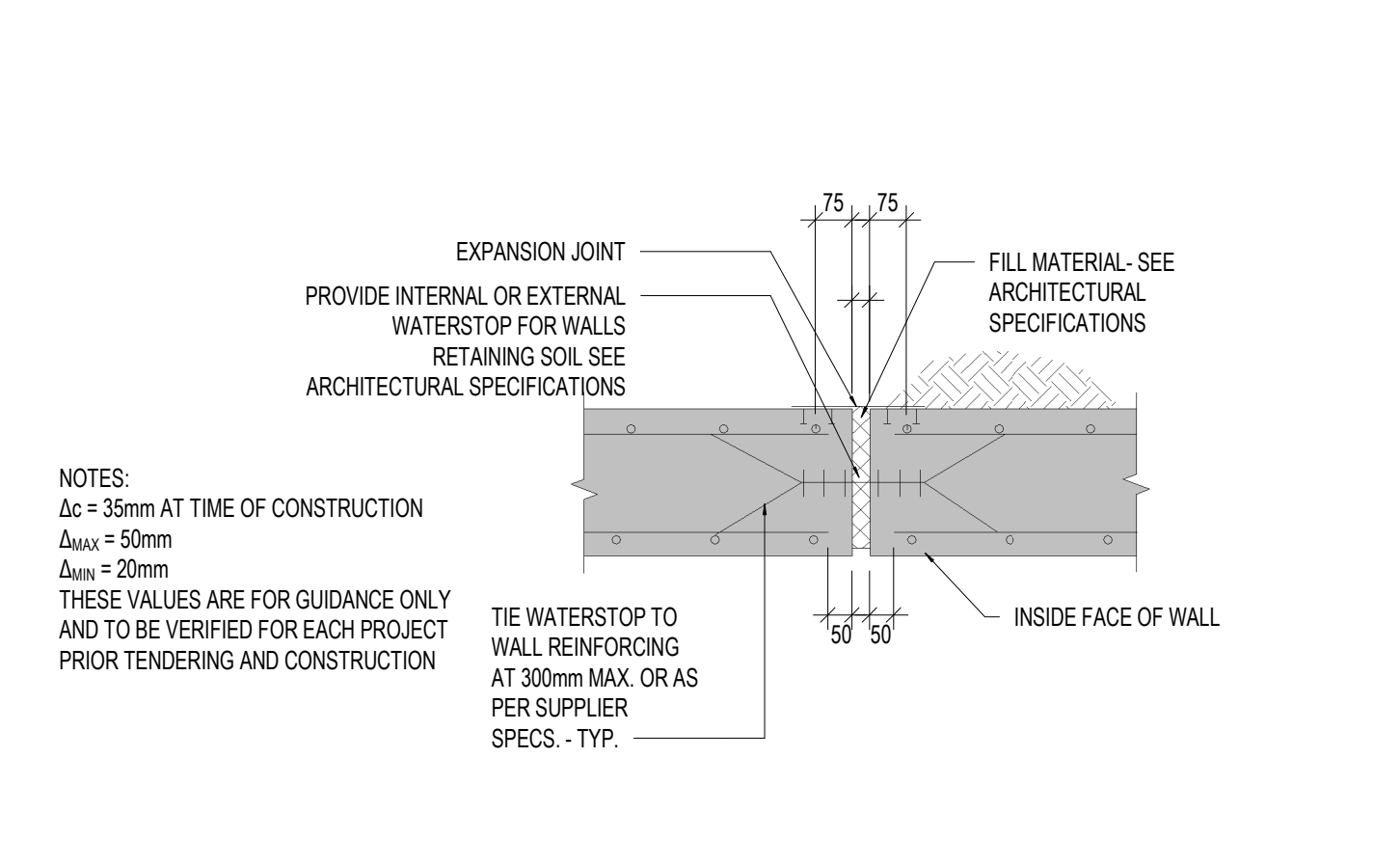


T.D.25 **SLAB THICKENING FOR MECHANICAL UNITS (EXTERIOR)**

T.D.26 **CONTROL JOINT @ PIER**

T.D.28 **TYPICAL SQUARE FOOTING FOR STEEL POST**

T.D.29 **TYPICAL SQUARE FOOTING FOR WOOD POST**

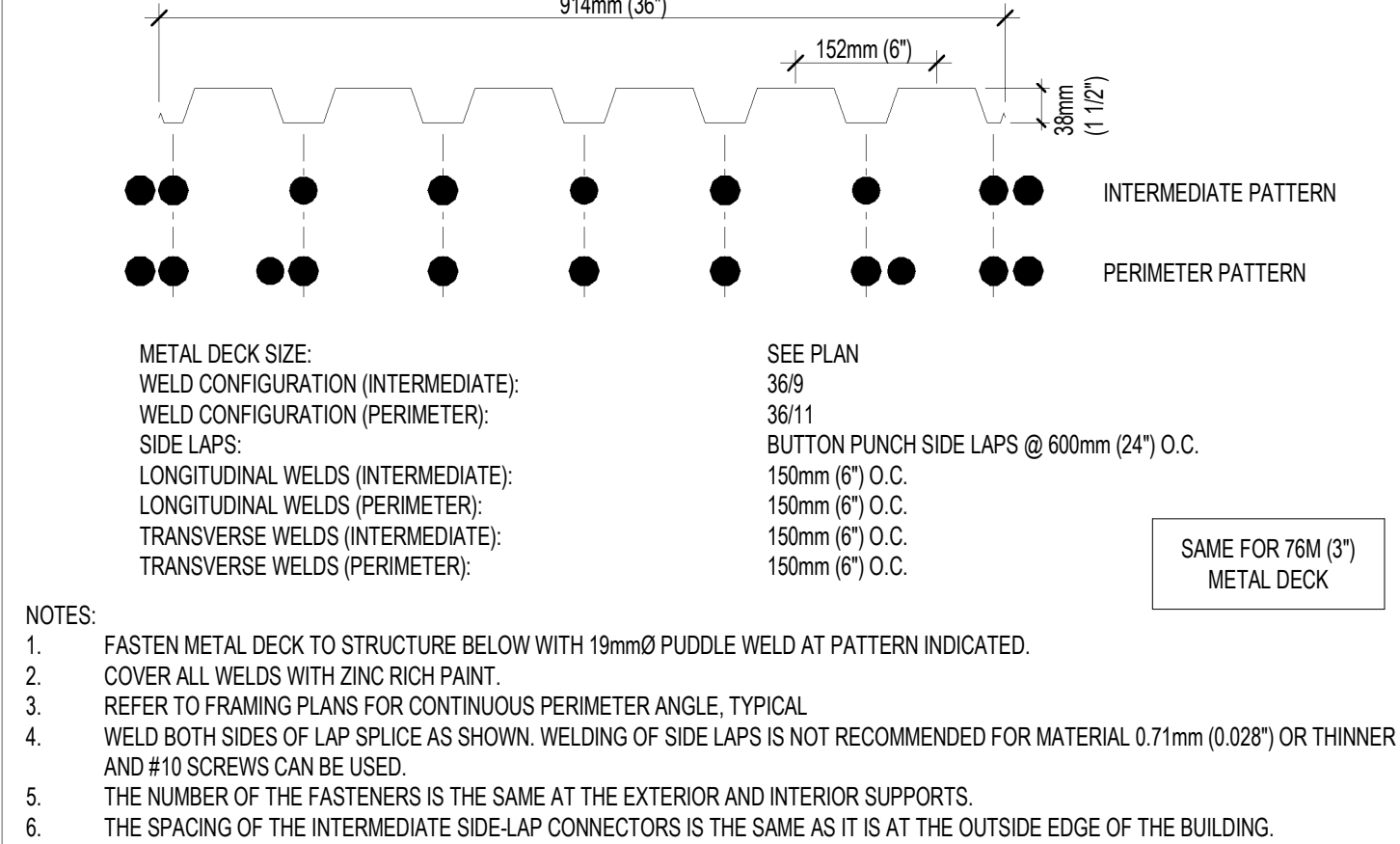
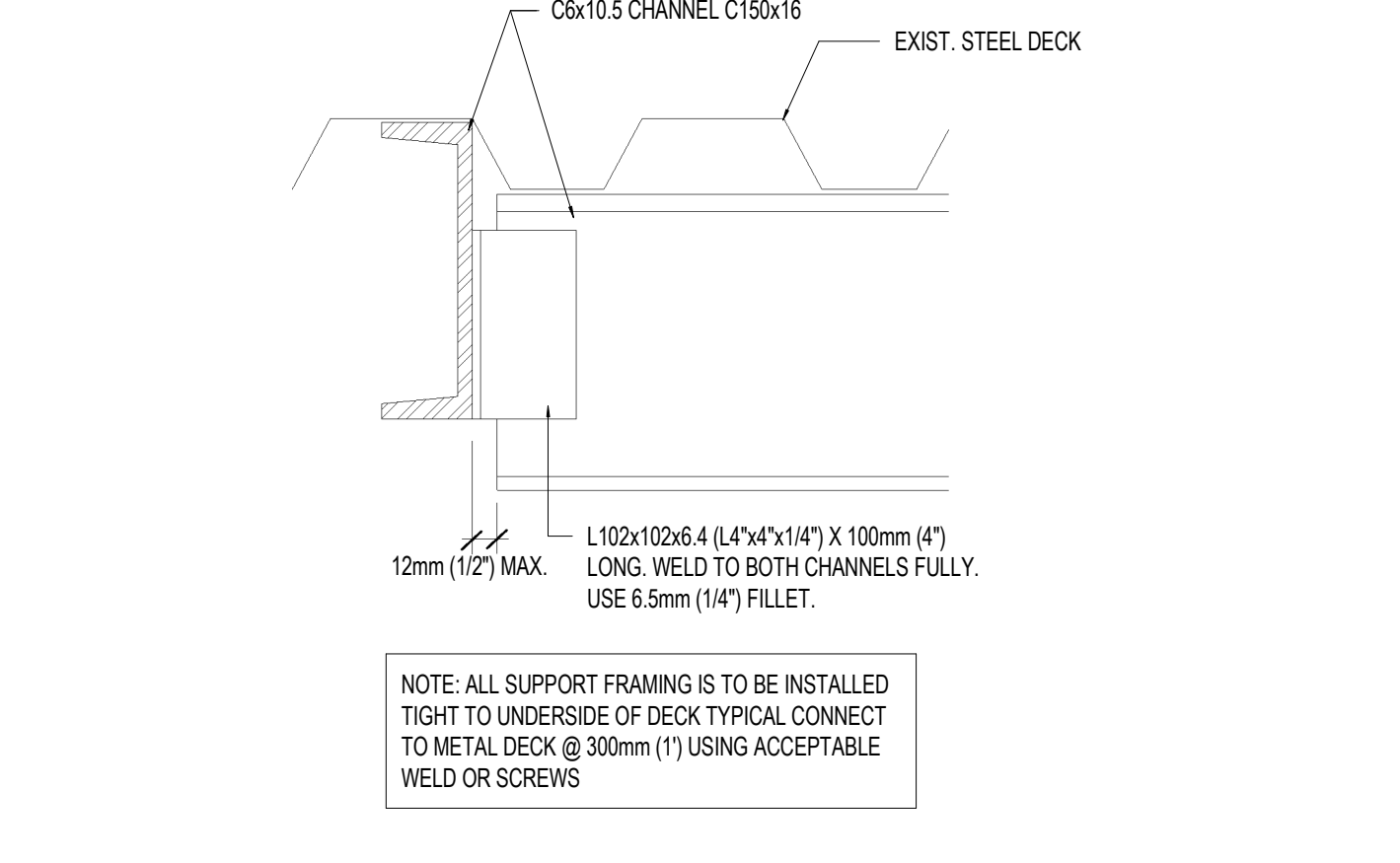
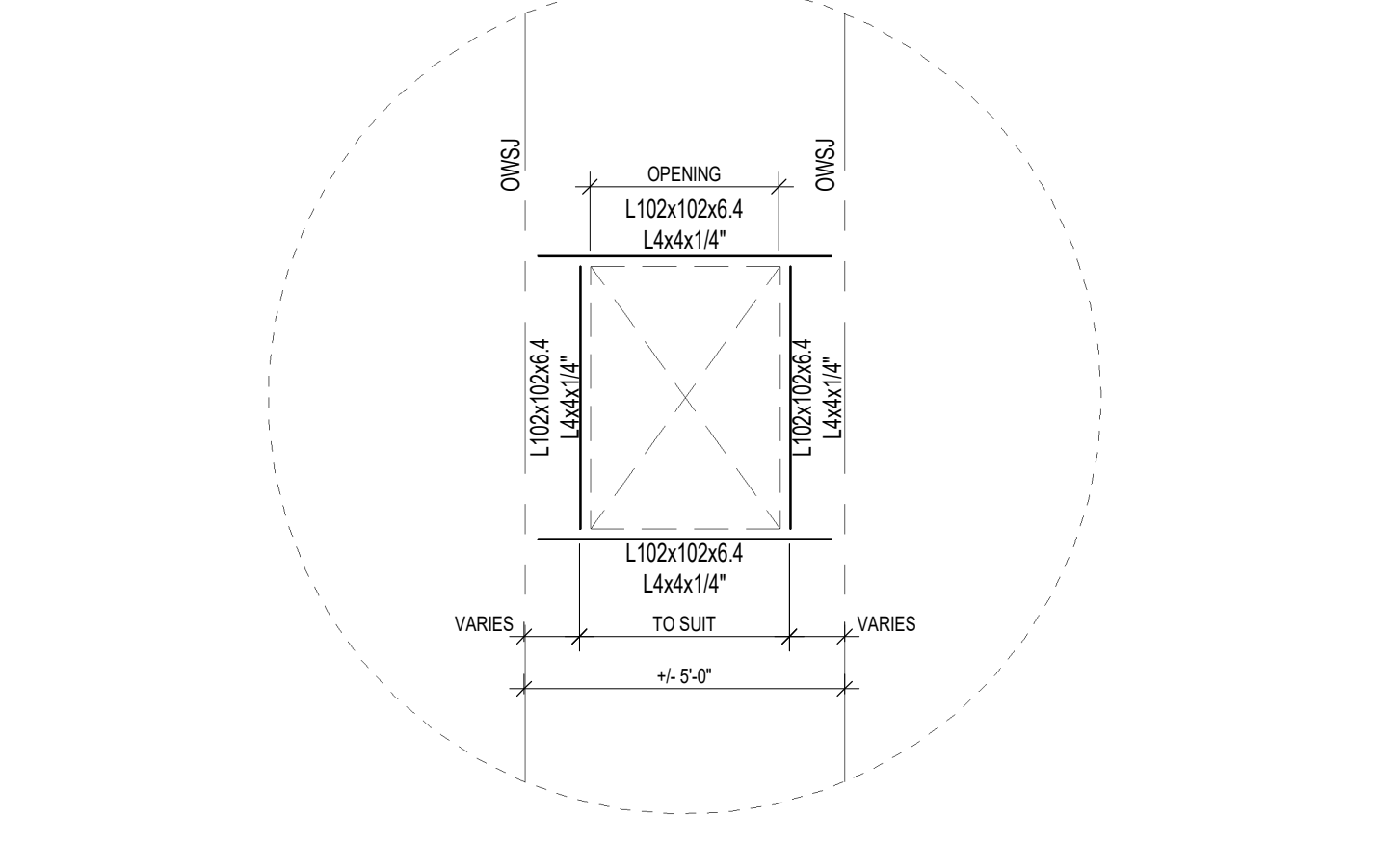
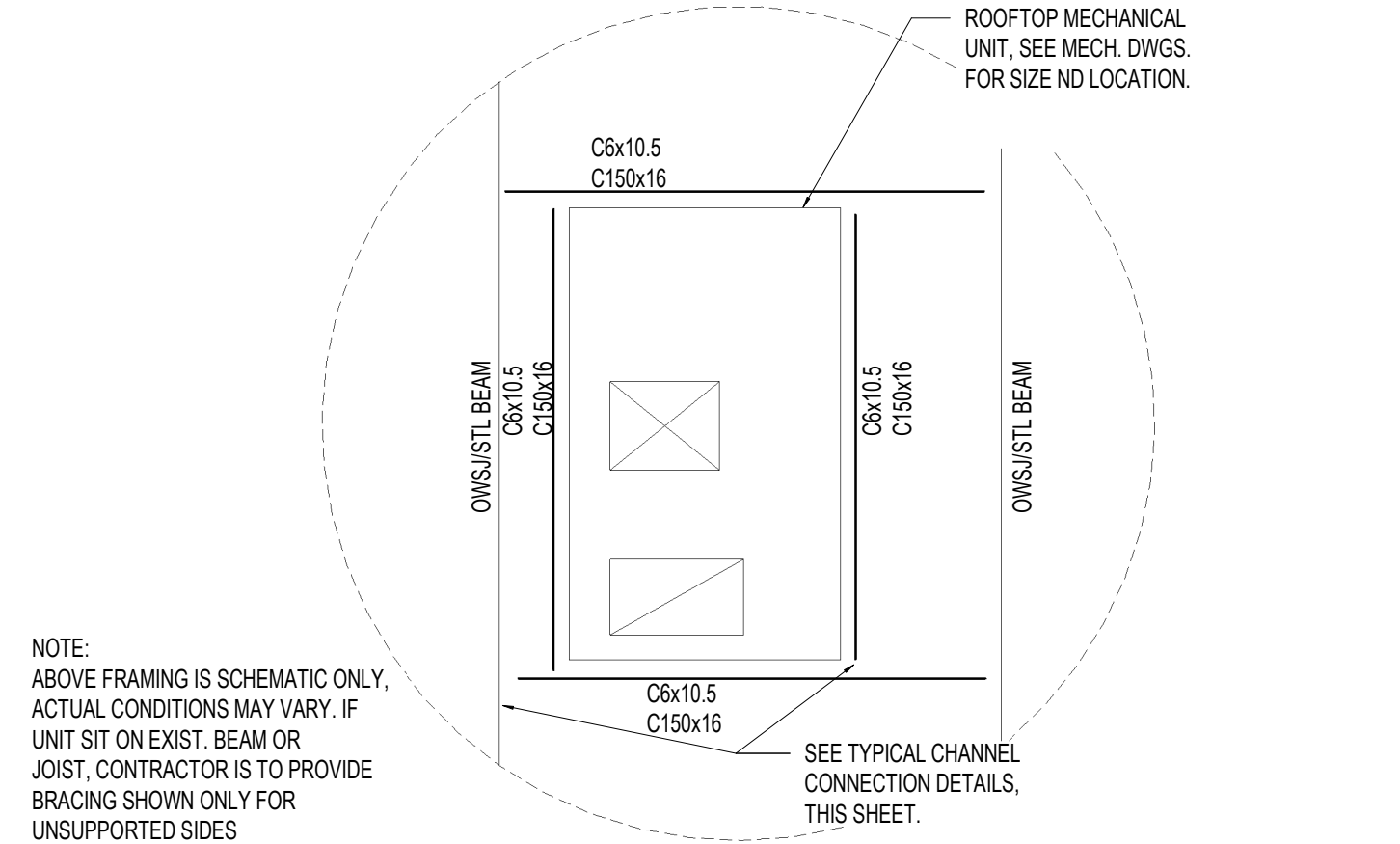


T.D.33 **WALL EXPANSION JOINT**

T.D.41 **ELECTRICAL FLOOR BOXES IN SLAB-ON-GRADE**

T.D.47 **THICKENING SLAB ON GRADE AT CONCRETE STAIRS**

S.2 **CHANNEL TO JOIST CONNECTION**

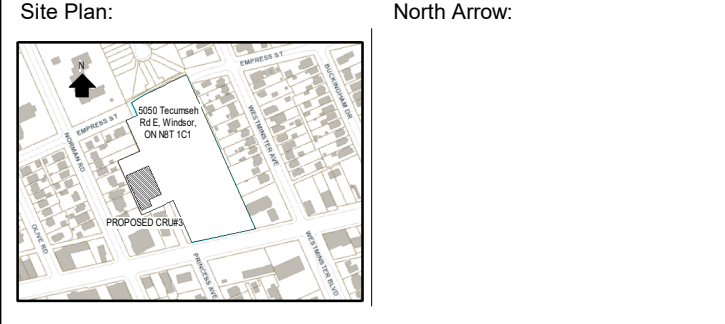
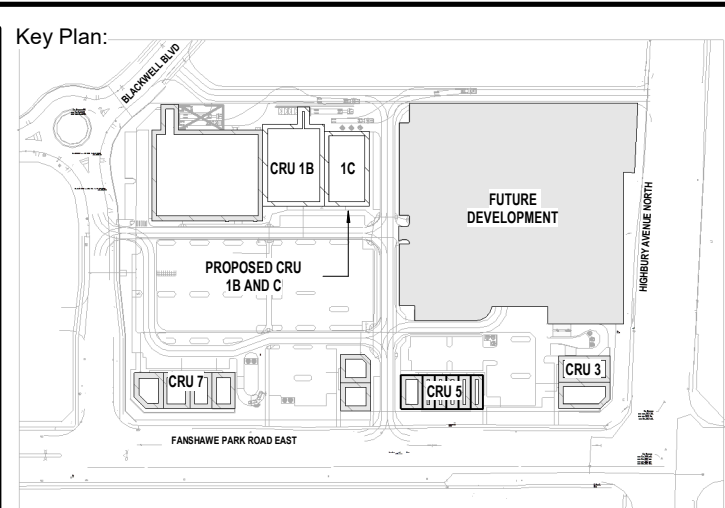


S.5 **SUPPORT FRAMING FOR ROOF TOP MECHANICAL UNITS**

S.6 **SUPPORT FRAMING FOR EXHAUST FAN/ACCESS HATCH OPENINGS**

S.9 **CHANNEL TO CHANNEL CONNECTION**

S.11 **ROOF DECK ATTACHMENT**



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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 23 2024	ISSUED FOR COORDINATION

ISSUES/REVISION TABLE

Project:

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Drawing Title: **TYPICAL DETAILS II**

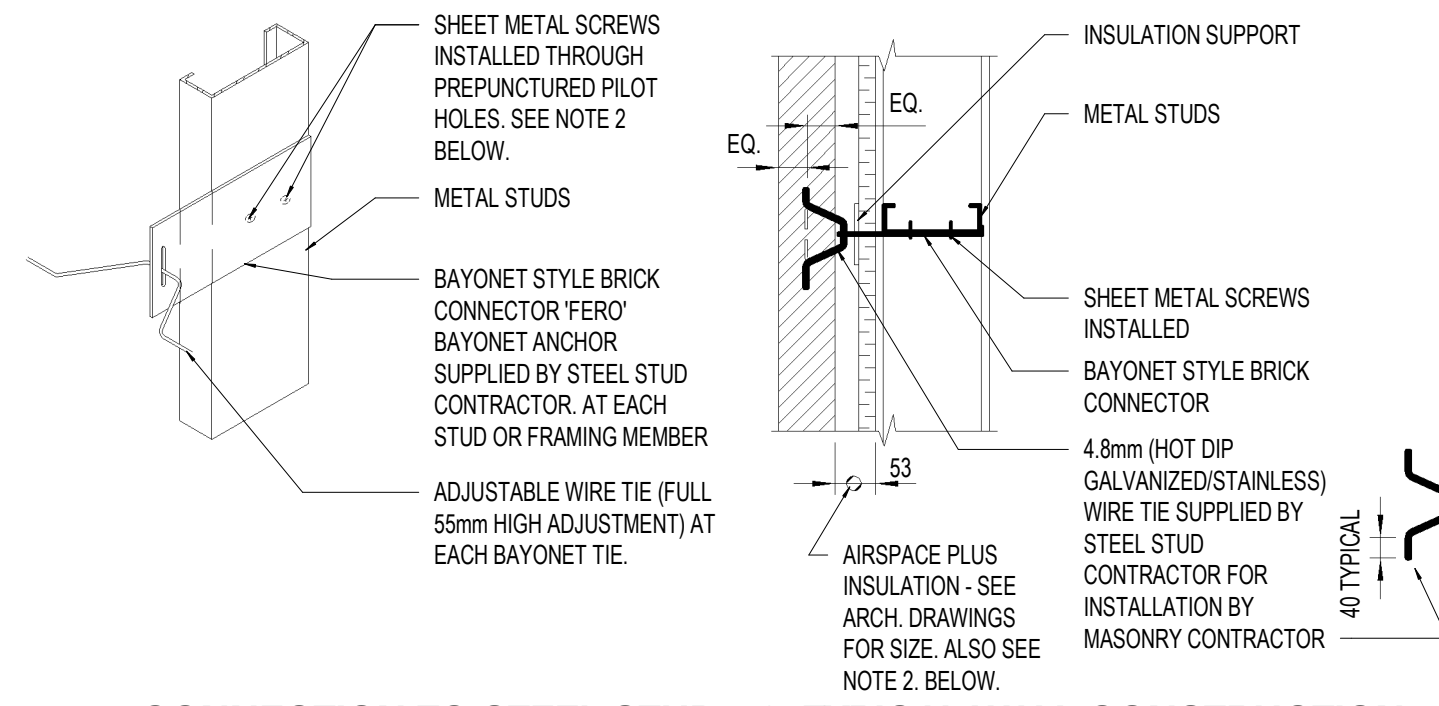
Drawn By: **D.H./S.D./D.K.** Scale: **AS INDICATED**

Checked By: **M.A.F./J.G.** Plot Date: **APR. 03-2024**

Project Date: **AUG. 2023**

Project No: **2023-102**

Drawing No: **S-6.1** Revision: **2**



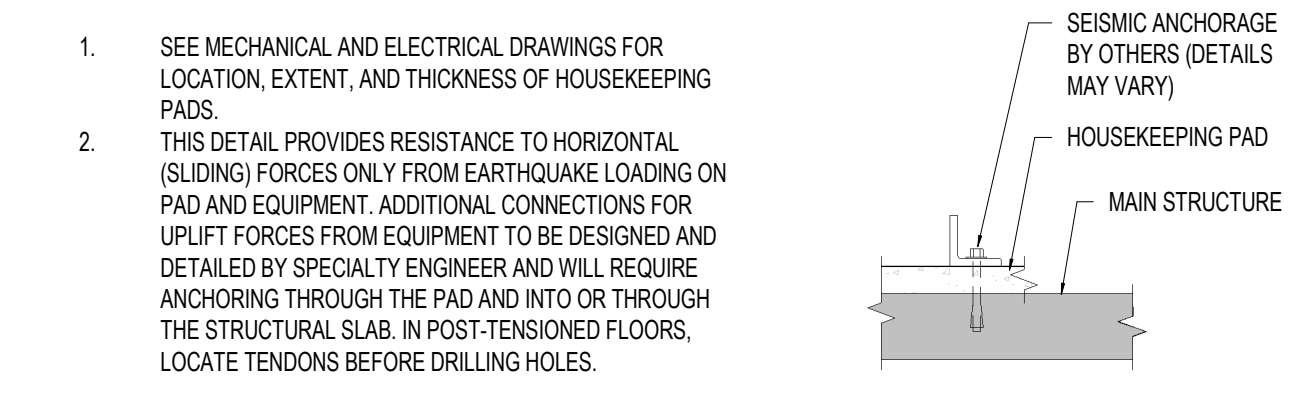
1. CONNECTION TO STEEL STUD 2. TYPICAL WALL CONSTRUCTION

ANCHOR SPACING TABLE	
STUD FRAMING HORIZONTAL SPACING	MAXIMUM VERTICAL ANCHOR SPACING
FRAMING AT 600 O.C.	400mm O.C.
FRAMING AT 400 O.C.	600mm O.C.
FRAMING AT 300 O.C.	600mm O.C.
AT JAMB LOCATIONS OF ALL WALL OPENINGS	300mm O.C.

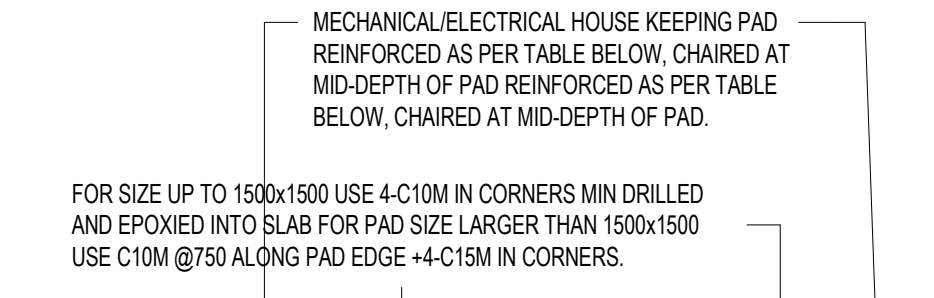
ALL DETAILS TO FOLLOW SUPPLIER AND CSA REQUIREMENTS AND P.ENG. STAMP IS REQUIRED BY SUPPLIER

- BAYONET TIES TO BE FERRO SLOTTED TIE (TYPE I) COMPLETE WITH 4.8mm DIAMETER (HOT DIPPED GALVANIZED/STAINLESS) V-ANCHOR AND INSULATION SUPPORT. NOTE THAT INSULATION SUPPORT IS TO BE PLACED OVER AIR BARRIER SYSTEM.
- MINIMUM GAUGE OF TIE SYSTEM TO BE 16 GAUGE MATERIAL FOR CAVITIES UP TO 127mm WIDE. ANCHOR SUPPLIER TO DESIGN TIE WITH INCREASED THICKNESS FOR WIDER CAVITIES. DESIGN OF ANCHOR TO ASSUME MAXIMUM 0.78kN UNFACTORED ANCHOR LOAD WITH NO LATERAL SUPPORT OF PLATE SUPPLIED BY INSULATION OR EXTERIOR SHEATHING.
- ALL TIE COMPONENTS ARE TO HAVE A MINIMUM CORROSION PROTECTION SUPPLIED (HOT DIP GALVANIZING/STAINLESS) AFTER FABRICATION. IF PLATE SYSTEM IS TO BE GALVANIZED, FOLLOW CSA-CAN3-A370 AND ASTM A123 REQUIREMENT OF 401 g/m². V TIE GALVANIZING TO BE IN ACCORDANCE TO CSA CAN-A370 AND ASTM A153 REQUIREMENT OF 458 g/m². REFER TO SPECIFICATIONS IF STAINLESS STEEL REQUIREMENT SUPERSEDES MINIMUM GALVANIZED COATING REQUIREMENT.
- STEEL STUD CONTRACTOR IS TO CONSTRUCT A STOREY ROD OF MASONRY COURSING FOR ALL MASONRY VENEER WALLS TO ENSURE ANCHORS ARE INSTALLED AT THE CENTRE OF ALL MASONRY JOINTS TO RECEIVE A MASONRY TIE. IF MASONRY UNIT LAYOUT DOES NOT ALLOW PLACEMENT OF ANCHORS AT THE MAXIMUM VERTICAL SPACING SHOWN IN ABOVE TABLE, EXTRA TIES MUST BE INSTALLED ABOVE AND BELOW EACH MASONRY UNIT AT SMALLER VERTICAL SPACING SUCH THAT THE MAX VERTICAL SPACING IS NOT EXCEEDED.

T.D.46 ADJUSTABLE BAYONET STYLE MASONRY VENEER ANCHORS AT METAL STUD WALLS



- SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION, EXTENT, AND THICKNESS OF HOUSEKEEPING PADS.
- THIS DETAIL PROVIDES RESISTANCE TO HORIZONTAL (SLIDING) FORCES ONLY FROM EARTHQUAKE LOADING ON PAD AND EQUIPMENT. ADDITIONAL CONNECTIONS FOR UPLIFT FORCES FROM EQUIPMENT TO BE DESIGNED AND DETAILED BY SPECIALTY ENGINEER AND WILL REQUIRE ANCHORING THROUGH THE PAD AND INTO OR THROUGH THE STRUCTURAL SLAB. IN POST-TENSIONED FLOORS, LOCATE TENDONS BEFORE DRILLING HOLES.

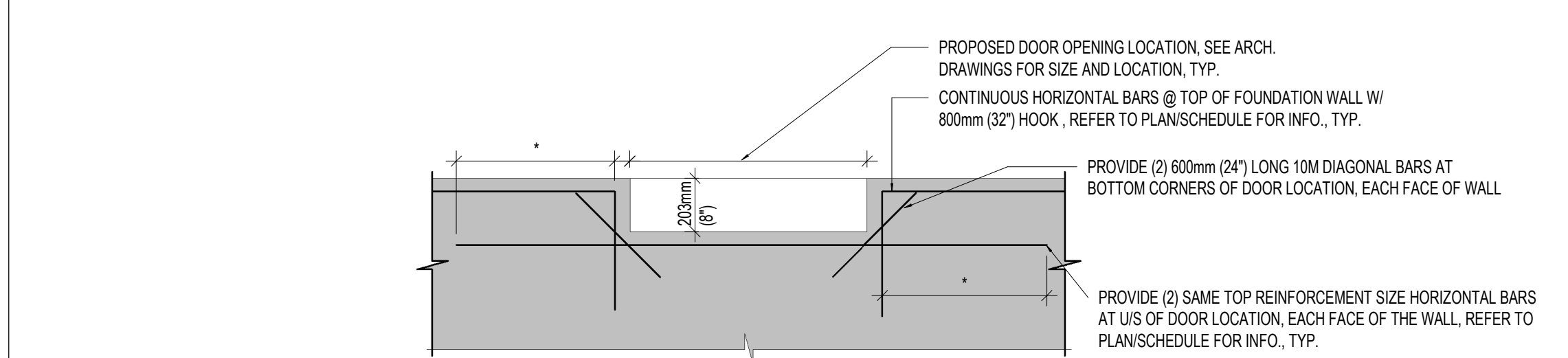


PAD THICKNESS 'T' <= 100 100 < PAD THICKNESS 'T' <= 200

STRUCTURAL SLAB OR COMPOSITE DECK. IF STEEL DECK AND TOPPING, REPLACE C10M WITH 10mm Ø HILTI HDI EMBEDDED 50mm INTO TOPPING C/W 10M HOOKED THREADED BAR.

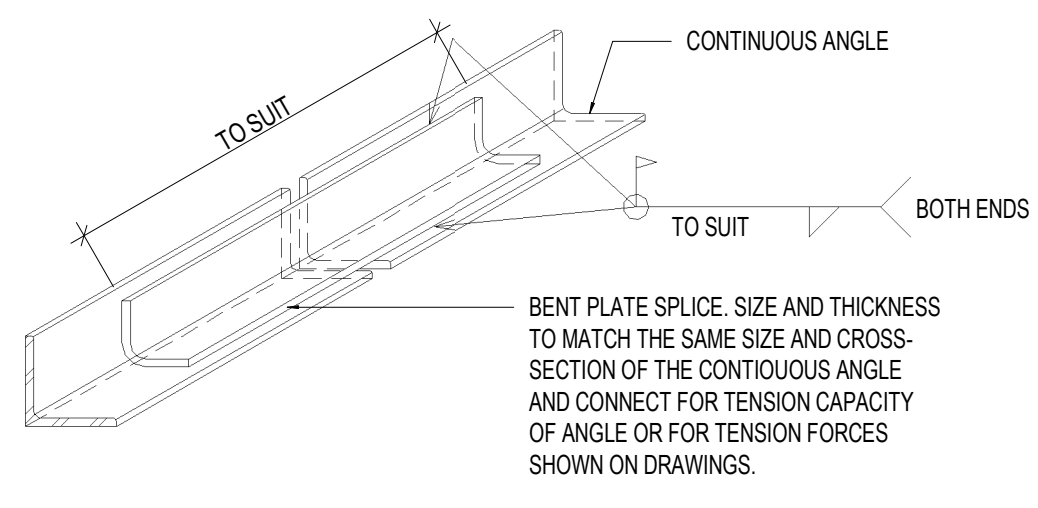
PAD THICKNESS 'T'	REINFORCEMENT
100	152x152MM/18.7MM/18.7MM/1 LAYER
150	10M @300 EW AT CENTER
200	10M @400 T AND BEW

T.D.49 MECHANICAL/ELECTRICAL HOUSEKEEPING PAD AND FLOATING SLAB

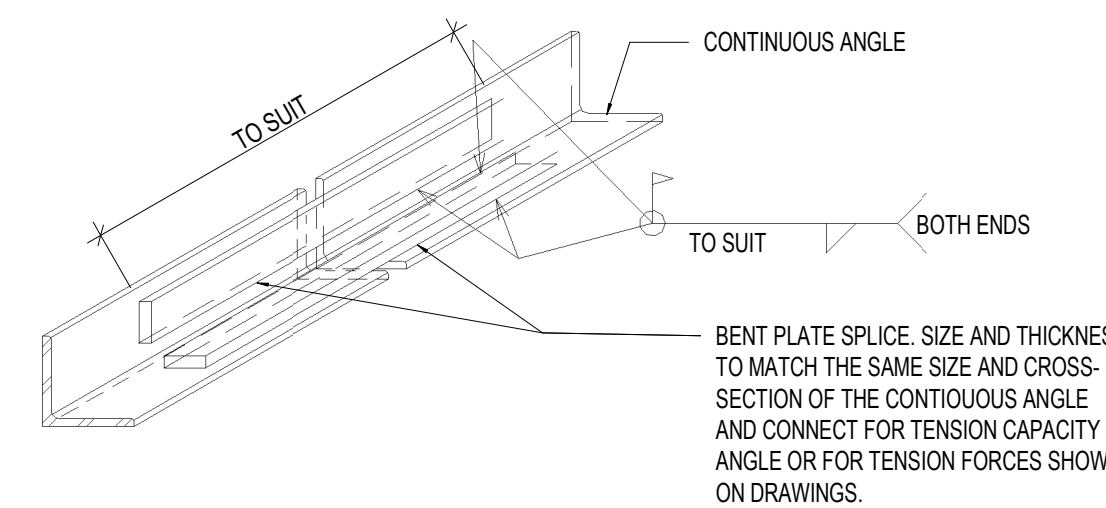


S.12 FOUNDATION WALL DROP @ DOOR OPENING

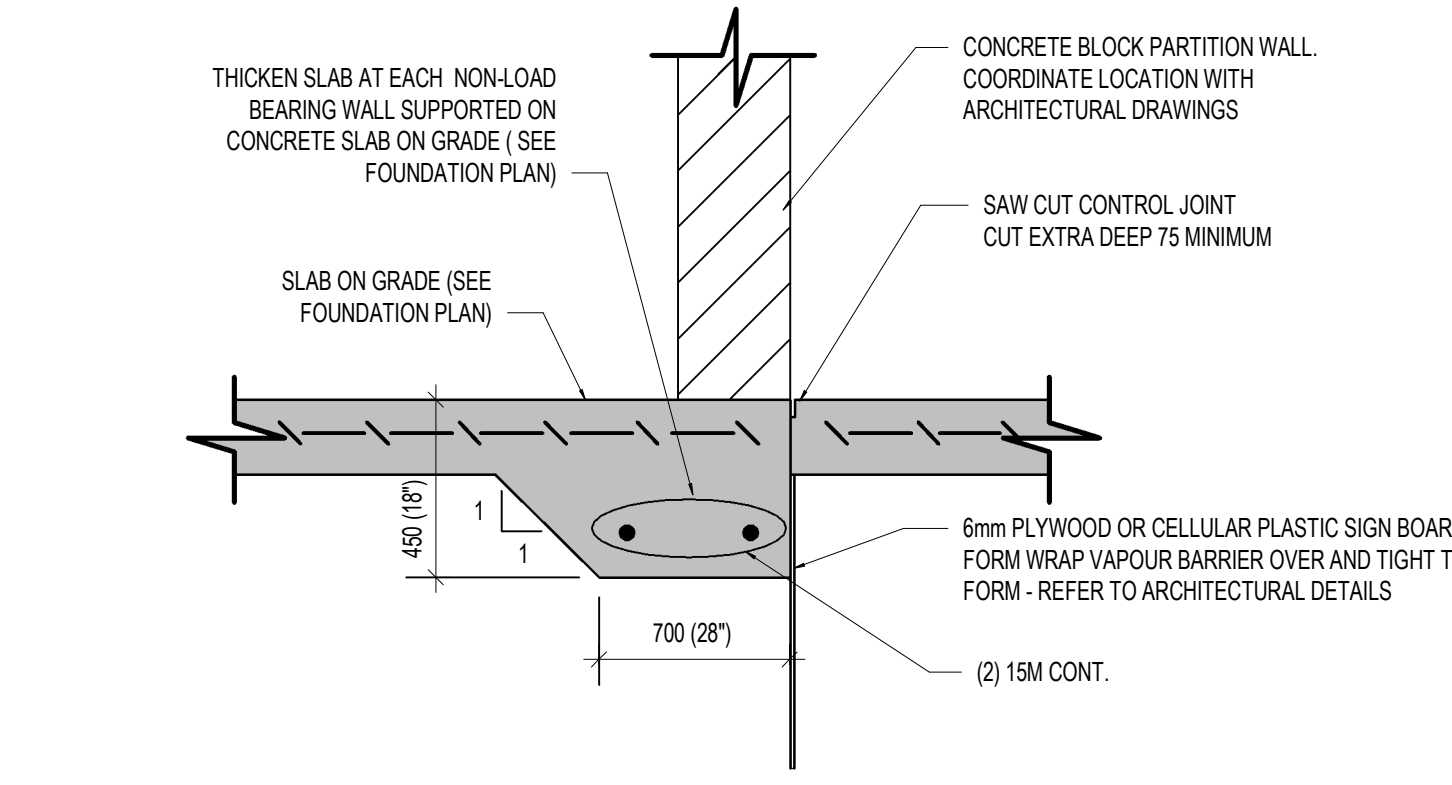
ALTERNATE 1



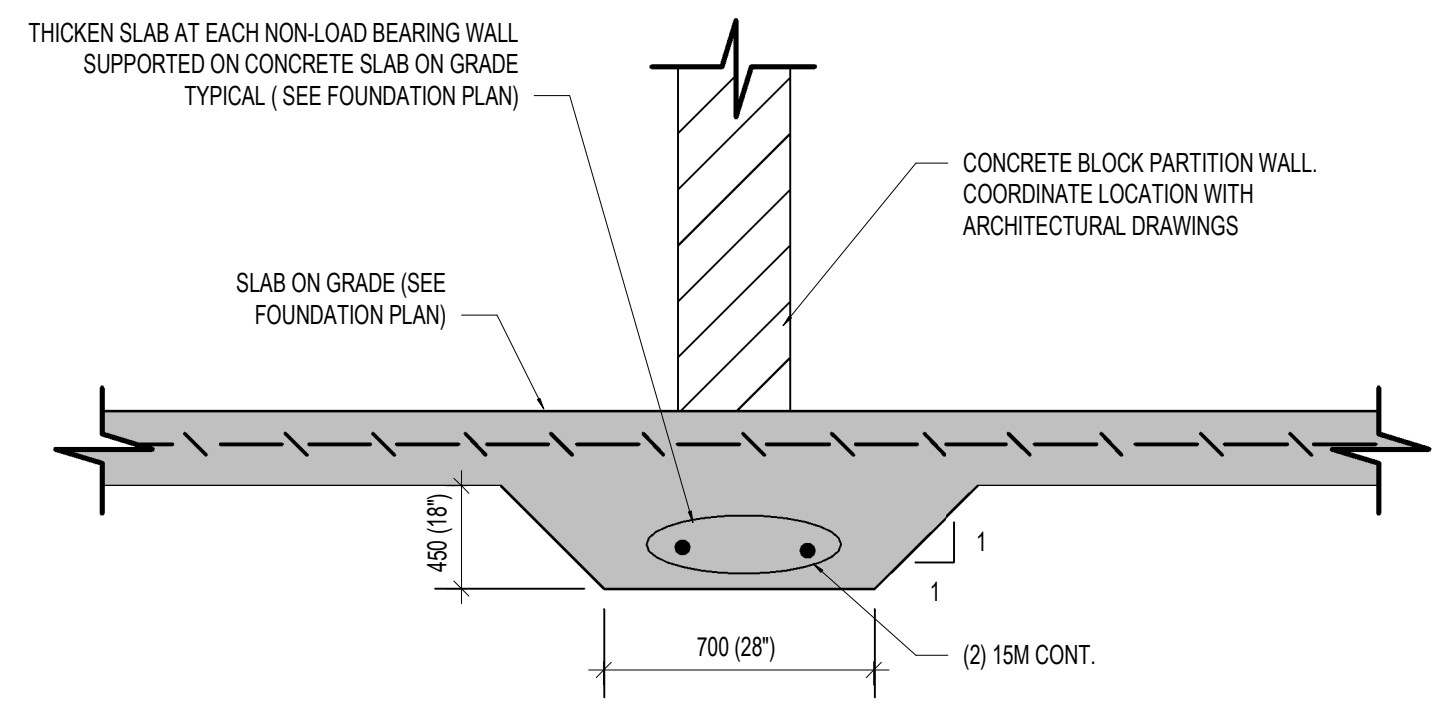
ALTERNATE 2



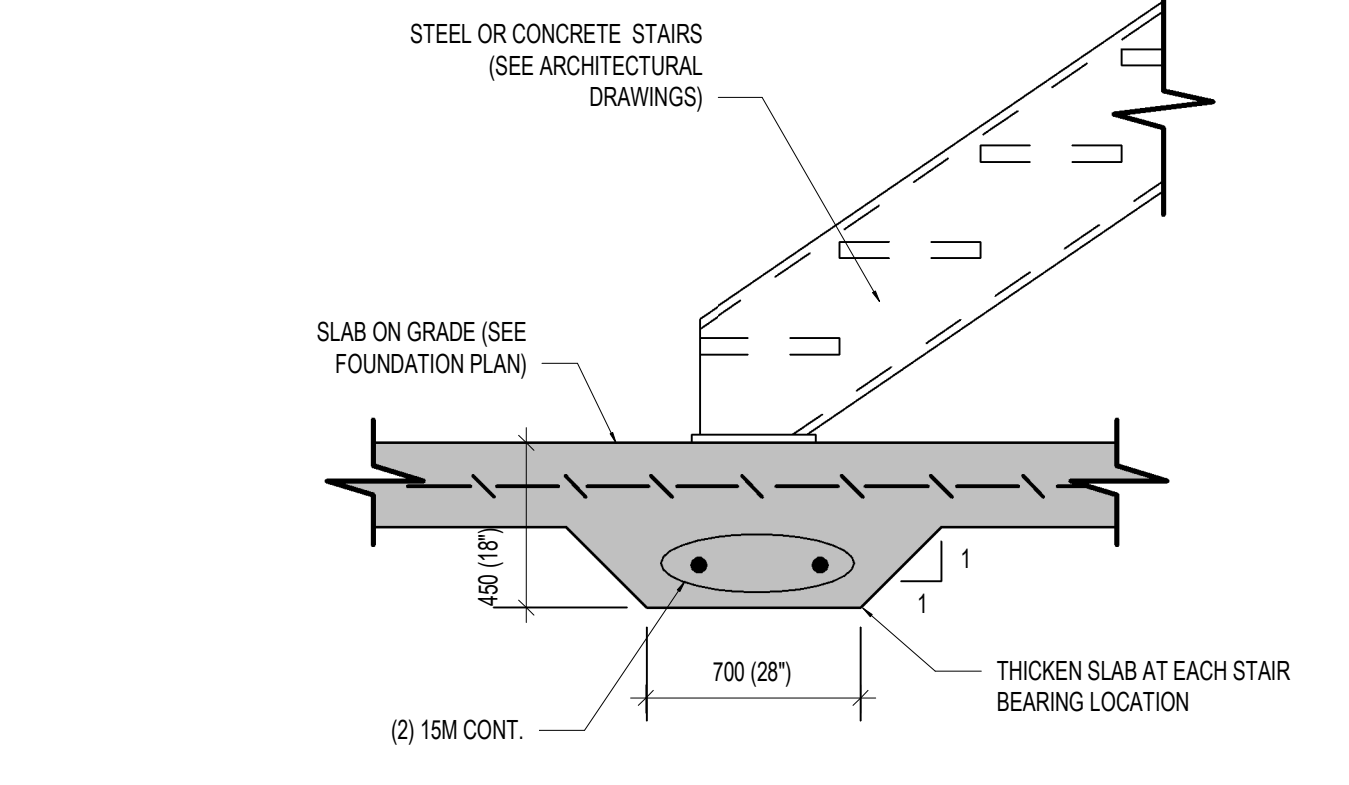
S.19 TYPICAL TENSION SPLICE FOR ANGLES



CONTROL JOINT NEAR TO OR ADJACENT WALL

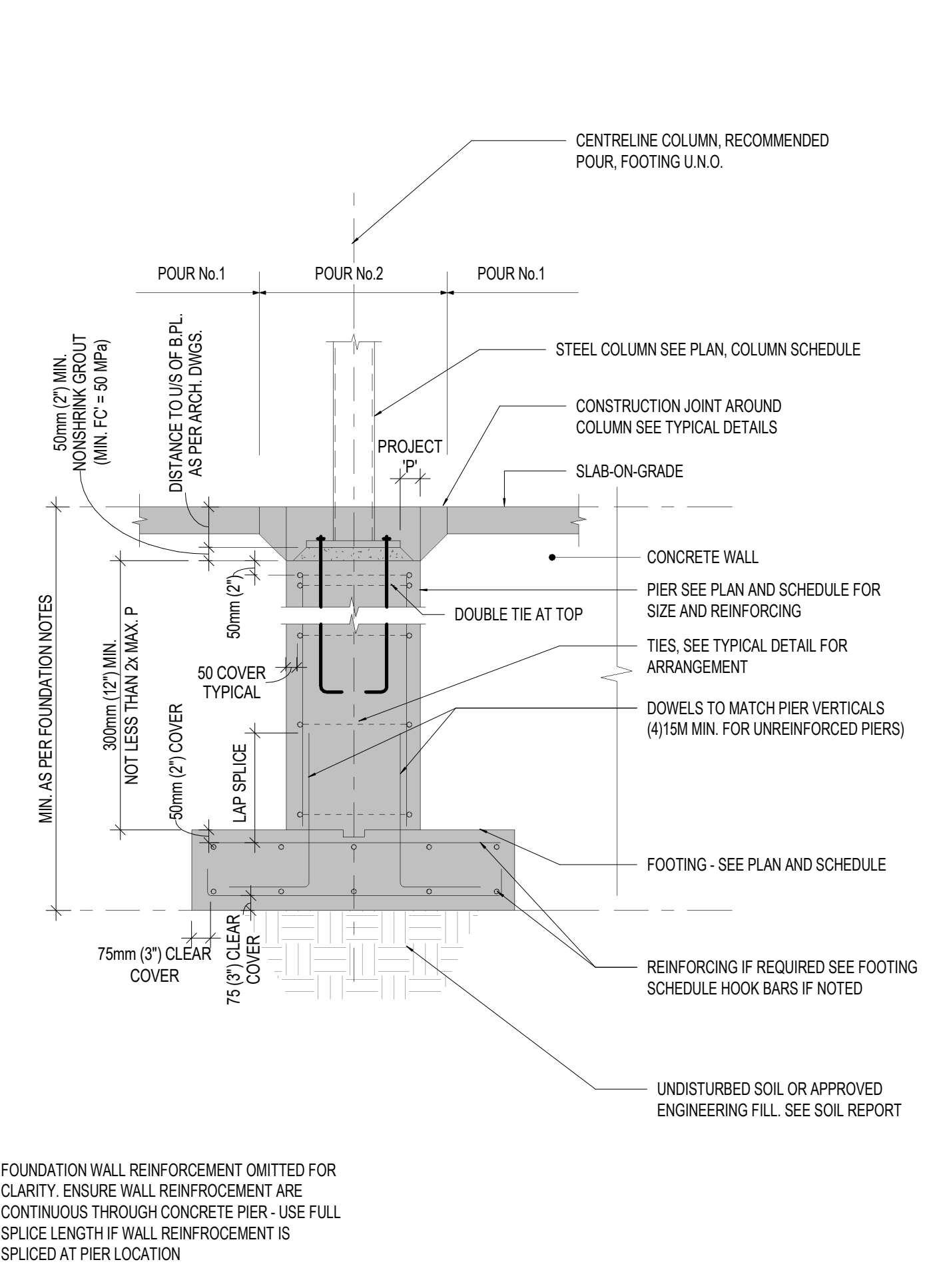


T.D.37 TYPICAL SLAB ON GRADE THICKENINGS

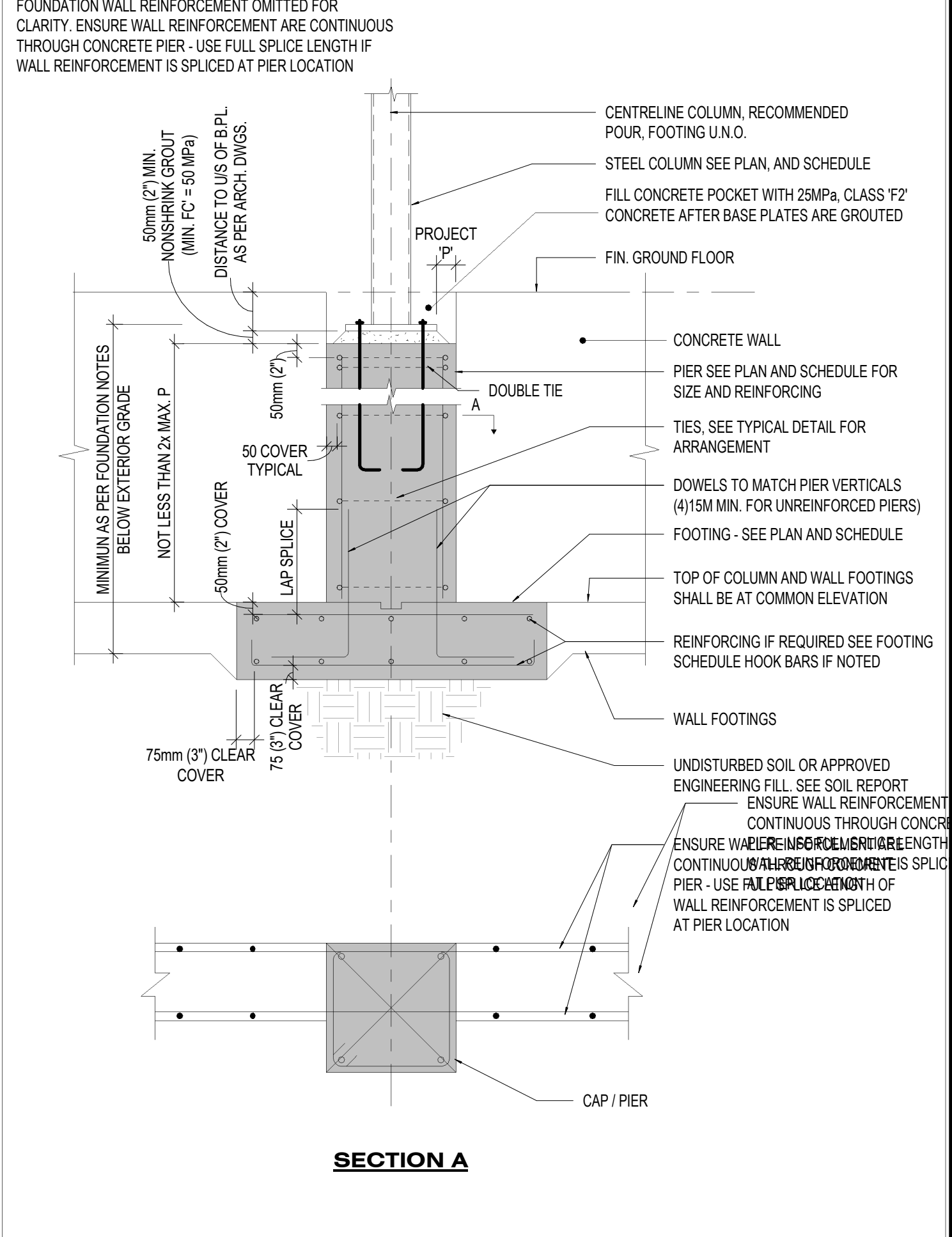


- NOTES:**
- COORDINATE LOCATION OF NON-LOAD BEARING BLOCK AND STAIR BEARING LOCATIONS WITH ARCHITECTURAL DRAWINGS.
 - EXTEND SLAB THICKENING 300mm (12") PAST EACH SIDE OF STAIR BEARING LOCATIONS AND DISCONTINUOUS ENDS OF BLOCK WALLS.
 - STEEL/CONCRETE STAIRS ANCHORS TO THICKENED SLAB BY OTHERS - PROVIDE STAMPED DRAWINGS PRIOR TO CONSTRUCTION.
 - REMOVE ALL ORGANIC AND DELETERIOUS MATERIAL OR AS SPECIFIED BY THE GEOTECHNICAL REPORT OR GEOTECHNICAL ENGINEER AND REPLACE WITH CONCRETE MIN. FC = 20 MPa.

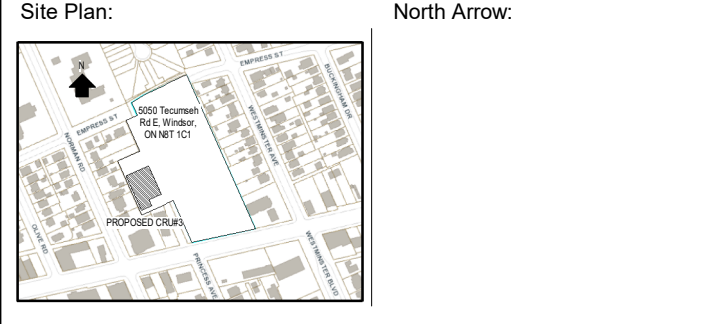
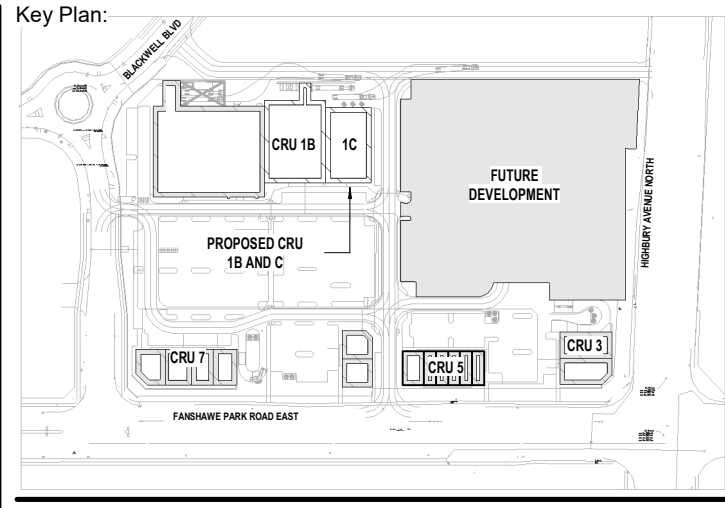
S.14 TYPICAL INTERIOR COLUMN DETAIL, STEEL COLUMN



S.14 TYPICAL INTERIOR COLUMN DETAIL, STEEL COLUMN



S.15 TYPICAL EXTERIOR COLUMN FOUNDATION DETAIL, STEEL COLUMN



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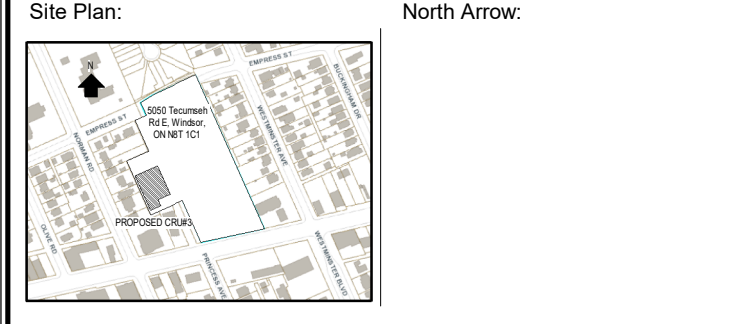
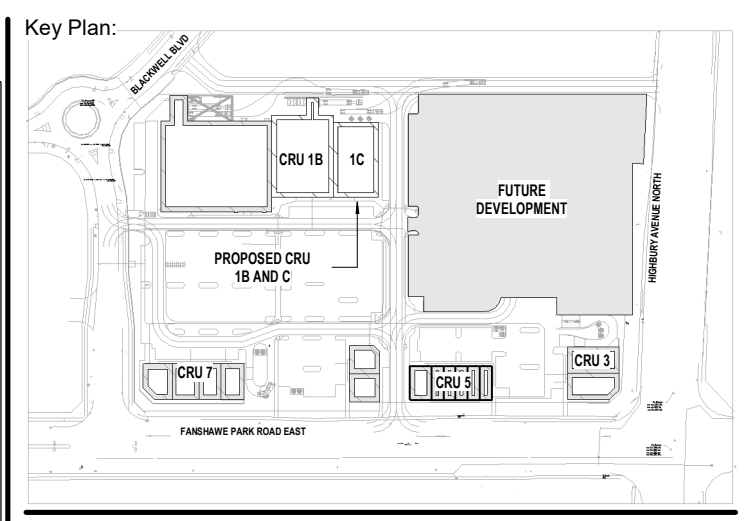
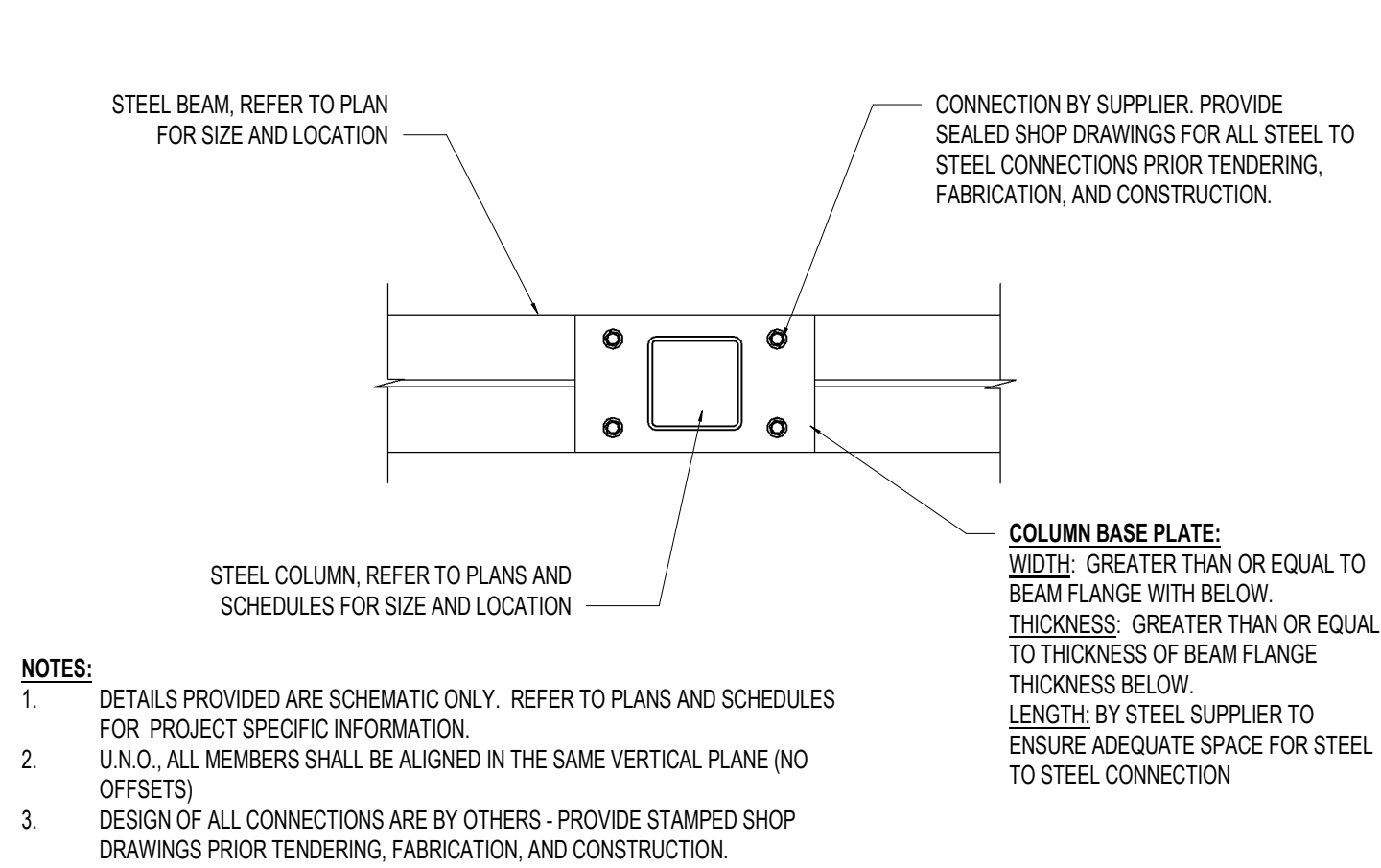
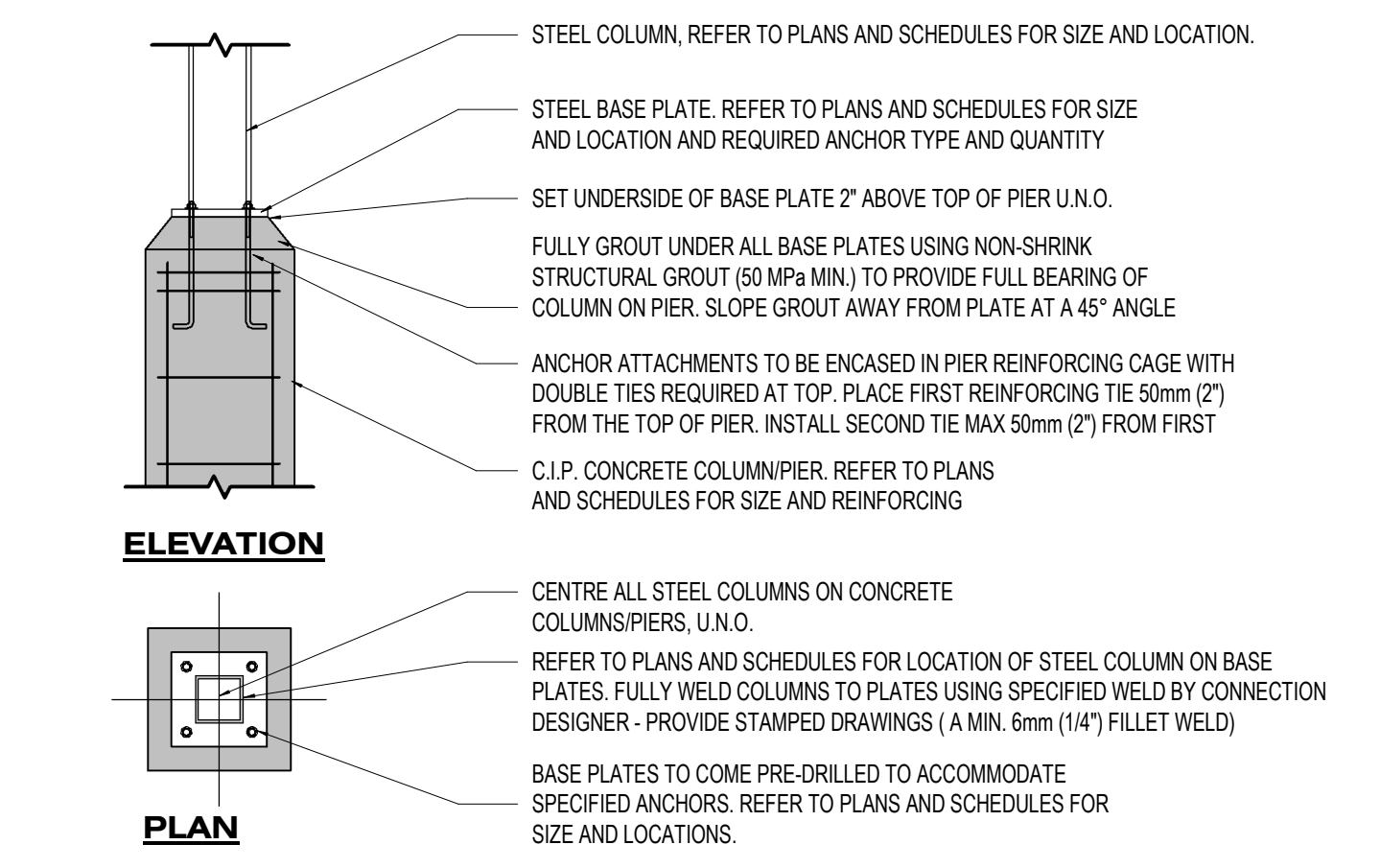
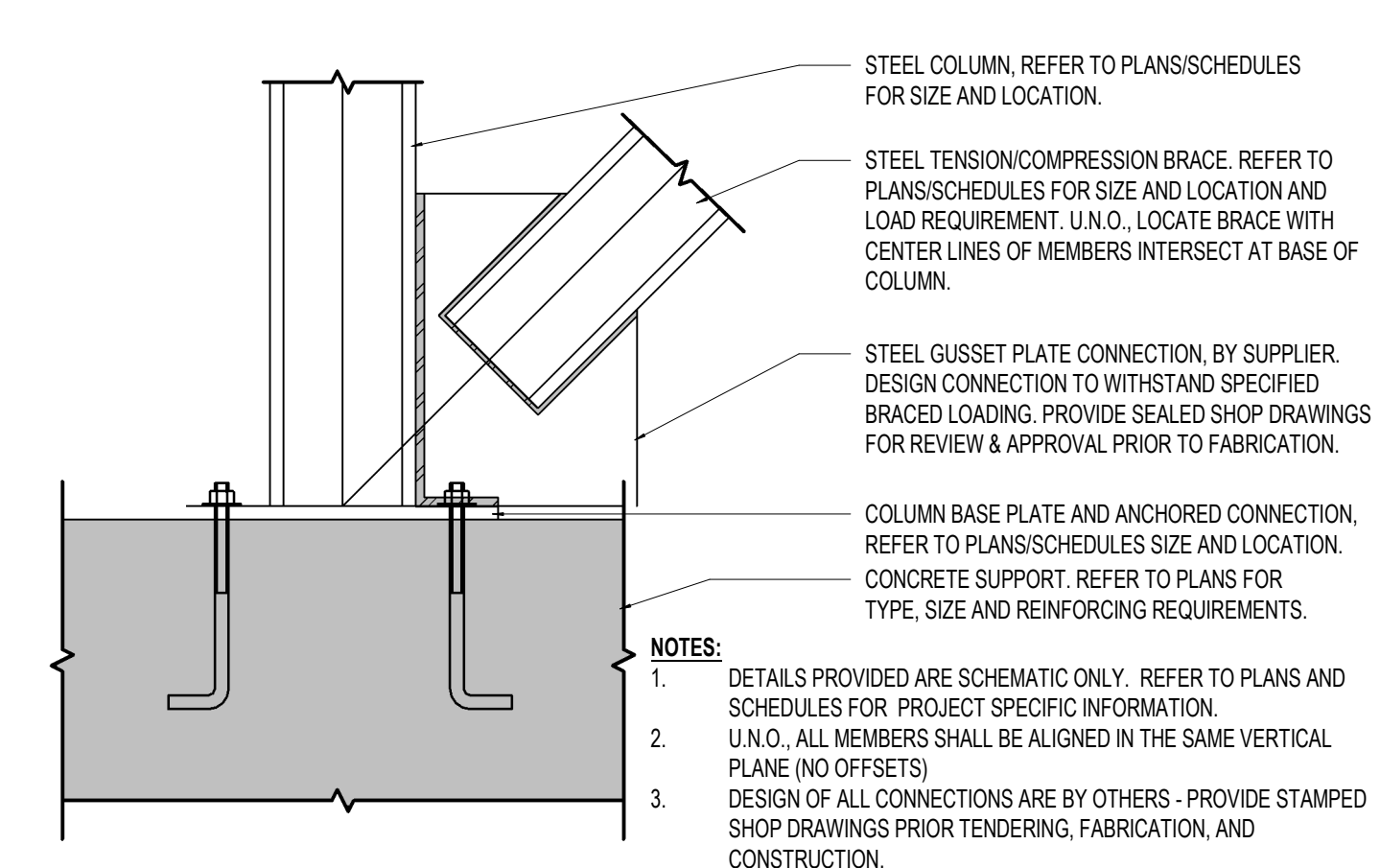
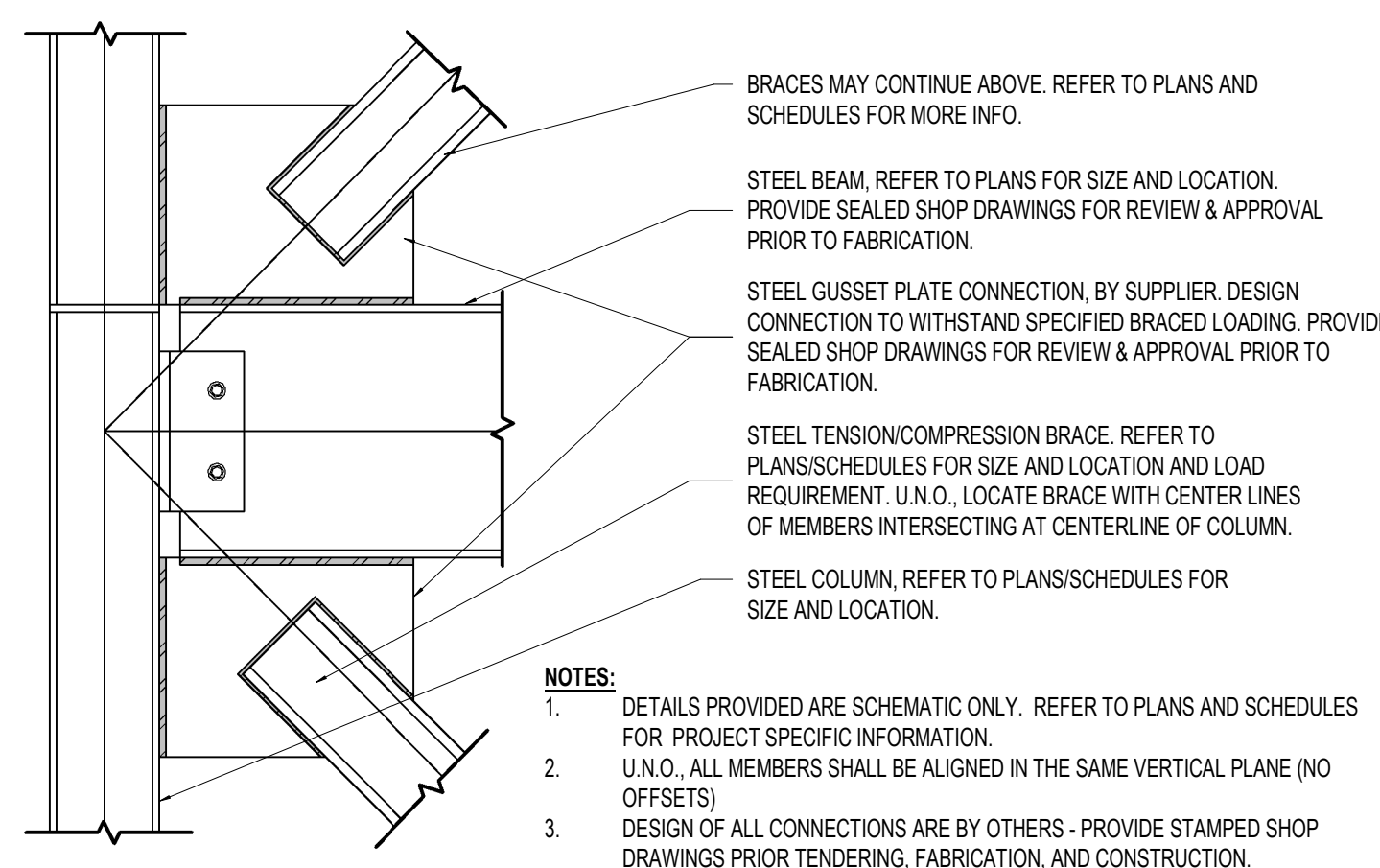
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Project: **WESTDELL** DEVELOPMENT CORP
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Drawing Title: **TYPICAL DETAILS III**

Drawn By: D.H./S.D./D.K.	Scale: AS INDICATED
Checked By: M.A.F./J.G.	Plot Date: APR. 03-2024
Project Date: AUG. 2023	
Project No: 2023-102	
Drawing No:	Revision



Consultant:

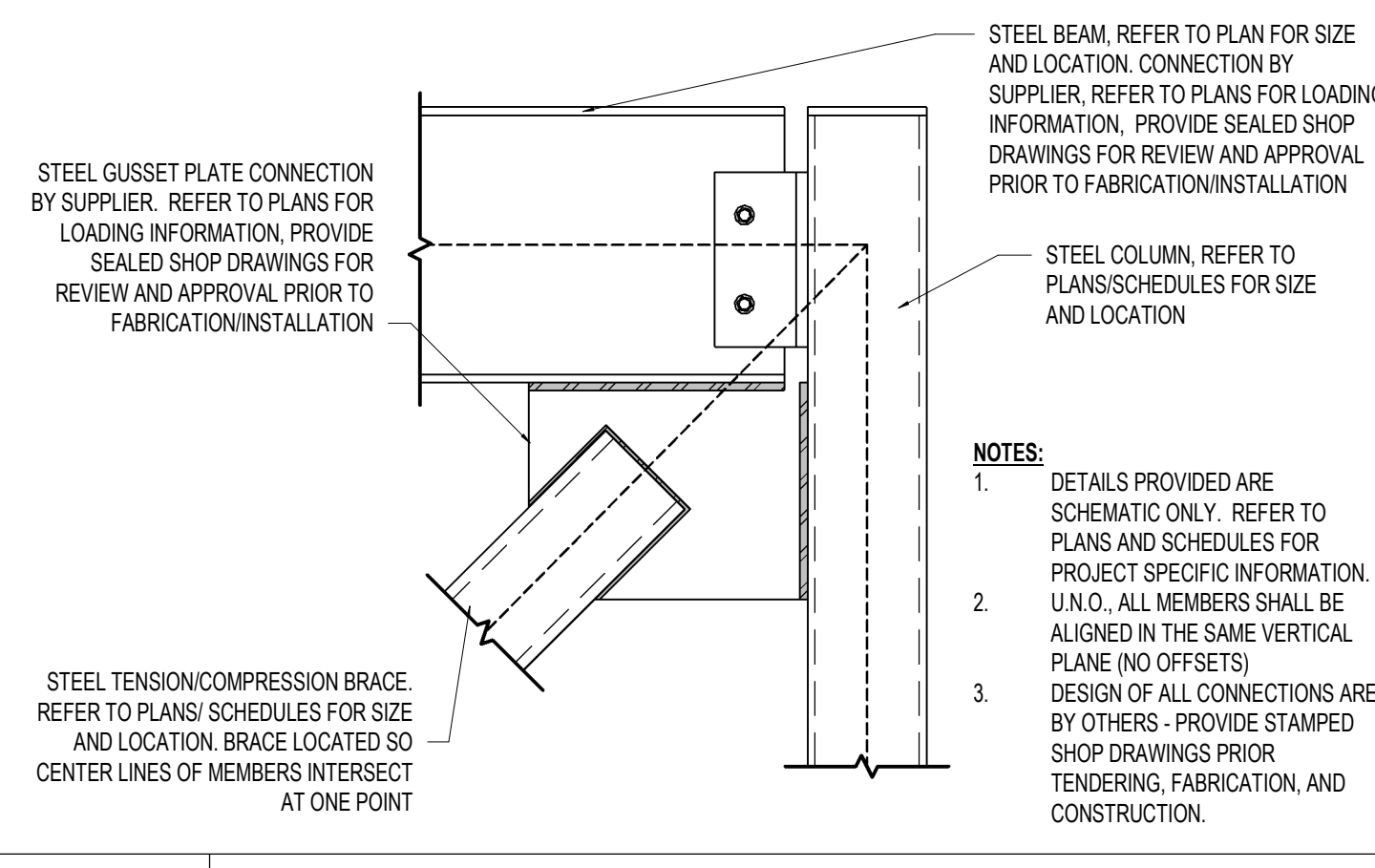
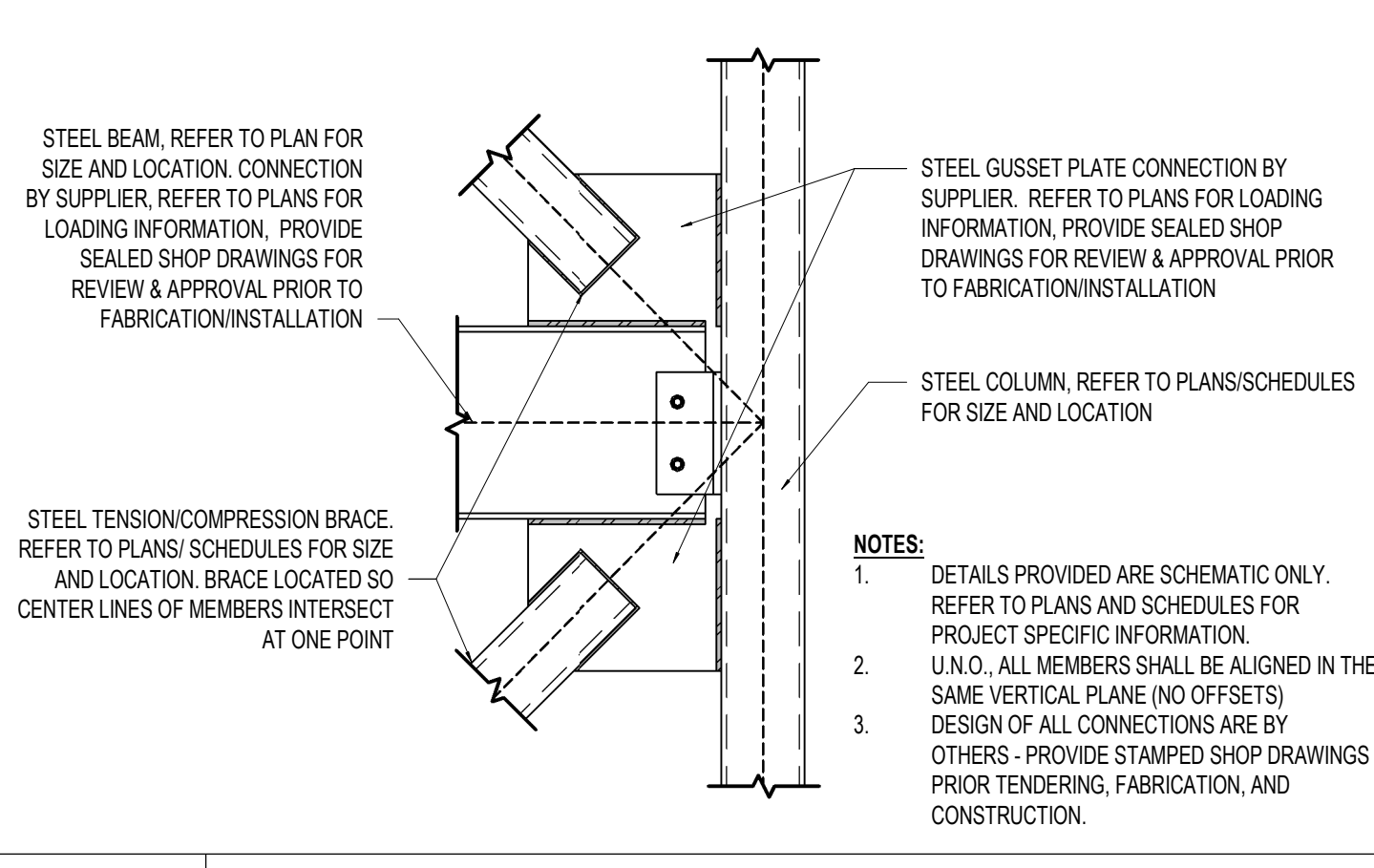
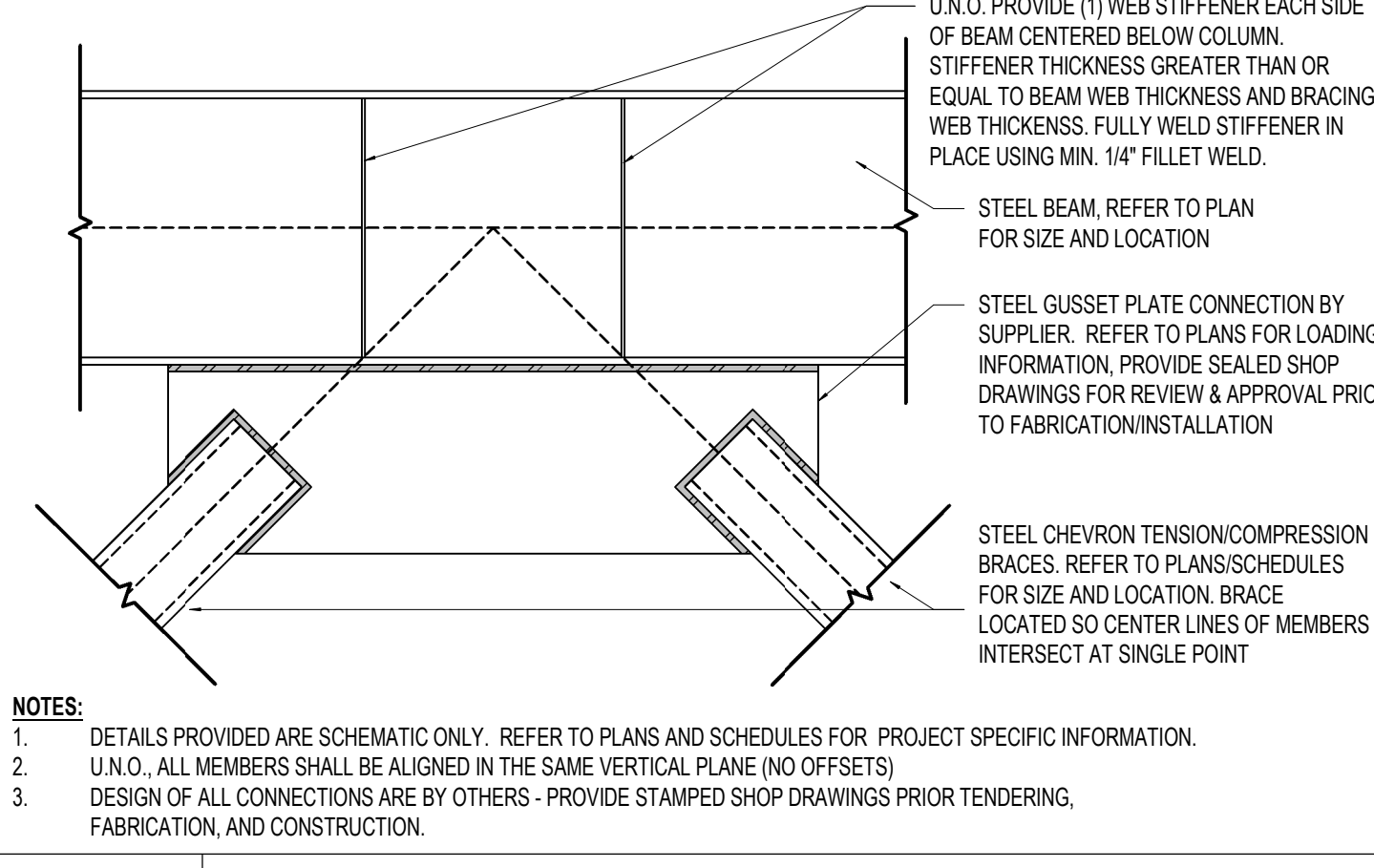
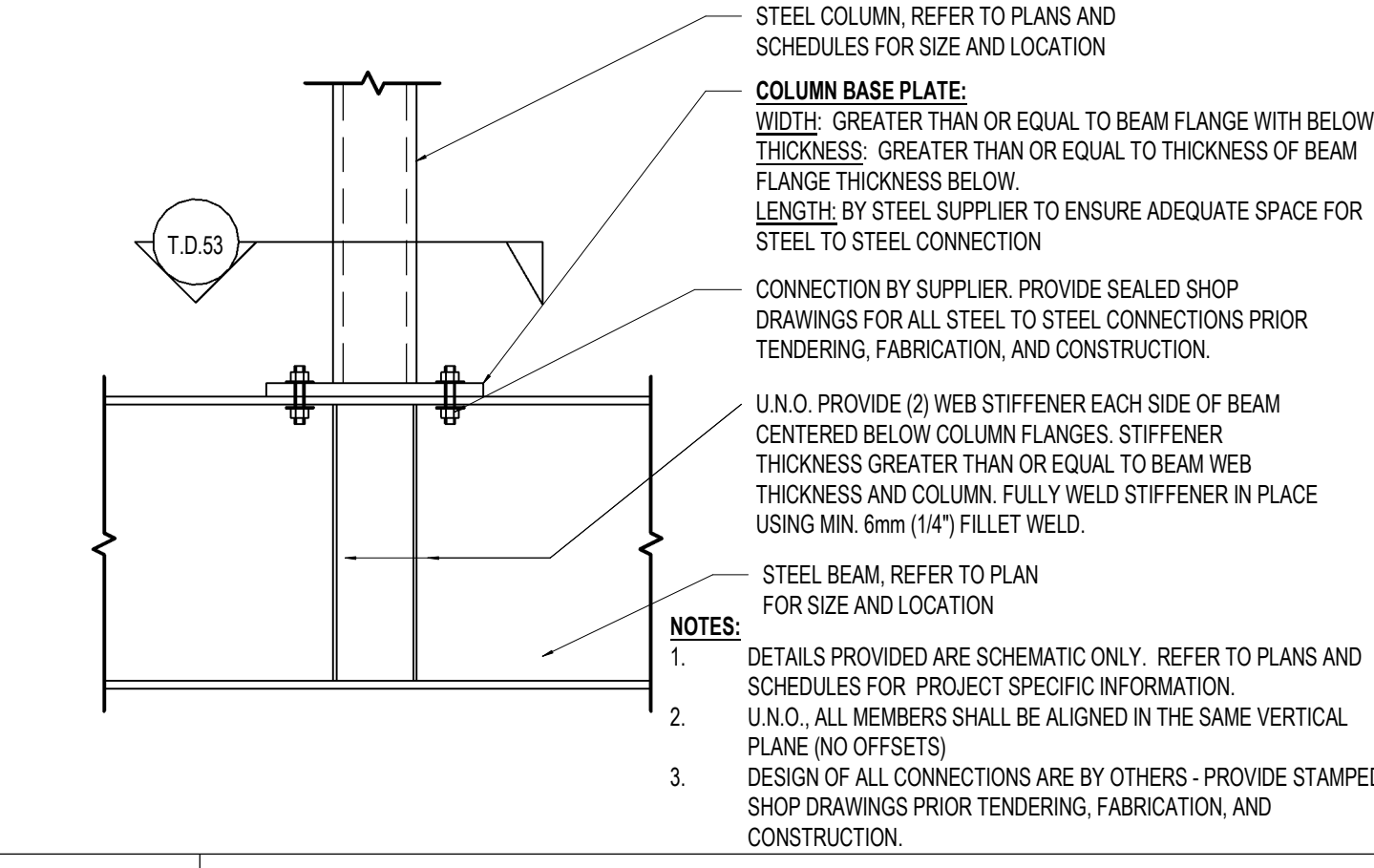
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Intelligent Engineering Design Ltd.
STRUCTURAL ENGINEERS
iedesign@ie-design.co.uk www.ie-design.co.uk

T.D. 50 **STEEL BRACE TERM. - STEEL COLUMN (FLOOR/ROOF)**

T.D. 51 **STEEL BRACE TERM. - STEEL COLUMN BASE**

T.D. 52 **STEEL COLUMN TO CONCRETE COLUMN/PIER CONNECTION**

T.D. 53 **STEEL COLUMN SUPPORTED ON STEEL BEAM - SECTION**

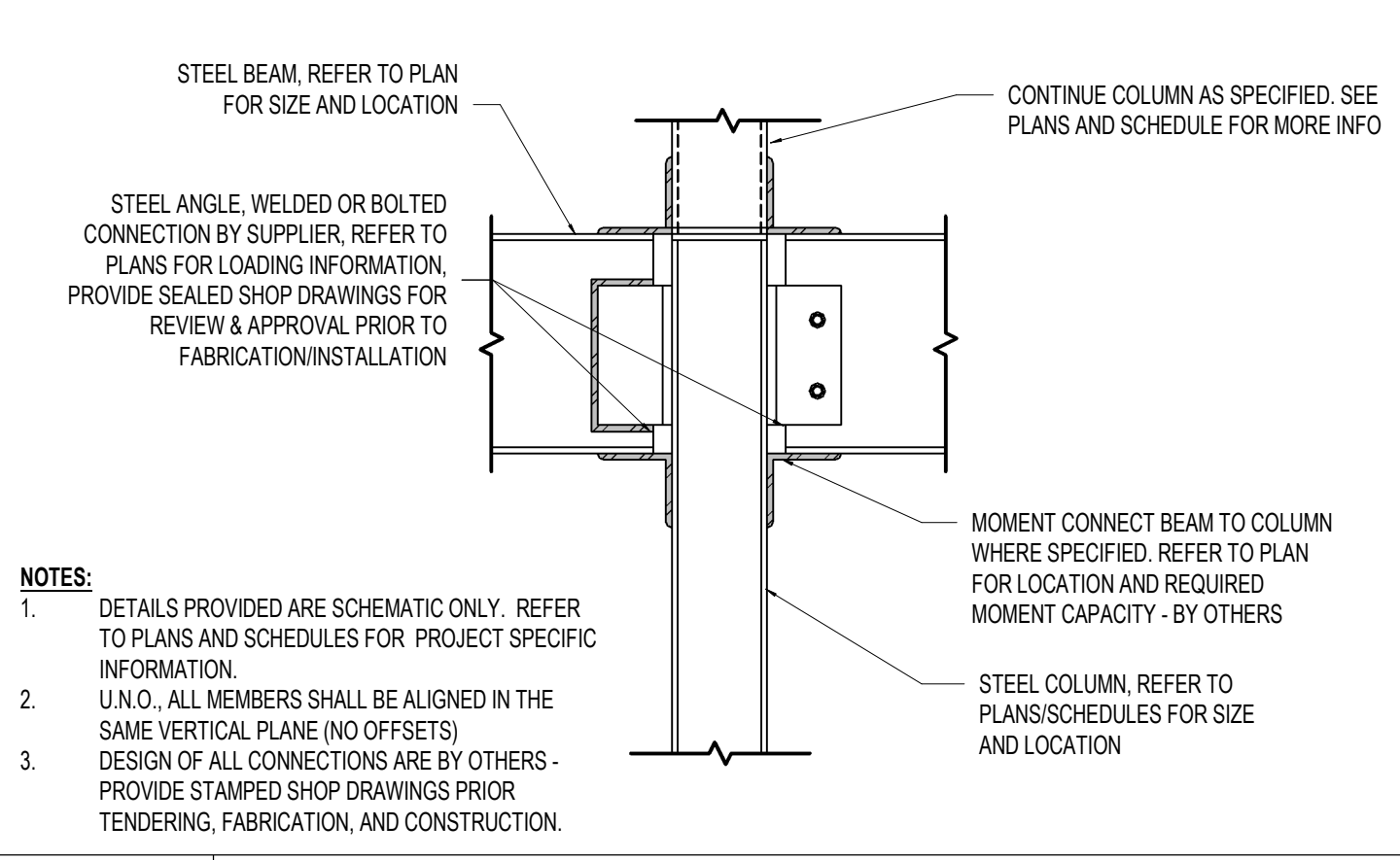
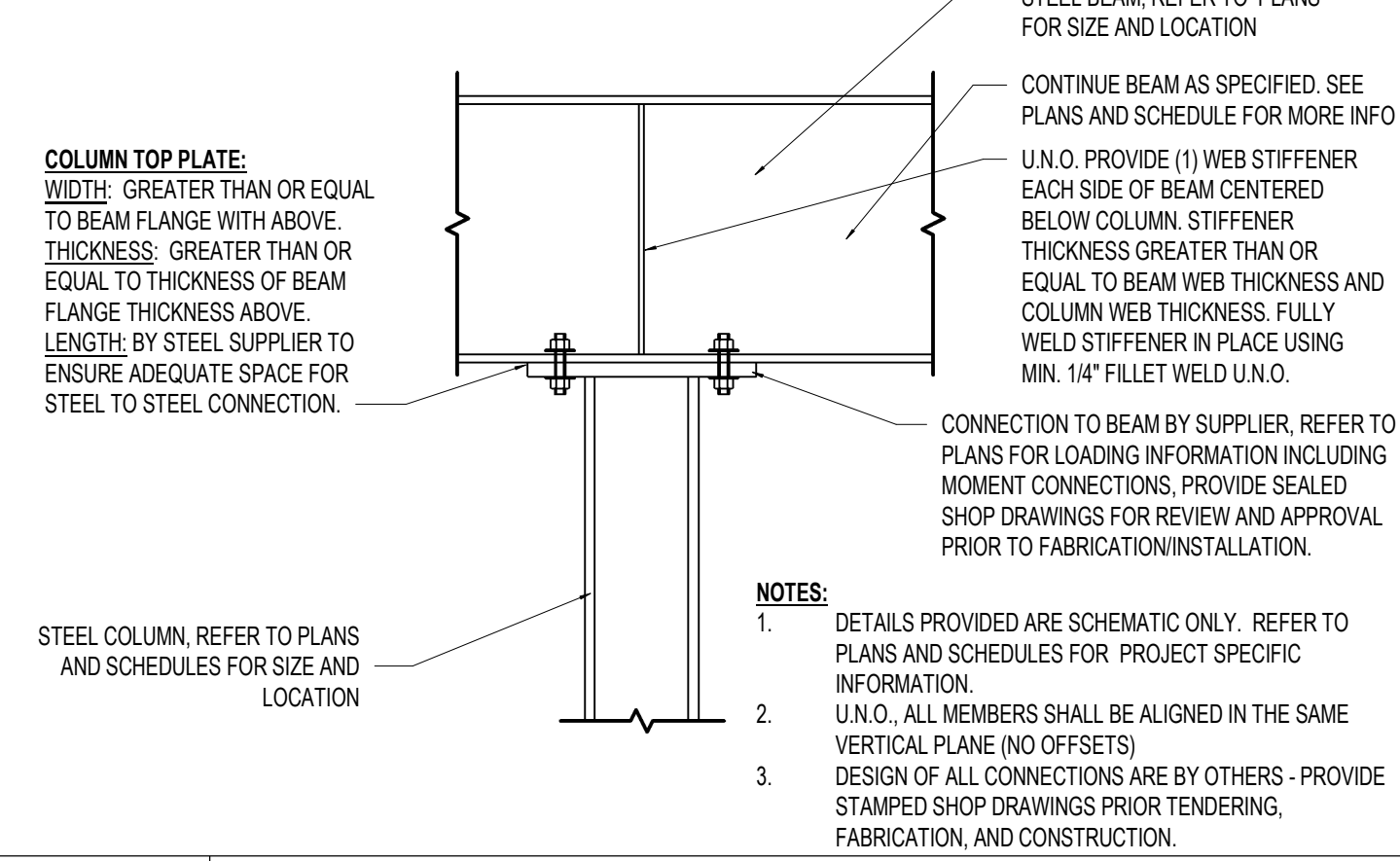
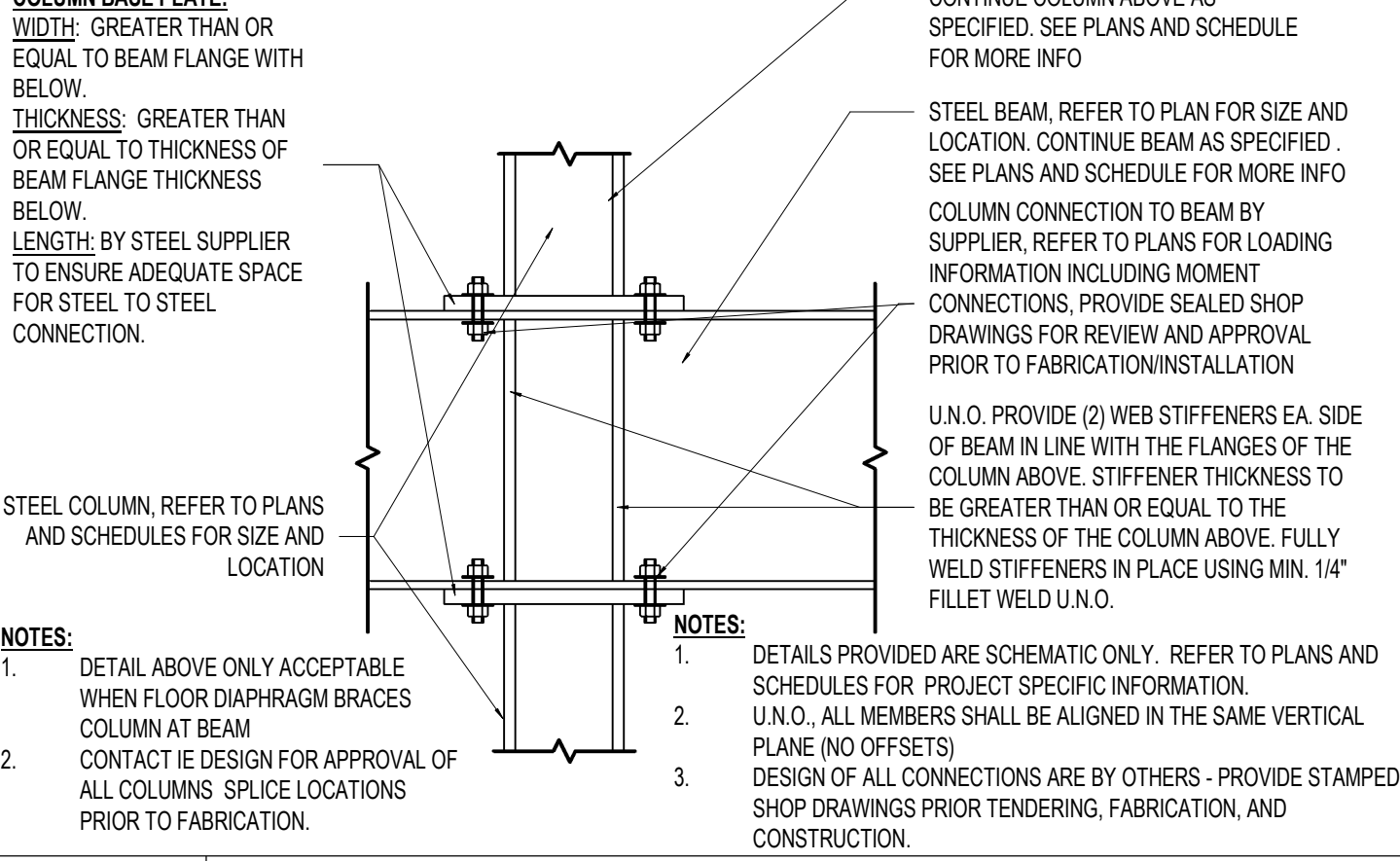
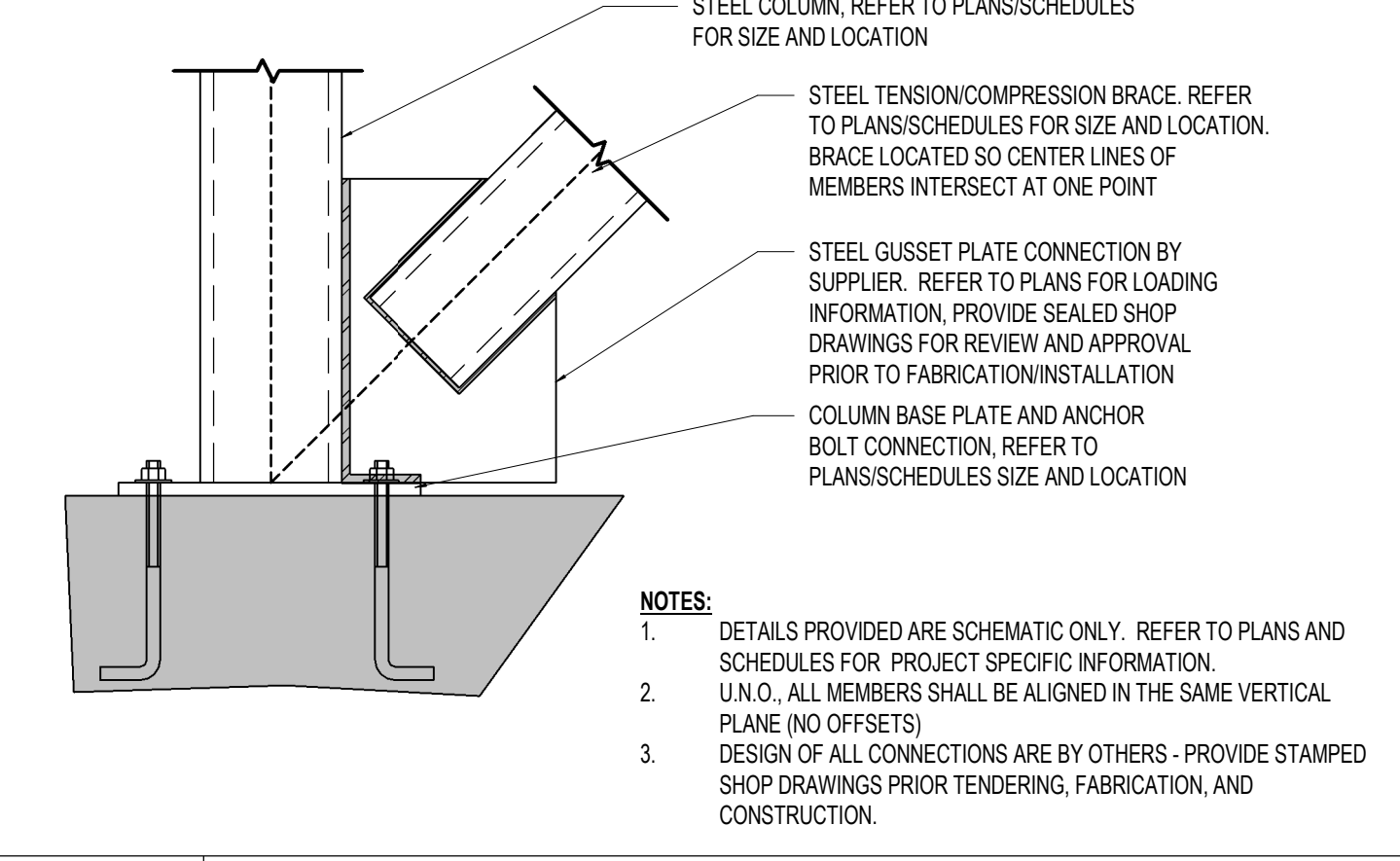


T.D. 54 **STEEL COLUMN SUPPORTED ON STEEL BEAM**

T.D. 55 **STEEL BRACED FRAME CONNECTION TOP CHEVRON**

T.D. 56 **STEEL BRACED FRAME CONNECTION MID LEVEL**

T.D. 57 **STEEL BRACED FRAME CONNECTION TOP X-BRACE**

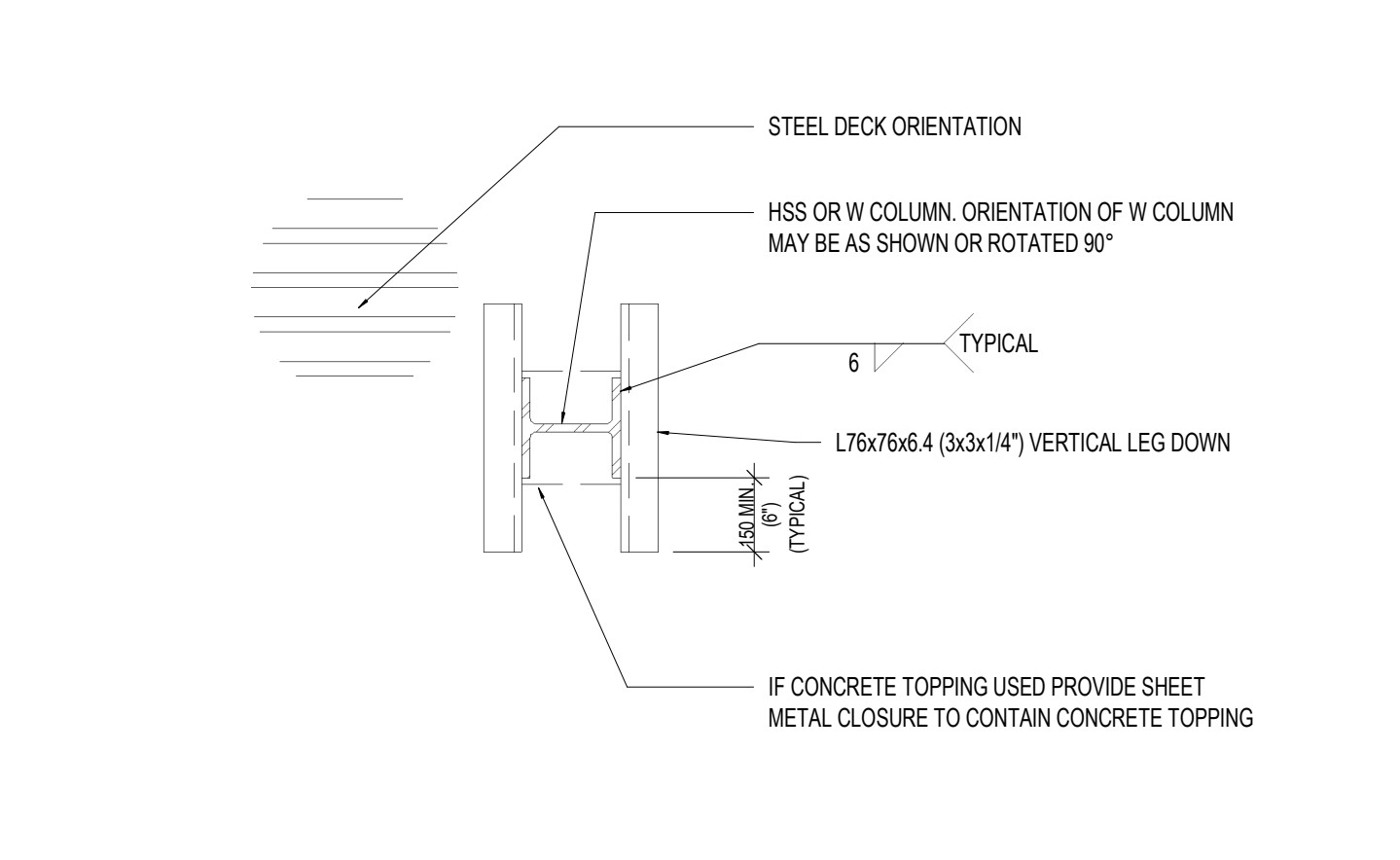
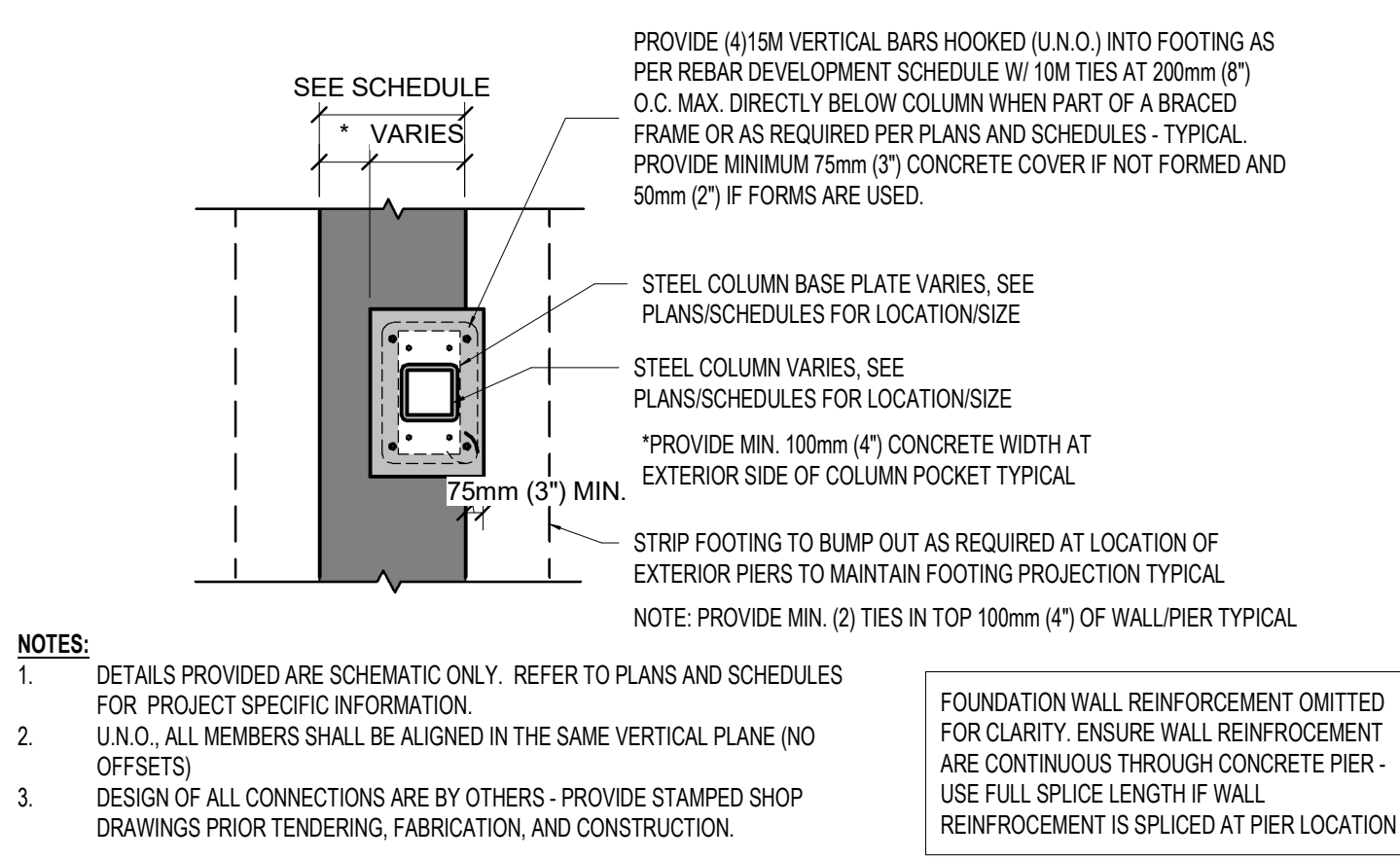
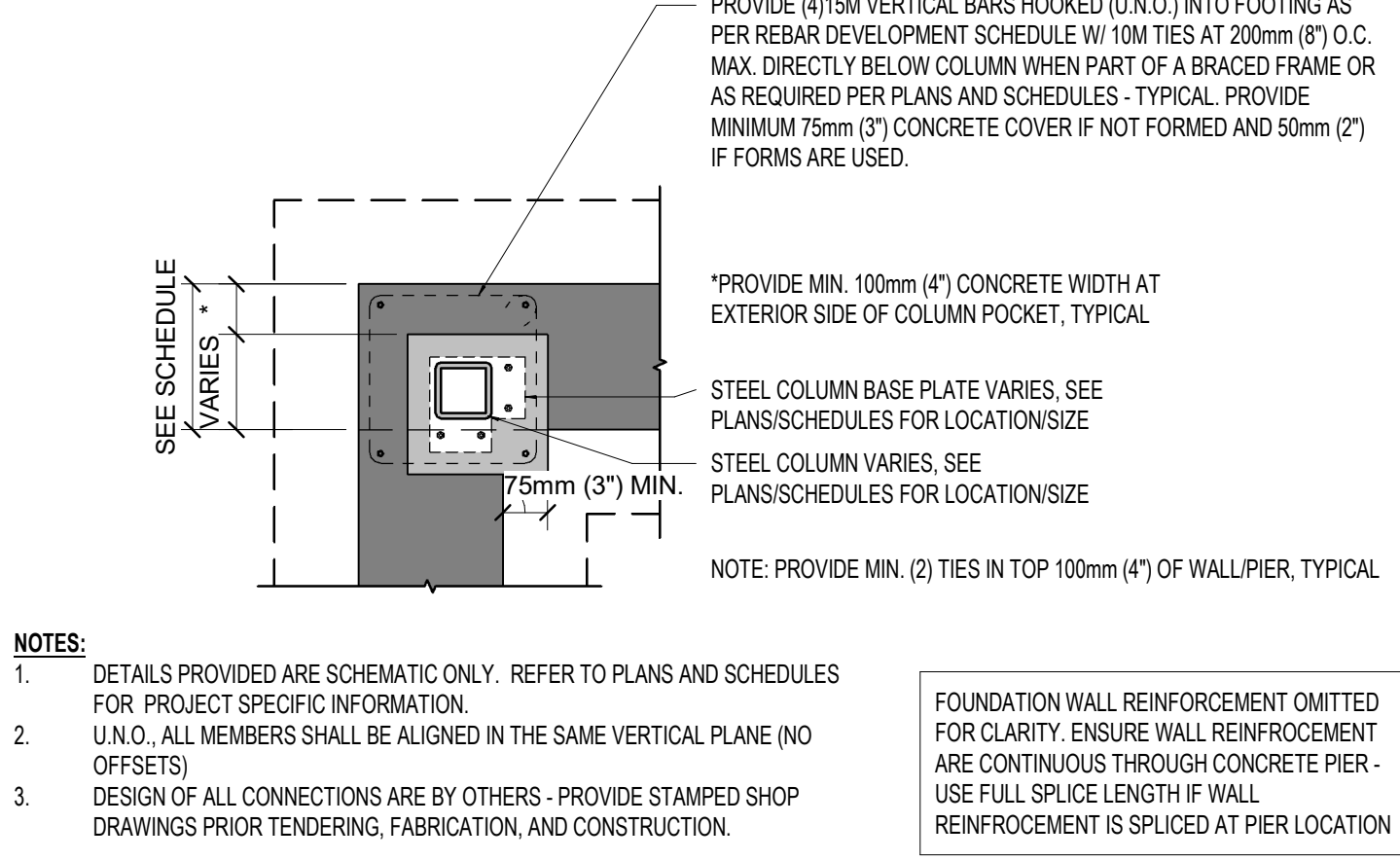


T.D. 58 **STEEL BRACED FRAME CONNECTION COLUMN BASE**

T.D. 59 **STEEL BEAM TO COLUMN CONNECTION - COLUMN SPLICE**

T.D. 60 **STEEL BEAM TO COLUMN CONNECTION - TOP BEARING**

T.D. 61 **STEEL BEAM TO COLUMN CONNECTION - SIDE CONNECTION**



T.D. 63 **FOUNDATION COLUMN POCKET DETAIL - CORNER AT BRACED BAYS**

T.D. 64 **FOUNDATION COLUMN POCKET DETAIL - AT BRACED BAYS**

S.17 **TYPICAL DECK SUPPORT AT COLUMNS**

Seal: Seal:

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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 23 2024	ISSUED FOR COORDINATION
1		

ISSUES/REVISION TABLE

No.	Date	Revision

Project:

WESTDELL
DEVELOPMENT CORP
1300 FANSHAW PARK RD.
EAST. - CRU #1B AND C
1300 FANSHAW PARK RD. EAST, LONDON, ON

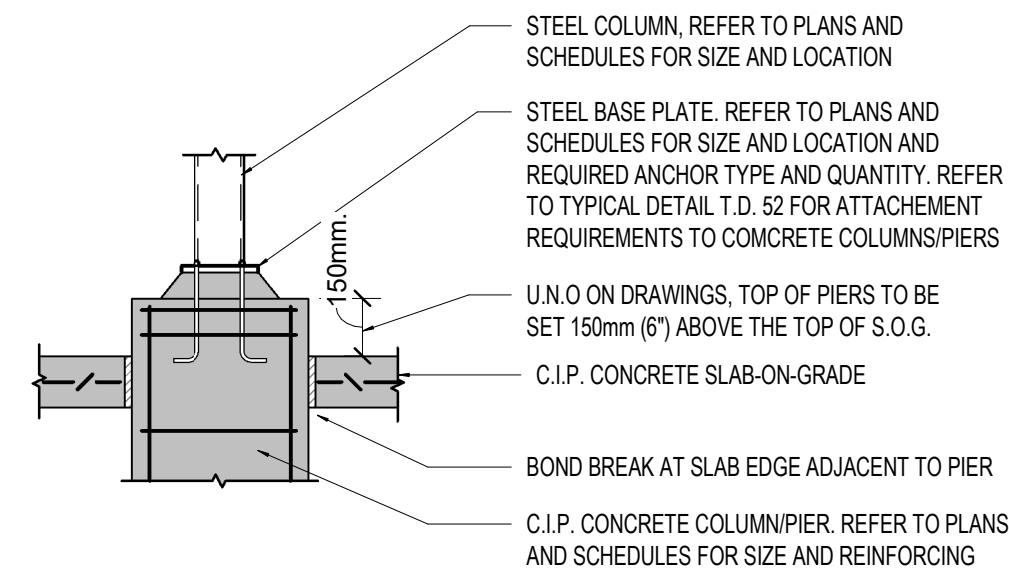
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TYPICAL DETAILS IV

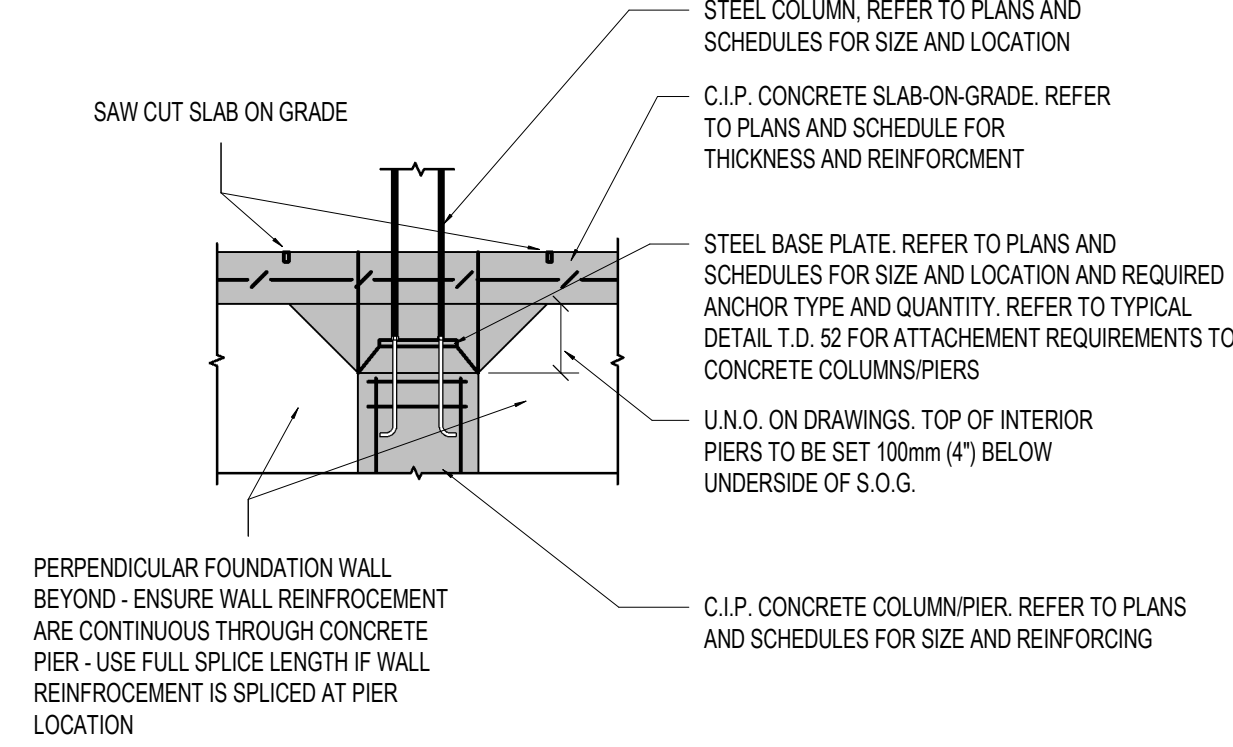
Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102

Drawing No: Revision

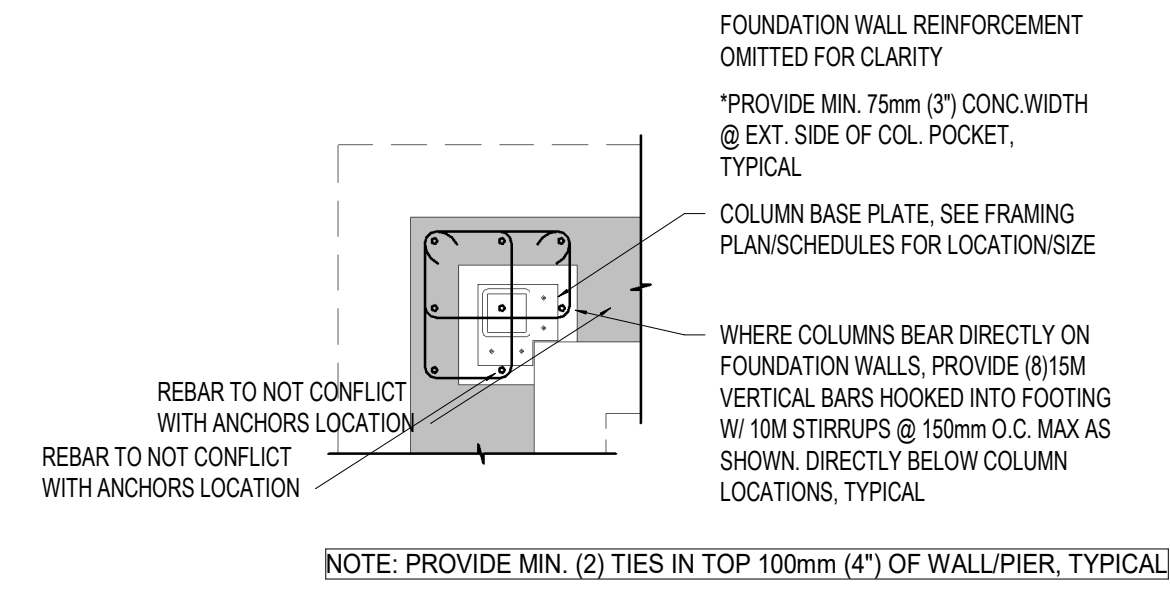
S-6.3 **2**



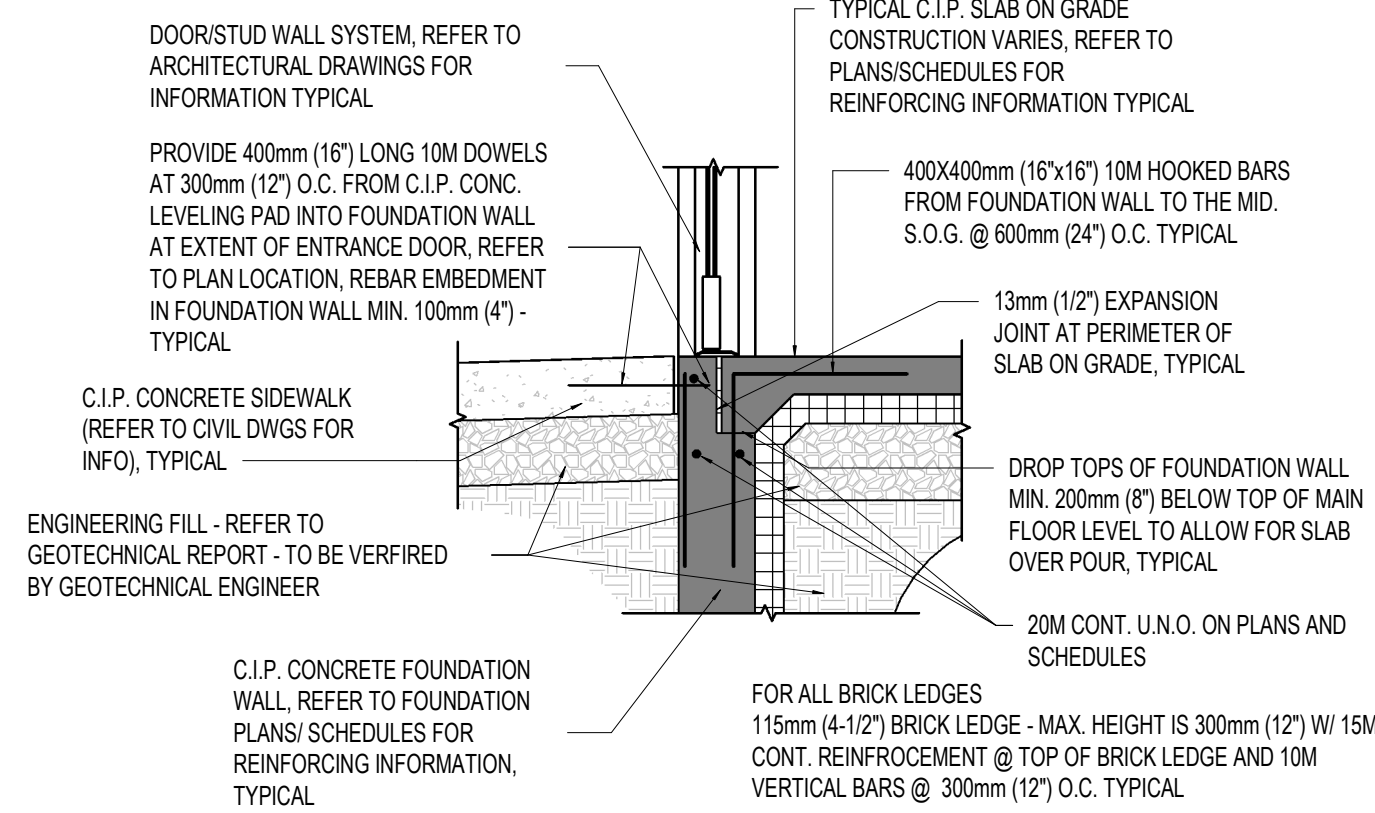
T.D. 66 STEEL COLUMN TO FOUNDATION PIER - EXTERIOR



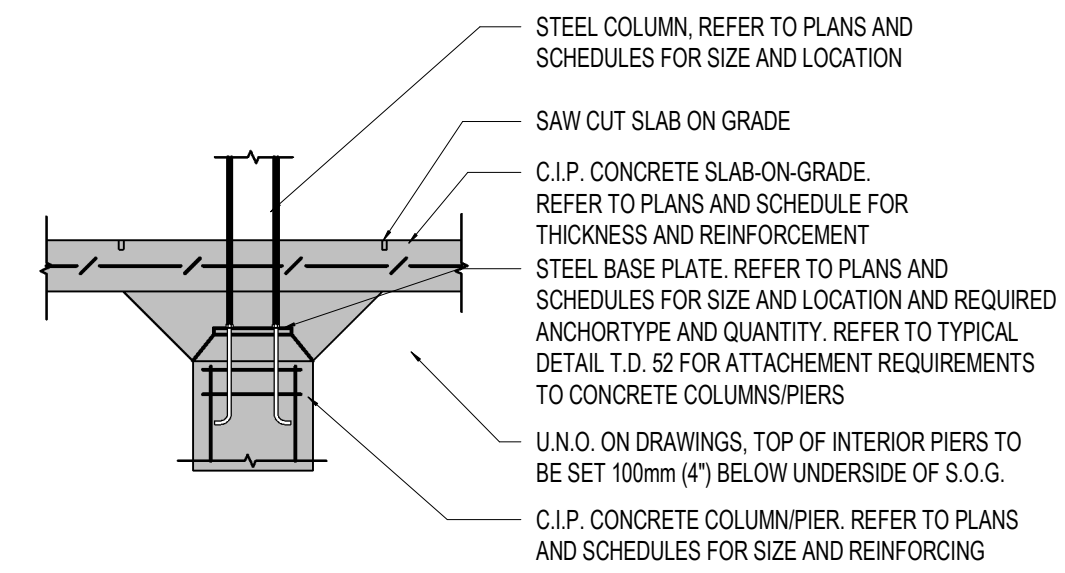
T.D. 68 STEEL COLUMN TO FOUNDATION PIER W/ FOUNDATION WALL - EXTERIOR



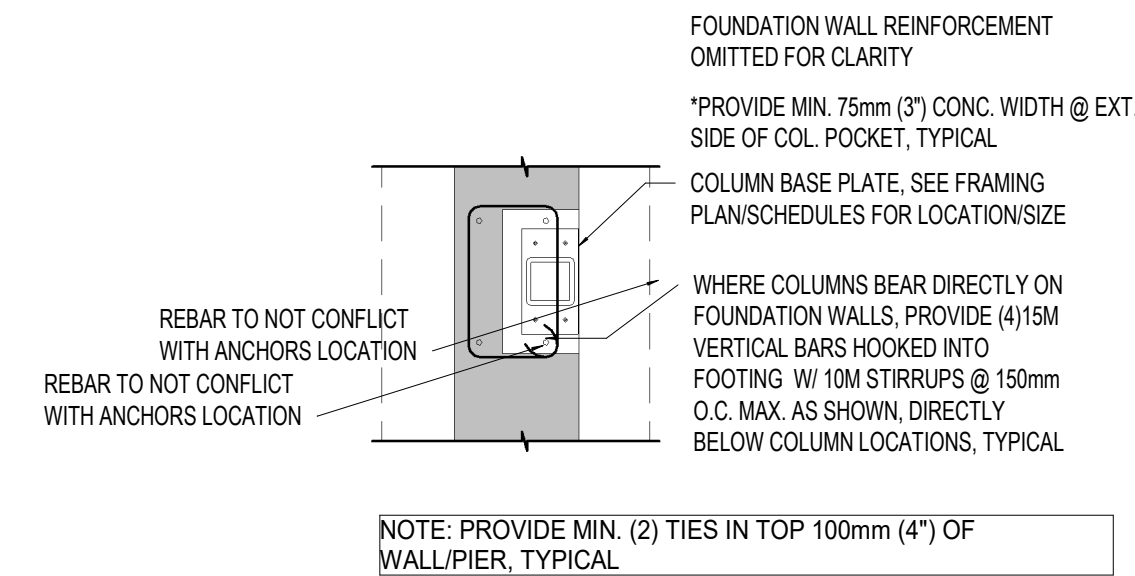
S.40 CORNER COLUMN POCKET DETAIL



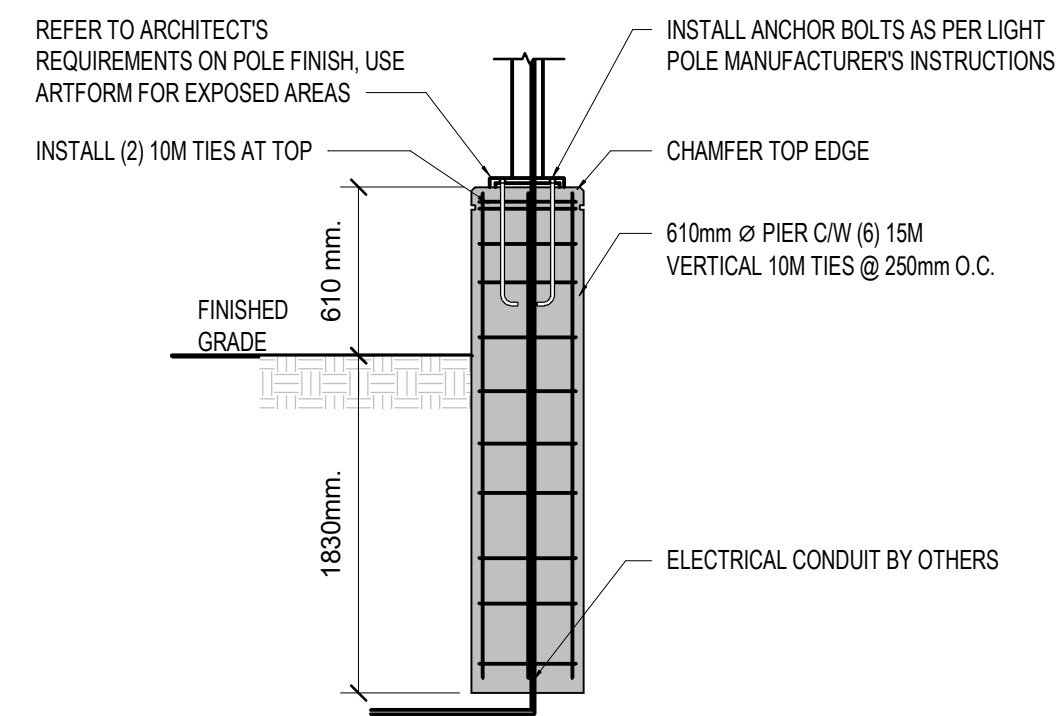
T.D. 69 TYPICAL FROST SLAB AT ENTRY DOOR DETAIL



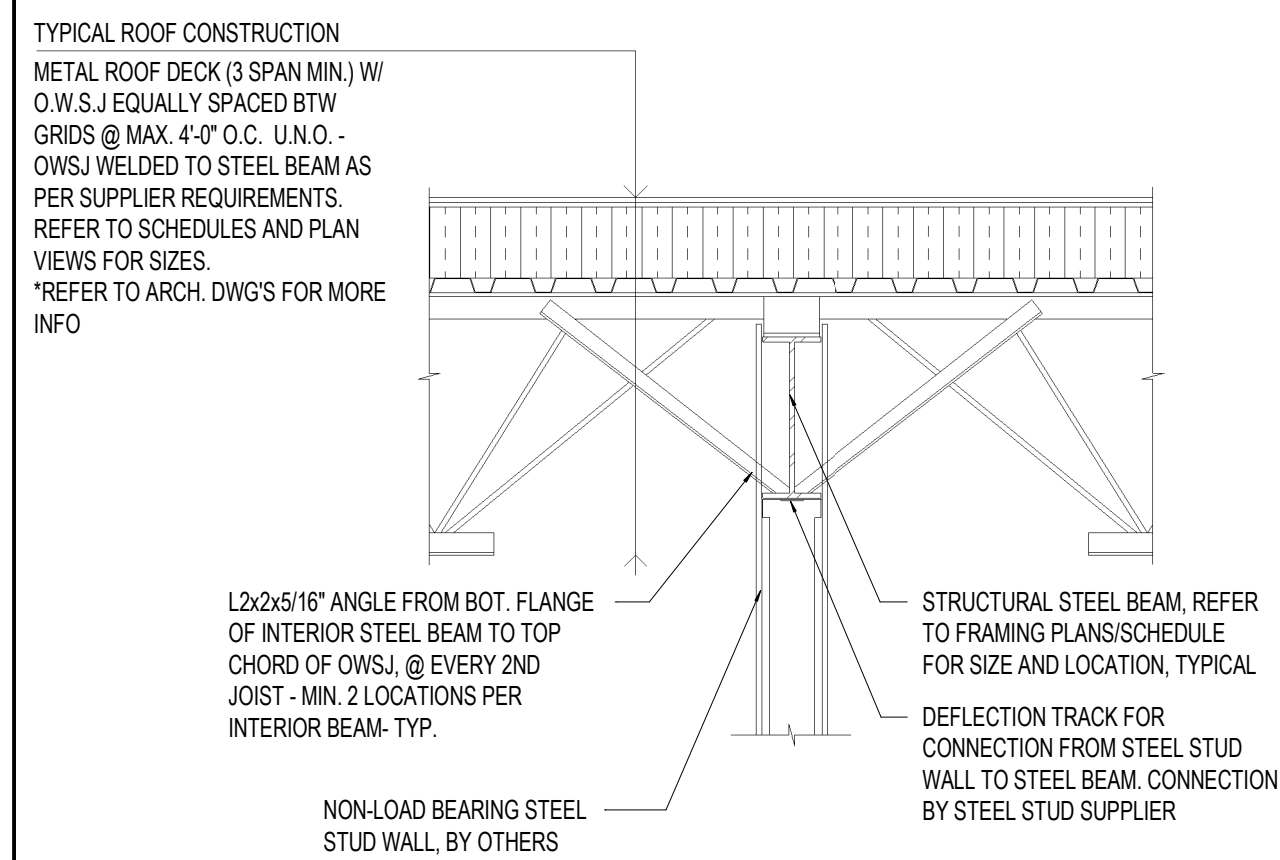
T.D. 70 STEEL COLUMN TO FOUNDATION PIER - INTERIOR



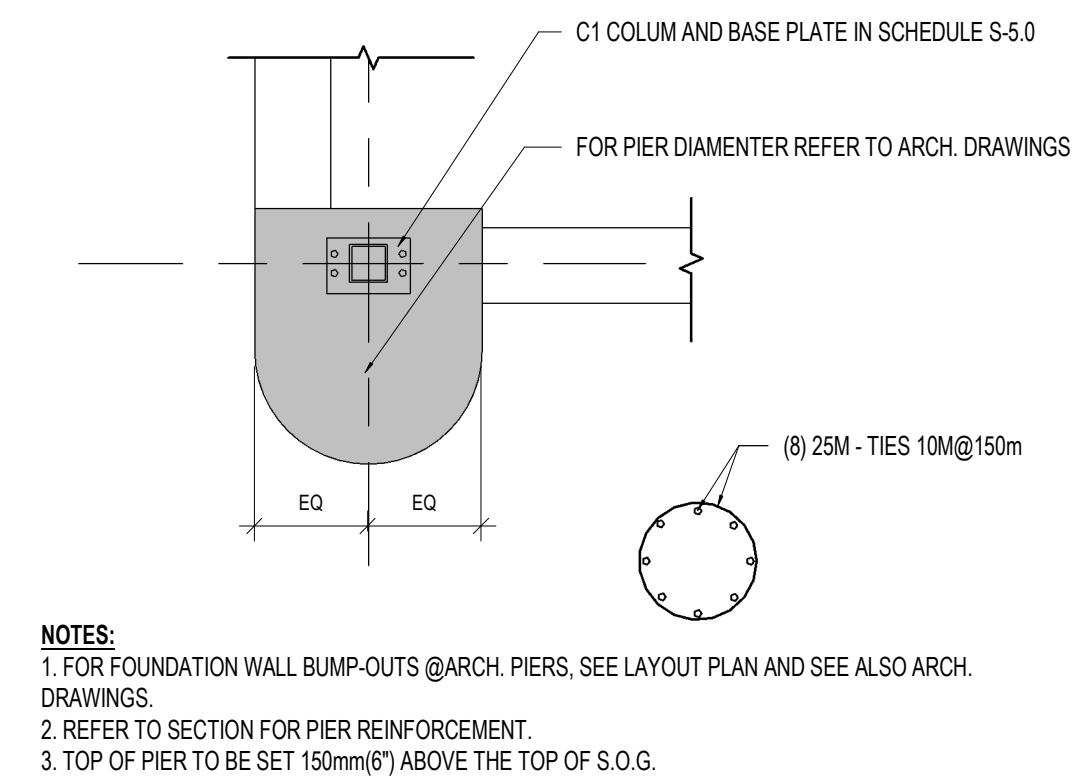
S.70 EDGE COLUMN POCKET DETAIL



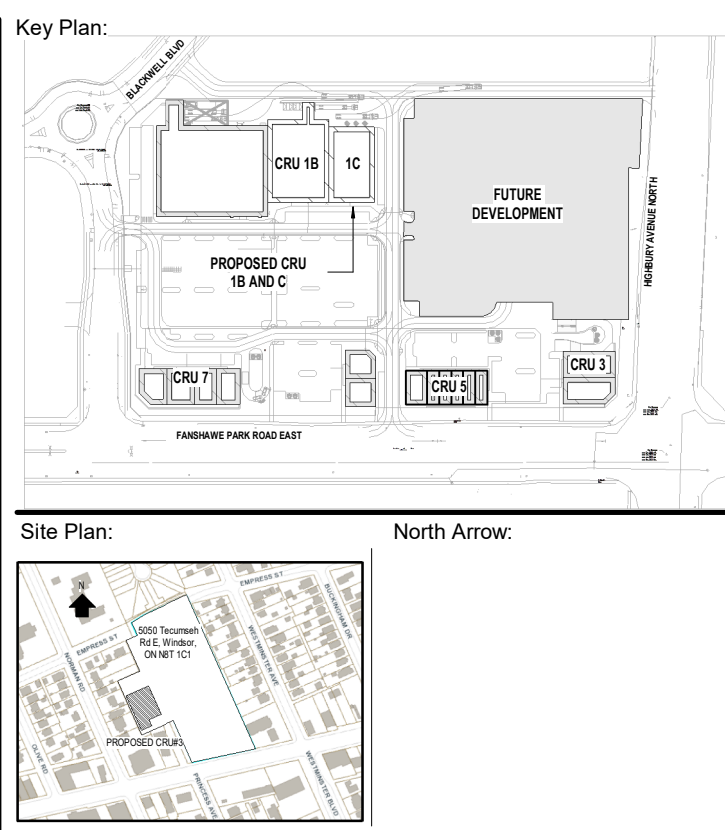
T.D.71 LIGHT POLE FOUNDATION DETAIL



S.20 TYPICAL INTERIOR BEAM REINF.



S.50 STRUCTURAL STEEL COLUMN SUPPORT PIER DETAIL



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No.	Date	Revision
2	Apr 03 2024	ISSUED FOR TENDER
1	Feb 23 2024	ISSUED FOR COORDINATION

ISSUES/REVISION TABLE

Project: **WESTDELL DEVELOPMENT CORP**

1300 FANSHAW PARK RD. EAST. - CRU #1B AND C

1300 FANSHAW PARK RD. EAST. LONDON, ON

Drawing Title: **TYPICAL DETAILS V**

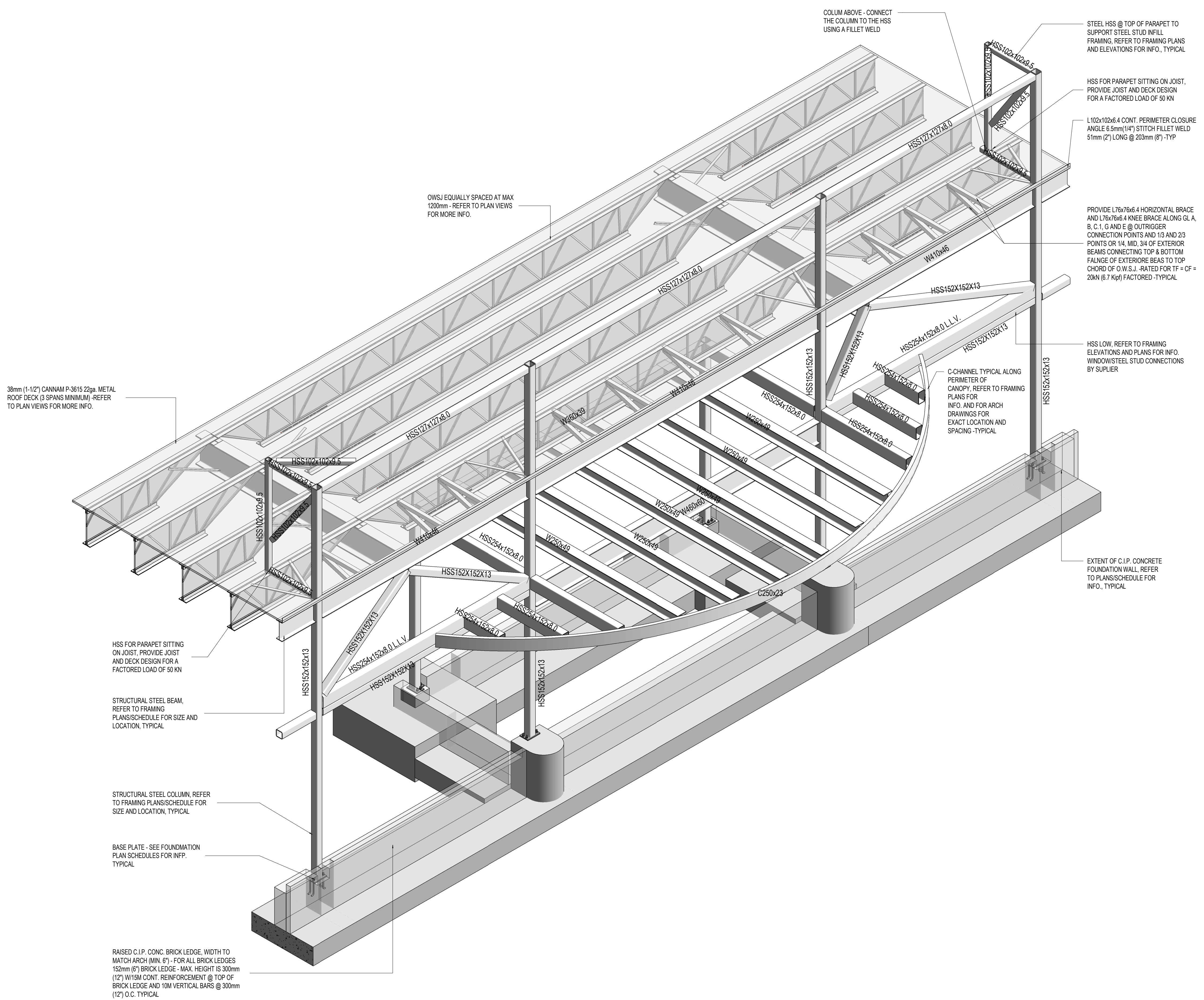
Drawn By: **D.H./S.D./D.K.** Scale: **AS INDICATED**

Checked By: **M.A.F./J.G.** Plot Date: **APR. 03-2024**

Project Date: **AUG. 2023**

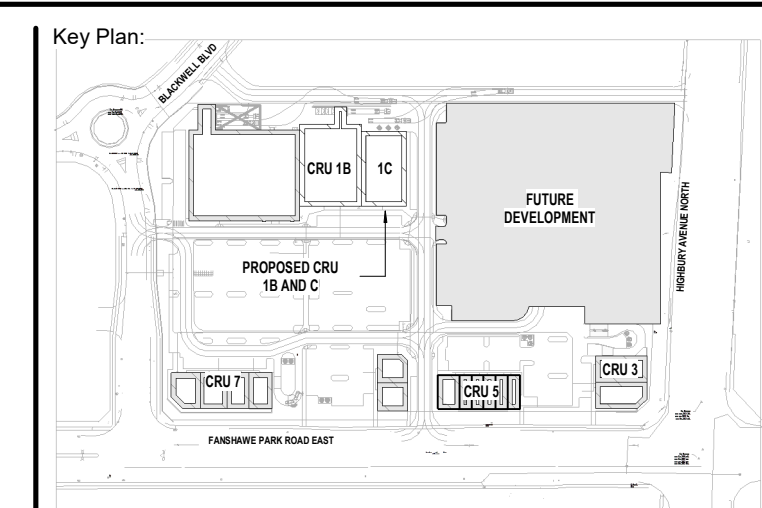
Project No: **2023-102**

Drawing No: _____ Revision _____



SCHMATIC II - 1B (NOT FOR CONSTRUCTION)
SCALE: NTS

- NOTES:**
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 2. U.N.O., ALL MEMBERS SHALL BE ALIGNED IN THE SAME VERTICAL PLANE (NO OFFSETS).
 3. ALL STEEL STUD ELEMENTS (INCLUDING, BUT NOT LIMITED) WALLS, PARAPETS, CANOPIES, EXTERIORS, AND OTHERS ARE DESIGNED BY OTHERS - PROVIDE STAMPED SHOP DRAWINGS PRIOR TENDERING, FABRICATION, AND CONSTRUCTION.
 4. ALL WOOD ELEMENTS (INCLUDING, BUT NOT LIMITED) WALLS, PARAPETS, CANOPIES, EXTERIORS, AND OTHERS ARE DESIGNED BY OTHERS - PROVIDE STAMPED SHOP DRAWINGS PRIOR TENDERING, FABRICATION, AND CONSTRUCTION.
 5. ALL CANOPIES ARE NOT TO BE CONNECTED TO THE STRUCTURE OR ATTACHED TO ANY STRUCTURAL MEMBERS PRIOR INSTALLING THE ROOF/FLOOR DIAPHRAGM AND FULLY CONNECT TO ROOF/FLOOR JOISTS, BEAMS, AND COLUMNS.



Site Plan: North Arrow:

Consultant:

IE DESIGN
Intelligent Engineering Design Ltd.
STRUCTURAL ENGINEERS
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Seal: Seal:

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Project:

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1300 FANSHAW PARK RD. EAST. LONDON, ON

Drawing Title:

THREE-DIMENSIONAL SCHEMATICS II

Drawn By: D.H./S.D./D.K. Scale: AS INDICATED
Checked By: M.A.F./J.G. Plot Date: APR. 03-2024
Project Date: AUG. 2023
Project No: 2023-102
Drawing No: Revision

S-7.1 2

